# HCAOG 2024 REGIONAL TRANSPORTATION IMPROVEMENT PROGRAM (RTIP) – PROJECT CANDIDATE FORM

RTIP programming background:

If the project is on a State Highway, a Project Study Report (PSR) is required. If not, a PSR equivalent is required. The PSR equivalent at a minimum must be adequate to define and justify the project scope, cost and schedule. The PSR or PSR equivalent must be submitted with this programming request.

Applicant Agency:

Project Title:

Total Funding Requested:

Of the total funding, amount for active transportation components of project:\$400,000

Project Purpose: What transportation deficiency will this project address (safety, congestion, operations, plan implementation, etc.)? If a safety project, will the project reduce fatalities or number and severity of injuries?

Project Location (community name, corridor, street name, etc.):

Project Description:

Is the project in the 2022 RTP?

Yes No

Are you requesting State only funding?

Yes No

To the maximum extent feasible, have complete streets elements been included in the project? Explain.

If a rehabilitation project, is it located on a federal-aid eligible road (higher than a local or minor collector road? Link to Caltrans maps: <u>http://www.dot.ca.gov/hq/tsip/hseb/crs\_maps</u>

Yes No

Provide Project Component funding needs:

Project Component	Cost Estimate	STIP Funding Request	Other fund contribution	Allocation Schedule
Environmental Studies & Permits	\$	\$	\$	
Plans, Specifications & Estimates	\$	\$	\$	
Right of Way	\$	\$	\$	
Construction	\$	\$	\$	
Total	\$	\$	\$	

Please describe any other relevant information about this project you feel will be useful in project selection. Additional attachments (i.e. maps, photos) may also be included with the submittal.



MARK ARSENAULT CALTRANS DISTRICT 1 NORTH REGION ARCHAEOLOGIST

DATE

RUSSELL HANSEN CALTRANS DISTRICT 1 PROJECT LOCAL ASSISTANCE DATE

BRENDAN BYRD CITY OF FORTUNA CITY ENGINEER

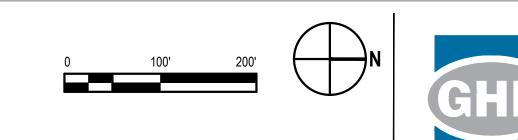
LEGEND:

AREA OF POTENTIAL EFFECTS

RD TUNA

10/13/2022

DATE



781 3rd St Eureka, CA 95501 USA T 1 707 443 8326 W www.ghd.com Filename: N:\US\Eureka\Projects\561\11214735\Digital\_Design\ACAD 2020\Figures\2132EX025.dwg Plot Date: 30 August 2022 - 12:58 PM



CITY OF FORTUNA KENMAR Rd/US 101 IC PROJECT 
 Project No.
 11214735

 Comp No.
 2132

 Date
 Aug 2022

PROJECT FOOTPRINT

FIGURE 2

# Project Study Report-Project Development Support (PSR-PDS)

То

# Request Programming for Capital Support (Project Approval and Environmental Document Phase) in the 2018 STIP

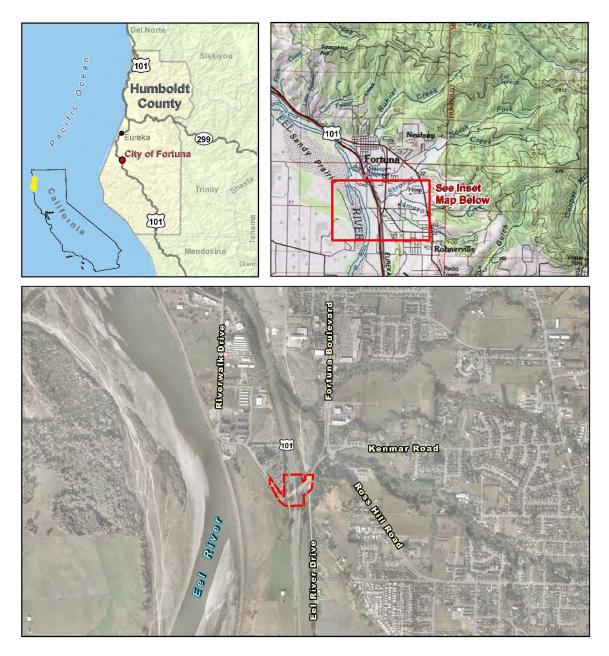
On Route	Kenmar Road
Between	500 Feet West of Kenmar Road US 101 Undercrossing
And	800 Feet East of Kenmar Road US 101 Undercrossing

APPROVED:

Merritt Perry, Director of Public Works/City Engineer

 $\frac{12/n/17}{\text{Date}}$ 

## Vicinity Map







This project study report-project development support has been prepared under the direction of the following registered civil engineer. The registered civil engineer attests to the technical information contained herein and the engineering data upon which recommendations, conclusions, and decisions are based.

12/8/2017 REGISTERED CIVIL ENGINEER DATE PROFESSIONAL Joshua Wolf C70358 No Exp. <u>9/30/18</u> STATE CIVIL OF CP

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## **Attachments**

- A. Location map
- B. Traffic Counts and LOS Analysis
- C. Review of Geometric Design Standards
  D. Conceptual Design Drawings
  E. Truck Turning Analysis
  F. Fast Path Exhibits

- G. Preliminary Structures AnalysisH. Landscaping/Gateway Concepts

- I. Cost Estimates
  J. Right-of-Way and Property Ownership
  K. Environmental Constraints Analysis

# 1. INTRODUCTION

## Project Description:

The project proposes to improve traffic, pedestrian, and bicycle operations at the Kenmar Road interchange with US 101 in Fortuna in Humboldt County. The existing intersection controls, roadway geometry, and the high volumes of local and regional traffic on Kenmar Road result in poor traffic operation at and near the interchange. Proposed project components include two roundabouts ("dog bone") on Kenmar Road at the intersections with the US 101 interchange, modifications to the US 101 on- and off-ramps, and the realignment of Eel River Drive. In addition to the proposed roadway improvements, the project includes a segment of Class I bike path through the project area in additional to other at-grade pedestrian and bicycle improvements to enhance pedestrian connections and promote regional bicycle network continuity.

This PSR-PDS was developed in conjunction with the Highway 101, Fortuna Downtown and Riverwalk Area Complete Streets and Connectivity Planning Study Study (GHD, 2016) which provides a detailed evaluation of interchange alternatives.

Table 1. I Toject Guillinary	
Project Limits	Kenmar between 500 feet west and 800 feet east of the Kenmar Road US 101 Undercrossing (BR
	04 0128, PM 59.50).
Number of Alternatives	5
Current Capital Outlay Support	\$550K
Estimate for PA&ED	
Current Capital Outlay Support	\$800K - \$1.1M
Estimate for PS&E	
Current Capital Outlay	\$4M - \$5.4M
Construction Cost Range	
Current Capital Outlay Right-of-	\$200K-\$300K
Way Cost Range	
Funding Source	RTIP/STIP
Type of Facility	Kenmar Road: 2-lane Other Principal
	Arterial/Major Collector
	Riverwalk Drive: 2-lane Major Collector
	Eel River Drive: 2-lane Major Collector US 101: 4-
	lane expressway/freeway
Number of Structures	1 (US 101 Kenmar Road UC)
Anticipated Environmental	CEQA Mitigated Negative Declaration
Determination or Document	NEPA CE
Legal Description	On Kenmar Road In Humboldt County in Fortuna
	between 500 feet west of US 101 Undercrossing
	and 800 feet east of US 101 Undercrossing.
Project Development Category	3

#### Table 1: Project Summary

# 2. BACKGROUND

The project need originates from desires expressed in the City's 2010 General Plan, user-based experiences and public request for improvements.

In 2016, a planning study was conducted to identify ways to improve access to the Riverwalk area and improve safety for all users (motorized & non-motorized), improve operations, apply Complete Streets concepts and create an entry statement/gateway, and ready the project for next steps in project development. The study was focused on US 101 interchanges at 12th Street and Kenmar Road and was funded by a 2015-2016 Sustainable Communities Planning Grant awarded to the Humboldt County Association of Governments (HCAOG) and the City of Fortuna as a sub-recipient.

The study process included researching and evaluating existing conditions, including right-of-way boundaries and ownership, maintenance responsibilities, identifying potentially sensitive environmental areas and potential permits, and obtaining traffic counts (motorized and non-motorized). Community meetings and stakeholder outreach were used to understand concerns with the existing facilities, solicit ideas for improvements, and obtain comments on preliminary design concepts. A deficiency analysis was performed to identify existing facilities which do not conform to current design standards or City goals. Traffic modeling showed that many of the intersections were operating below a level of service (LOS) C (Fortuna's standard) for current conditions, with the LOS expected to significantly decline for full buildout over 20-years with no improvements to the intersections.

### **Existing Conditions**

The project study area is focused on Kenmar Road from around 500 feet west and 800 feet east of Kenmar Road/US 101 undercrossing. Kenmar Road crosses under US 101, where the highway occupies parallel SB and NB bridges above grade. Within a short distance (approximately 600 feet), Kenmar Road has three intersections: at the SB on and off-ramps, the NB on and off-ramps, and at Eel River Drive. The Fortuna Park and Ride, which includes a bus stop for the Redwood Transit Main Line, is off Eel River Drive. A railroad crosses the road on the east side of US 101 between the NB on-ramp and off-ramp, and Eel River Drive intersection.

Kenmar Road consists of one vehicular travel lane in each direction with paved shoulders. The road varies in right-of-way and geometry due to intersections with Eel River Drive and South Fortuna Boulevard within 900 feet east of the Kenmar interchange. The current roadway configuration of the underpass consists of two 12 foot lanes, with eight foot shoulders, and a guard rail.

The intersection of Kenmar Road and US 101 SB Ramps is stop-controlled for the US 101 SB offramp and the eastbound approach of Kenmar Road. Left turns at the US 101 NB off ramp at Kenmar Road are stop controlled, with yield control only for the right turn. The existing intersection geometrics and control are shown in **Figure 1**.

There is a significant grade differential between Kenmar Road and the agricultural field to the south around the horizontal curve; guardrail is currently provided at the edge of travel way.

There are no designated pedestrian or bicycle facilities through the Kenmar Road corridor. However, there is a well-worn path behind the guardrail on the north side of the Kenmar Road underpass.

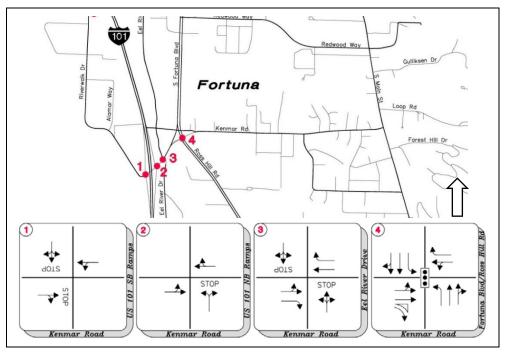


Figure 1: Existing Intersection Geometrics and Control

# 3. PURPOSE AND NEED

### Purpose:

- Simplify and improve navigation and traffic operations on Kenmar Road between Riverwalk Drive and Eel River Drive, including the Kenmar Road/US 101 interchange;
- Improve operations, reduce congestion, and minimize conflicts at the Kenmar Road intersections;
- Improve safety at Kenmar Road intersections;
- Improve the local and regional bicycle and pedestrian facilities through the Kenmar Road/US 101 interchange area; and
- Create a Gateway into south Fortuna.

### Need:

- Existing and future poor Level of Service (LOS) at the Kenmar Road intersections during peak hours as a result of stop-controlled intersections;
- Existing vehicle queue spillback from the Kenmar Road/US 101 ramp intersections onto the freeway off-ramps, especially in the southbound US 101 direction;
- No existing bicycle or pedestrian facilities resulting in a barrier to bicycle and pedestrian circulation and connectivity; and
- Intersections lack directional legibility, making it difficult for visitors to access the City's existing amenities.

Humboldt County's most significant regional thoroughfare for economic, tourist, recreational and commuting activity is US 101. The City of Fortuna is divided by US 101, which parallels the Eel River, and separates the Eel River and the Riverwalk Area from the majority of the City. Safer transportation alternatives, wayfinding signage, and improved traffic operations will support active living, provide better service to users, and support economic development and land use goals of the City.

# 4. TRAFFIC ENGINEERING PERFORMANCE ASSESSMENT

The information contained in this section is based on the findings from a preliminary traffic assessment conducted for the intersection at the Kenmar Road/US 101 interchange. The preliminary assessment include an Access Strategy and Configuration Assessment/Screening in accordance to the Intersection Control Evaluation (ICE) process outlined in Caltrans Traffic Operations Policy Directive 13-02. Refer to **Attachment B** for traffic counts and the LOS analysis. A Traffic Analysis Report will be developed during the PA&ED phase to further define the scope of work, and more accurately analyze and identify the forecasted operational impacts of the proposed improvements.

## **Existing Conditions**

<u>Traffic Counts:</u> The AM and PM peak hour intersection turn movement traffic counts were collected in March 2016. The AM peak hour is defined as one-hour of peak traffic flow counted between 7:00 am and 9:00 AM. The PM peak hour is defined as one-hour of peak traffic flow counted between 4:00 pm and 6:00 PM. The existing peak hour traffic volumes are presented in **Figure 2**.

<u>Bicycle and Pedestrian Counts:</u> HCAOG obtained bicycle and pedestrian counts for the project area in May 2016. The existing bicycle and pedestrian daily counts are presented in **Table 1**.

Intersection Name	Ave. Daily Bicycle Count	Ave. Daily Pedestrian Count
Kenmar Road/US 101 SB	22	20
Kenmar Road/US 101 NB	23	18

### Table 1: Average Totally Daily Bicycle and Pedestrian Counts

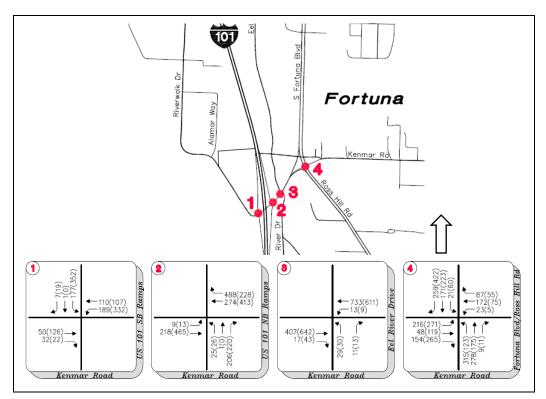


Figure 2: Existing Peak Hour Traffic Counts (2016)

## Modeling Tools and Methodologies

Operational deficiencies were estimated using future traffic volumes estimated using the travel demand model (TRAFFIX) prepared for the City's General Plan update. The cumulative condition was established by adding additional trips to the traffic counts obtained in 2016, by assuming full buildout over 20 years in accordance to the City's General Plan.

The existing, no build and signal alternatives were analyzed using Synchro/SimTraffic traffic analysis software. Roundabout alternatives were analyzed using Signalized and Unsignalized Intersection Design and Research Aid (SIDRA) analysis software. The LOS for all intersection control types were calculated using the methods documented in the Transportation Research Board Publication Highway Capacity Manual, 2010.

Synchro/SimTraffic was used to provide the queuing analysis. SimTraffic data was seeded into the network for 15 simulated minutes, and then recorded five runs of 60 simulated minutes. The 95<sup>th</sup>-percentile queue lengths were determined for each lane group based on an average of the five recorded runs. The 95<sup>th</sup>-percentile queue was defined to be the queue length (in feet) that has a 5-percent probability of being exceeded during the analysis time period. The 95<sup>th</sup>-percentile queue was utilized to determine the appropriate length of turn pockets.

### Summary of Existing Conditions Analysis and Findings

Existing weekday AM and PM peak hour intersection traffic operations were quantified utilizing the exiting traffic volumes and existing intersection lane geometrics and control. **Table 2** provides a summary of the existing vehicular AM and PM peak hour intersection delay and LOS. The following intersections were found to currently operate below the LOS C target:

- Kenmar Road and US 101 SB Ramps
- Kenmar Road and Eel River Drive

#### Table 2: Existing Levels of Service

Control	Target	AM Pea	k Hour	PM Pea	k Hour
Type <sup>1,2</sup>	LOS	Delay	LOS	Delay	LOS
TWSC	С	17.6	С	189	F
TWSC	С	10.8	В	14.4	В
OWSC	С	37.9	E	37.7	E
Signal	С	30.8	С	19.2	В
	Type <sup>1,2</sup> TWSC TWSC OWSC	Type1,2LOSTWSCCTWSCCOWSCC	Type1,2LOSDelayTWSCC17.6TWSCC10.8OWSCC37.9	Type1,2LOSDelayLOSTWSCC17.6CTWSCC10.8BOWSCC37.9E	Type1,2LOSDelayLOSDelayTWSCC17.6C189TWSCC10.8B14.4OWSCC37.9E37.7

Notes:

1. OWSC = One Way Stop Control; TWSC = Two Way Stop Control

2. LOS = Delay based on worst minor street approach for TWSC intersections, average of all approaches for AWSC and Signal

#### Summary of No Build Operation Analysis and Findings

**Table 3** provides a summary of the No Build intersection LOS for cumulative conditions. All intersections are expected to operate below an acceptable LOS for the No Build alternative with all operating at a LOS of F for PM peak hour conditions.

 Table 3: No Build Levels of Service

Intersection	Control	Target	AM Pea	k Hour	PM Pea	k Hour
intersection	Type <sup>1,2</sup>	LOS	Delay	LOS	Delay	LOS
Kenmar Road and US 101 SB Ramps	TWSC	С	94.5	F	>300	F
Kenmar Road and US 101 NB Ramps	TWSC	С	14.9	В	136.9	F
Kenmar Road and Eel River Drive	TWSC	С	181.2	F	>300	F
Kenmar Road and South Fortuna Boulevard/Ross Hill Road	Signal	С	67.8	E	168.5	F

Notes:

1. TWSC = Two Way Stop Control

2. LOS = Delay based on worst minor street approach for TWSC intersections, average of all approaches for AWSC and Signal

### Summary of Traffic Signal Operation Analysis and Findings

**Table 4** provides a summary of the intersection LOS for the signal intersections. All intersections are projected to operate at or above the threshold LOS for the signal alternative. **Figure 3** presents the cumulative peak hour volumes at the signalized intersections.

Table 4: Signalized Intersection Levels of Service

Intersection	Control	Target	AM Pea	k Hour	PM Pea	k Hour
Intersection	Type <sup>1</sup>	LOS	Delay	LOS	Delay	LOS
Kenmar Road and US 101 SB Ramps	Signal	С	21.8	С	31.2	С
Kenmar Road and US 101 NB Ramps	Signal	С	14.5	В	13.9	В
Kenmar Road and Eel River Drive	Signal	С	3.0	А	13.0	В
Kenmar Road and South Fortuna Boulevard/Ross Hill Road	Signal	С	30.8	С	23.1	В

Notes:

1. LOS = Delay based on worst minor street approach for TWSC intersections, average of all approaches for AWSC and Signal

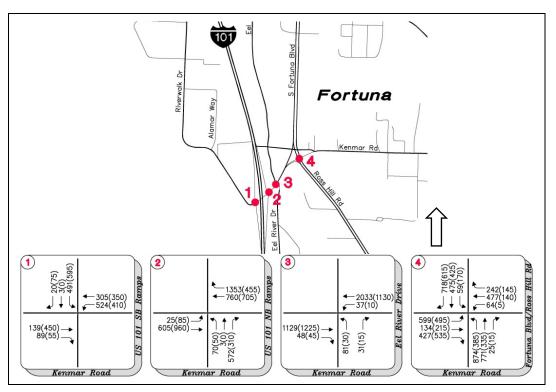


Figure 3: Cumulative Peak Volumes - Signalized Intersections

### Summary of Roundabout Operation Analysis and Findings

**Table 5** provides a summary of the intersection LOS for the roundabout intersections. All intersectionsare projected to operate at or above the threshold LOS for the roundabout alternatives. Figure 4presents the cumulative peak hour volumes at the roundabout intersections.

	Control	Target	AM Pea	k Hour	PM Pea	k Hour
Intersection	Type <sup>1,2</sup>	LOS	Delay	LOS	Delay	LOS
Kenmar Road and US 101 SB Ramps	RNDBT	С	8.4	A	16.6	В
Kenmar Road and US 101 NB Ramps	RNDBT	С	5.3	A	8.3	А
Kenmar Road and Eel River Drive	RNDBT	С	5.4	А	8.3	A
Kenmar Road and South Fortuna Boulevard/Ross Hill Road	RNDBT	С	11.0	В	18.0	В

Notes:

1. RNDBT = Roundbout

2. LOS = Delay based on worst minor street approach for TWSC intersections, average of all approaches for AWSC and Signal

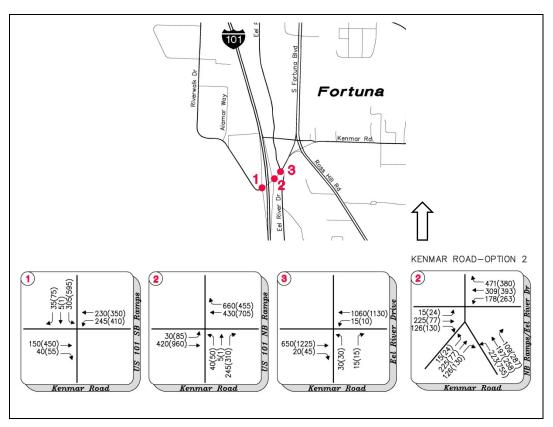


Figure 4: Cumulative Peak Volumes – Roundabout Intersections

#### Scope of Future Traffic Engineering Studies, Activities, & Tasks

The following discussion highlights the scope of traffic engineering studies, activities and tasks to be completed during the PA&ED phase.

Project Study Limits: The existing interchange study area includes the following intersections:

- Kenmar Road and US 101 Southbound Ramps
- Kenmar Road and US 101 Northbound Ramps
- Kenmar Road and Eel River Drive
- Kenmar Road and South Fortuna Boulevard/Ross Hill Road

<u>Traffic Data Collection</u>: The preliminary traffic assessment was prepared using existing AM and PM peak hour intersection traffic counts collected in March 2016 and bicycle/pedestrian counts collected in May 2016 for a preliminary study. Future traffic engineering studies may obtain new vehicle, pedestrian and bicycle traffic counts or may rely on the data already collected. Future traffic data obtained may include origin-destination surveys to gauge the movements and to observe driver behavior upon entry to the intersection.

<u>Traffic Forecasting</u>: In the PA&ED phase, the Project Development Team (PDT) may update the future design year forecasting of traffic volumes and movements for the Kenmar Road intersections within the project area based on new data or assumptions, if available.

<u>Traffic Safety Analysis:</u> A detailed study of the collision history will be developed during the PA&ED phase. The analysis will include the most recent collision data available for the project intersections.

<u>Intersection Control Evaluation</u>: An ICE Engineering Analysis (Step Two) will be prepared which may include intersection traffic control warrant studies, a capacity, operations and safety analysis, design performance checks, an economic analysis, and consultations with the District ICE Coordinator.

<u>Traffic Impacts during Construction</u>: The traffic impacts during construction will be evaluated and mitigation strategy identified. Special attention will be paid to the performance of non-standard geometric features, if any.

<u>Pedestrian and Bicycles Improvement Analysis:</u> During the PA&ED phase, additional analysis will be prepared to ensure the inclusion of context sensitive bicycle and pedestrian improvements, such as dedicated bike lanes, shared-use paths and crosswalks, some of which are included in the conceptual layouts of the alternatives. Preliminary designs will be analyzed to ensure adequate facilities are included to support bicyclists and pedestrians.

<u>Traffic Index for Pavement Design</u>: The traffic index (TI) required for the pavement design for the new pavement at the roundabout alternatives will be completed during the PA&ED Phase.

# 5. DEFICIENCIES

US 101 bisects the community cutting off the Riverwalk area from other areas of the City. The existing Kenmar interchange creates a significant barrier to bicycle and pedestrian movement, do not conform to current design standards, and will not accommodate future projected traffic volumes or the needs of roadway users. The interchanges also lack directional legibility, making it difficult for visitors to access the City's existing amenities.

### Existing and Forecasted (No Build) Operational Deficiencies

Traffic modeling conducted in 2016 showed that 2 of the 4 intersections analyzed are currently operating below a LOS C (Fortuna's standard). For full buildout over 20-years with no improvements to the intersections (No Build), all 4 intersections are expected to operate below a LOS C. Refer to **Table 6** for a summary of LOS for existing and no build future conditions.

#### Table 6: Existing Levels of Service

Intersection	Control	Target	Existin	g LOS	Future No	Build LOS
intersection	Type <sup>1,2</sup>	LOS	AM Peak	PM Peak	AM Peak	PM Peak
Kenmar Road and US 101 SB Ramps	TWSC	С	С	F	F	F
Kenmar Road and US 101 NB Ramps	TWSC	С	В	В	В	F
Kenmar Road and Eel River Drive	TWSC	С	E	E	F	F
Kenmar Road and South Fortuna Boulevard/Ross Hill Road	Signal	С	С	В	F	F
Notes:						

1. TWSC = Two Way Stop Control

2. LOS = Delay based on worst minor street approach for TWSC intersections, average of all approaches for AWSC and Signal

#### Geometric Design Deficiencies

The following summarizes the non-standard features and geometric deficiencies identified for existing conditions in the project area:

Kenmar Road:

- Curve Radii per HDM Index 203.2
- Decision Sight Distance per HDM Index 201.7
- Vertical Clearance per HDM 309.2

#### Eel River Drive:

- Curve Radii per HDM Index 203.2
- Decision Sight Distance per HDM Index 201.7
- Intersection Spacing per HDM Index 504.3
- Stopping Sight Distance per HDM Index 201.1

Refer to **Attachment C** for a more comprehensive review of existing conditions and project design standards.

#### Pedestrian and Bicycle Deficiencies

The existing Kenmar Road interchange lacks ADA-compliant pedestrian facilities. Bicycle infrastructure are absent from project area, except for bike lanes at the Kenmar Road underpass. The bike lanes at this location have no signage, control or connection to continuing facilities.

## 6. CORRIDOR AND SYSTEM COORDINATION

The following discussion highlights the state, regional and local planning considerations for the proposed project improvements.

#### State Planning

#### Complete Streets

Caltrans Deputy Directive 64-Revision (DD-64R) provides for the needs of travelers of all ages and abilities in all planning, programming, design, construction, operations, and maintenance activities on the State Highway System. The Department views all transportation improvements (new and retrofit) as opportunities to improve safety, access, and mobility for all travelers and recognizes bicycle, pedestrian, and transit modes as integral elements of the transportation system.

#### **Regional Planning**

The Kenmar Road Interchange Improvement Project was prioritized in HCAOG's 2017 Regional Transportation Plan (RTP) Update.

#### Local Planning

#### General Plan

The City of Fortuna General Plan 2030 (General Plan) formalizes a long-term vision for the City's physical development. The Kenmar Road Interchange Improvement Project fulfils or meets many policies set forth in the General Plan, including specific direction to improve interchanges within the study area. These policies are detailed below.

#### Roadways and Highways

Policy TC-1.1 Reducing Mode Conflicts. The City shall seek to minimize conflicts between pedestrians, automobiles, and bicycles.

Policy TC-1.2 New Roadway Improvements. The City shall design and phase roadway improvements so that a level of service (LOS) C or better is maintained on all City streets, except that LOS D or better shall be maintained on Main Street.

Policy TC-1.3 Balanced Transportation System. The City shall strive to meet the level of service standard through a balanced transportation system that provides alternatives to the automobile and by promoting pedestrian, bicycle, and transit connections between employment areas and major residential and commercial areas.

Policy TC-1.4 Improved LOS. The City shall identify economic, design, and planning solutions to improve levels of service currently below LOS C. Where physical mitigation is infeasible, the City shall consider developing programs that enhance alternative access or otherwise reduce automobile travel demand.

Policy TC-1.15 Interchange Improvements. The City, through HCAOG in cooperation with Caltrans, shall allocate the costs for funding interchange improvements to areas of benefit and assign proportionate share costs to individual projects.

#### **Bicycle and Trail Facilities**

Policy TC-5.2 Bicycle System. The City shall develop and maintain a safe, convenient, and effective bicycle system that encourages increased bicycle use.

Policy TC-5.5 Rails-to-Trails. The City shall explore the concept of converting any abandoned railroad rights-of-way into multi-use bike and pedestrian paths for local and regional use per Sections 2540 through 2549 of the Streets and Highways Code.

The General Plan proposes a Class I bike path on Kenmar Road/Riverwalk Drive west of the interchange, a trail along the existing rail corridor, and a Class I bike path near Eel River Drive (refer to **Figure 5**).

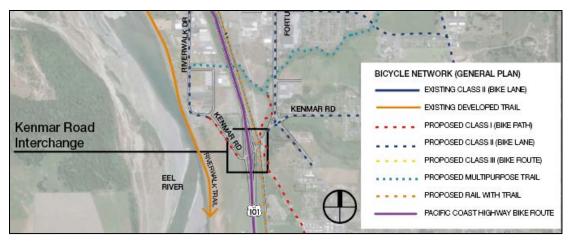


Figure 5: Bicycle Network in Fortuna General Plan

#### Pedestrian Facilities

Policy TC-4.2 New Developments. The City shall continue to require new development to finance and install sidewalks and pedestrian pathways connecting them to existing sidewalks or widening the right-of-way fronting the development to accommodate new sidewalks.

Policy TC-4.3 Specific Plans. The City shall encourage specific development plans to include design continuity of pedestrian access that enables residents to walk from their homes to places of work, recreation, and shopping.

Policy TC-4.7 Pedestrian Trails Interconnection. Where feasible, the City shall loop and interconnect pedestrian trails.

# 7. ALTERNATIVES

#### **Intersection Alternatives**

Unless noted otherwise, the alternatives identified below address the purpose and need of the project. Although the signal alternative does meet the projects purpose and need, its high estimated cost makes it infeasible for the City to implement, and therefore it is rejected from further consideration during the PA&ED phase. Only the roundabout are recommended to be carried forward in the PA&ED Phase of the project.

Based on the preliminary conceptual layouts, none of the build alternatives are anticipated to contain non-standard geometries with respect to both Caltrans Design Standards and City of Fortuna Design Standards. Further refinements to the proposed alternatives will be conducted during the PA&ED phase of the project.

Refer to **Attachment D** for conceptual design drawings, **Attachment E** for truck turning analysis, and **Attachment F** for roundabout fast path exhibits.

#### The "No Build" Alternative

This is the "No Build" condition, where the study intersections would remain unaltered with respect to intersection geometrics and stop control. This alternative does not meet the purpose and need of the proposed project.

#### Traffic Signal Alternative

This signalized intersection concept proposes three signals in close succession on Kenmar Road at the northbound on/offramp, southbound on/offramp, and at Eel River Drive. This alternative proposes

a mix of Class I and Class II bike facilities and a 7' wide sidewalk on the north side with connections to the three planned paths.

After analyzing the forecasted traffic volumes with Synchro, the lane geometry was determined for each intersection as shown conceptual design. Left-turn and right-turn pocket lengths were based on the 95<sup>th</sup>-percentile queue lengths.

For the signal alternative, the Kenmar Road corridor would require widening from the intersection of Kenmar Road and US 101 SB Ramps to the intersection of Kenmar Road and Fortuna Boulevard/Ross Hill Road. The current two lane roadway would require expansion to five lanes throughout the corridor to accommodate the projected growth. Widening of Kenmar Road and addition of bike lanes and sidewalks would require replacement of US 101 overcrossing bridge structure.

The signal alternative accommodates pedestrians and bicycles with standard Class II bike lanes, sidewalks, and intersection crossings along Kenmar Road and US 101 ramps. Each crossing is 10' wide and extends across the entire intersection length. Due to the number of lanes at each approach, long crosswalks would increase pedestrian crossing times and would affect the traffic signal timing to ensure that pedestrians can safely cross the roadway.

#### Roundabout Alternative 1 (a & b)

Roundabout alternative 1 uses roundabouts at the southbound and northbound on/off-ramps and two different options for the intersection with Eel River Drive:

- Option 1a: A third roundabout is included at the intersection with Eel River Drive.
- Option 1b: Eel River Drive is stop controlled with left turn movements onto and off Kenmar Road prohibited. Drivers desiring to make this movement would need to make a u-turn at the down-system intersections.

Both Option 1a and 1b include a 10' wide shared use path on the north side with connections to the three planned paths. In order to accommodate the path the Kenmar Road travel lanes would be reduced to 14 feet (with no shoulders).

Traffic modeling indicates that the NB Ramps intersection would operate at acceptable levels of service as a single lane roundabout with a westbound right-turn only lane. The SB off-ramp and EB Kenmar Road approaches the SB ramps intersection need a dedicated right-turn lane to operate at an acceptable level of service.

#### Roundabout Alternative 2

Roundabout alternative 2 uses a double ("dog bone") roundabout concept would place roundabouts on each side of US 101. The westerly roundabout accommodates traffic to and from the US 101 southbound offramp, Kenmar Road, and the southbound US 101 onramp. The easterly roundabout would manage traffic from Kenmar Road, the southerly reach of Eel River Drive, and northbound US 101 on and offramps. The northern portion of Eel River Drive would be realigned to cross the railroad and connect directly into the new roundabout located east of US 101.

Traffic modeling indicates that that the five-leg intersection would operate at an acceptable level of service as a single lane roundabout with a westbound right-turn only lane. The SB off-ramp and EB Kenmar Road approaches to the SB ramps intersection needed a dedicated right-turn lane to operate at an acceptable level of service. The concept includes 8' shoulders on each side of Kenmar Road under the existing freeway structure.

This design alternative includes a separated bike and walking path with connections to planned future trails, as well as pedestrian facilities throughout the system. The 10' wide shared use path on the north side would be located behind the existing structure columns. A retaining wall would be required beneath the structure to retain the highway/bridge embankment.

The realignment of Eel River Drive may allow for additional parking to be added to the park and ride lot, and access could be provided via a driveway on the realigned Eel River Drive or on Kenmar Road.

#### **Structure Alternatives**

A preliminary structures analysis was prepared to determine preliminary scope, feasibility, rough cost range, and a list of potential project risks for the proposed structures work. The full analysis and associated costs estimate are included in **Attachment G**.

US 101 spans over Kenmar Road on a bridge (Kenmar Road Undercrossing, BR. No. 04-0128). The bridge is skewed approximately 34 degrees to the right and is a 3-span, 133-foot-long, concrete teebeam structure, with a span arrangement of 34, 64, and 34 feet. The structure was constructed in 1962. End supports are diaphragm abutments on concrete pile foundations, and intermediate supports are 4-column bents on concrete pile foundations. The structure is in good condition with sufficiency rating equal to 98 and health index equal to 100. Kenmar Road currently passes under the 65 foot main span with a 14-foot 10-inch vertical clearance. The 40-foot-width of Kenmar Road currently accommodates two 12 foot travel lanes and two 8-foot shoulders. There are no sidewalks along either side of Kenmar Road.

In order to accommodate the proposed lane configurations and bicycle and connectivity on Kenmar Road at the US 101 interchange, the following structural alternatives were considered:

#### Signal Alternative

The signal alternative will add traffic signals and improve Kenmar Road in the City of Fortuna by widening the roadway, maintain profile grade, and adding a pedestrian sidewalk along the north side of the roadway. The widening would accommodate five 12-foot traffic-lanes, 5-foot shoulders each side of the roadway and a 7-foot-wide sidewalk along the north side of the road. The overall width of Kenmar Road improvement is approximately 77 feet including the sidewalk. In order to provide for widening and improving Kenmar Road to this extent, it will be necessary to replace the existing 3-span undercrossing. The existing bridge is in fair condition, however its' main span is insufficient dimension to accommodate the Kenmore Road improvements.

Based on the conditions at the site and the interchange geometrics, the new undercrossing will be a single-span, approximately 114 feet in length. The most economical structure type will likely be a precast, prestressed, concrete girder structure with a 6-foot structure depth. Supports would be high-cantilever wall type abutments founded on concrete piling. An increase in elevation of U.S. 101 on the order of 2 feet will be necessary to allow for a minimum 15 feet vertical clear distance from the bottom of soffit to Kenmar Road. The undercrossing will be designed to accommodate a Type 742 concrete left barrier, a minimum 10-foot left shoulder, two 12-foot lanes of southbound traffic, 5-foot southbound median shoulder, a Type 60 median barrier, a 5-foot northbound median shoulder, two 12-foot lanes northbound traffic, a 10-foot right shoulder, and a Type 742 concrete right barrier. Falsework would not necessary to erect this type of girder structure.

The new undercrossing can be constructed in two phases. The initial phase would likely be to remove and construct approximately the west half of the new bridge, while U.S. 101 traffic utilizes the east half of the existing bridge. The final phase would be to reroute U.S. 101 traffic to the new west half and remove and construct the east half of the new structure and a 3-foot wide deck closure pour.

#### Roundabout Alternatives

Roundabout alternative 2 will require a permanent retaining wall parallel to and in front of the north abutment of the existing Kenmar Road Undercrossing (Abutment 4) and to add traffic roundabouts each side of the interchange on Kenmar Road. The retaining wall in front of the abutment is to accommodate a 10-foot-wide pedestrian/bicycle facility under the structure. The total length of proposed wall will be approximately 180 feet.

The proposed wall layout line is 15 feet from the face of the existing columns; however, the layout line could be located as close as 10 feet from the face of existing columns. A Caltrans Type 7 retaining wall was considered for the proposed structure for the layout line 10 feet from the existing columns and the excavation for a Type 7 wall would likely be outside the influence zone of the Abutment 1 diaphragm. If the wall layout line is located more than 10 feet from the existing column face, then the new wall would need to be a permanent tie-back (ground anchor) diaphragm wall constructed from top

down in a minimum of three lifts. The maximum wall height above the pedestrian surface wouldbe approximately 12 feet depending on layout. The wall foundations would extend approximately 2 to 3 feet below finish grade. Cable railing will be mounted on top of the wall. Permanent tie-backs wjould require a permanent construction easement.

#### Pedestrian and Bike Facilities and Connectivity

#### Signal Alternative

The signal alternative accommodates pedestrians and bicycles with standard Class II bike lanes, sidewalks, and intersection crossings along Kenmar Road and US 101 ramps. Each crossing is 10' wide and extends across the entire intersection length. Due to the number of lanes at each approach, long crosswalks will increase pedestrian crossing times and will affect the traffic signal timing to ensure that pedestrians can safely cross the roadway.

### Roundabout Alternatives

Pedestrian crossings are provided along Kenmar Road and US 101 ramps for Kenmar Road Interchange roundabout alternatives. Crossings are 10 feet in width and set back a minimum of 20 feet from the roundabouts' circulating roadways. Where crosswalks intersect splitter islands or medians, a 6 foot long minimum paved pathway is provided between the travel lanes for safety and refuge when waiting to cross. Shared-use pathways, 10 feet in width and located outside of the roundabouts, are setback a minimum of 5 feet from the circulatory road with a landscape strip to increase accessibility and discourage pedestrians from crossing into the central traveled way.

Bicycles are accommodated by navigating through the roundabouts in two possible ways. Cyclists may choose to take the travel lane and travel through the roundabouts as a vehicle or may choose to take the separated bike ramp/shared use path and travel through the corridor as a pedestrian.

#### Gateway and Landscaping

Wayfinding, gateway aesthetics and plantings can be featured in each alternative in undeveloped open space along or within each intersection. Roundabouts, with their central landscape areas, lend themselves to focal points with artistic gateway treatments. Refer to **Attachment H** for preliminary landscape and gateway concepts.

### **Cost Estimates**

Capital, support, and total estimated costs for each alternative are summarized in **Table 7**. The total capital costs include traffic control, mobilization, right-of-way, utility relocation, and contingencies. The total support costs include costs for environmental clearance, plans, specifications, and estimates (PS&E), right-of-way engineering and acquisition, and construction support and management. Refer to **Attachment I** for detailed costs estimates for each alternative.

Alternative	Total Capital Cost	Total Support Cost	Total Estimated Cost (Rounded)
Signal	\$15.0M	\$6.7M	\$21.7
Roundabout 1a	\$4.4M	\$2.0M	\$6.4M
Roundabout 1b	\$4.2M	\$2.0M	\$6.2M
Roundabout 2	\$5.6M	\$2.4M	\$8.0M

#### Table 7: Cost Estimate Summary

#### Alternatives Comparison

A preliminary alternatives analysis was conducted to identify a preferred alternative. The analysis considered the following: cost, truck accommodation, safety, local access, complete streets, environmental impacts, right-of-way impacts, public input, and the purpose and need.

As previously stated, the signal alternative does meet the projects purpose and need, however, its high estimated cost makes it infeasible for the City to implement, and therefore it is rejected from further consideration.

The roundabout alternatives were generally considered comparable in terms of meeting the performance criteria, however roundabout alternative 2 was identified as the preferred alternative as it as it best met the performance criteria and was preferred by the public and stakeholders.

No design exceptions have been identified as at this point. However as the project is further developed, the need for exceptions to design standards should be analyzed.

## 8. RIGHT-OF-WAY

Initial research was conducted to determine road widths, rights-of-way, adjacent parcel ownerships and maintenance responsibilities, as these factors can affect feasible design solutions or preferred alternatives. Refer to **Attachment J** for additional information on right-of-way and ownership. Right-of-way data sheets will be prepared during the PA&ED phase of the project.

#### Ownership

Generally, the property in the immediate vicinity of the Kenmar interchanges is owned by public entities: Caltrans, the County of Humboldt, and the City of Fortuna. The underpass is owned by Caltrans and maintained by the County. The undeveloped Mill District Parcel is privately owned and accessed from the northern leg of Eel River Drive. Commercial land use (Riverwalk RV Park) is located southwest of the interchange. The railroad corridor is owned by the North Coast Railroad Authority (NCRA). Caltrans owns a small park and ride lot on the corner of Kenmar Road and Eel River Drive.

### **Right-of-Way Widths**

The width of Riverwalk Drive right-of-way west of US 101 has been determined to be 50-feet between the back of the walk on the east side and top of slope on the west side. Additional research and surveying will be needed to determine the right-of-way limits for Kenmar Road and Eel River Drive.

**Table 8** summarizes the approximate anticipated right-of-way impacts for each project alternative.

 Only roundabout alternative 1a is expected to require right-of-way acquisition (less than a tenth of an acre) to construct the northern leg of the Eel River Drive roundabout.

In addition to permanent acquisitions, temporary permissions/easements and/or encroachment permits will need to be obtained during the Right-of-Way phase of the project.

Alternative	APN #	Right-of-Wa	ay Acquisition
		SQFT	Acre
Signal	N/A	N/A	N/A
Roundabout Option 1a	201-331-005	3,772.58	0.09
Roundabout Option 1b	N/A	N/A	N/A
Roundabout Option 2	N/A	N/A	N/A

### Table 8: Right-of-way Impacts

### <u>Utilities</u>

Existing underground and above ground utilities in the vicinity of the Kenmar Road interchange will need to be modified or relocated to accommodate the proposed improvements. Utility ownership is presented in **Table 9**.

Utility	Owner
Storm Drain	Caltrans/City of Fortuna
Cable Television	Suddenlink
Telephone	AT&T
Electrical	PG&E
Water	City of Fortuna
RR Signal	NCAR

## Table 9: Utilities in Vicinity of Kenmar Road Interchange

### Railroad

The railroad corridor roughly parallels the east side of US 101 and crosses through the Kenmar Road project area. The NCRA is the public agency that owns right-of-way and the Northwestern Pacific Railroad (NWPRR) is the contract operator of the railroad. Together they have the responsibility for the safety, operation and maintenance of the railroad. Although there is currently not active rail service, any modifications to railroad crossings at roadway intersections will require the approval of the California Public Utilities Commission (CPUC) under General Order 88-B. As the project moves forward to project development, close coordination with the NCRA, NWPRR and the CPUC will be required to ensure that railroad operations are not impeded by interchange improvements.

## 9. STAKEHOLDER INVOLVEMENT

The project concepts were developed and vetted through a public process that included regular meetings of a Technical Advisory Group (TAG), the general public, and specific project stakeholders. This section discusses the results of the public and stakeholder engagement during the design development process.

### Technical Advisory Group (TAG)

A TAG was convened in January 2016 to support initial project planning and the development of project alternatives. The TAG met on five different occasions provide technical information relevant to the project, to coordinate with local agencies, and to act as the "eyes and ears" of the community to guide the project. Group members included representatives from HCAOG, the City of Fortuna, the Humboldt County Department Public Work, and Caltrans District 1.

### **Community Meetings**

Two workshops were held in March and July of 2016 to obtain public input into the project assessment and design. Outreach for the project was conducted with flyers, emails, radio public service announcements on six or more stations, social media posts, and targeted in-person outreach to colleagues and residents. In order to encourage participation, each workshop offered food, a childfriendly space with activities, and Spanish-English interpretation. Both workshops resulted in specific and helpful feedback from stakeholders that was utilized during the development and evaluation of design alternatives.

#### First Community Meeting (March 2016)

The goals of this first workshop was to understand how residents and visitors currently navigate Kenmar interchange area, to identify specific concerns related to safety, operations, and connectivity, and to discuss potential design treatments that could be implements

The attendees identified many challenges for pedestrians and bicyclists, including dark areas under crossings, narrow or virtually non-existent shoulders, and challenging road crossings. Meeting participants showed a preference for design alternatives involving roundabouts. Signalized intersection alternative comments were mostly mildly negative. Roundabout options with fewer roundabouts, and fewer bicycle/pedestrian crossings were preferred.

#### Second Community Meeting (July 2016)

The second workshop, conducted on July 20, 2016, was primarily focused on presenting design alternatives, answering questions and soliciting community feedback. There was a clear preference for the roundabout alternative 2 for Kenmar Road.

#### Public Presentations

The results of the 2016 Highway 101, Fortuna Downtown and Riverwalk Area Complete Streets and Connectivity Study was presented at the following public meetings:

- Fortuna City Council Meeting November 8, 2016
- HCAOG Board Meeting November 17, 2016
- HCAOG Technical Advisory Committee (TAC) Meeting December 1, 2016

In addition, the results of the study were presented to the Caltrans District 1 Executive Committee on January 3, 2017.

## 10. ENVIRONMENTAL COMPLIANCE

### Preliminary Environmental Analysis

In 2016, an initial environmental evaluation of the project and alternatives was conducted to help anticipate potential environmental constraints that may affect project design, alternatives, cost, schedule, and delivery. The evaluation included a reconnaissance-level site investigation of existing conditions in the project area to identify the presence or potential presence of biological resources listed under the Federal Endangered Species Act (ESA), the presence of wetlands and Waters of the US as regulated by the US Army Corps of Engineers (USACE), the presence or potential presence of species listed as endangered or threatened under the California Endangered Species Act (CESA) or considered a species of special concern (SSC) by the California Department of Fish and Wildlife (CDFW), or the potential for special-status plant species having a rare plant ranking as determined by the California Native Plant Society (CNPS) rare plant inventory, and to present the potential of sensitive habitats as listed by the CDFW. Refer to **Attachment K** for more information on the initial environmental evaluation that was prepared. During the PA&ED phase of the project, a formal Preliminary Environmental Analysis Report (PEAR) may be completed to satisfy Caltrans if required.

#### NEPA, CEQA and Permitting

During the PA&ED phase, the project will be evaluated for potential impacts on the environment in compliance with the California Environmental Quality Act (CEQA) and National Environmental Policy Act (NEPA). Feasible opportunities to avoid or reduce impacts will be pursued and mitigation measures will be developed to reduce potentially significant impacts as appropriate. The draft CEQA document will be made available to the public for review and comment.

Based on the information currently available, the expected compliance pathways are a Mitigated Negative Declaration of environmental impact for CEQA and a Categorical Exclusion for NEPA in conformance with the Federal Highways Administration/Caltrans programmatic process.

The wetland and riparian habitats in the project area have a moderate to high likelihood of supporting listed reptile, frog and fish species including Western Pond Turtle *Emys (Actinymys) marmorata*, Northern Red-legged Frog Rana aurora, and Foothill Yellow-legged Frog *Rana boylii*. Several sensitive plant species also have a moderate likelihood of occurring in the study area.

Subsequent environmental investigations including a wetland delineation will be needed to address potential sensitive species identified and address any impacts to protected habitats. Additionally, a variety of permits and related environmental review will be necessary for project planning and design.

Anticipated Environmental Permitting and Compliance Requirements are presented in Table 10.

Law/Regulation	Permit/Approval	Authority
CEQA	Mitigated Negative Declaration	Lead Agency
NEPA	Categorical Exclusion	Caltrans on behalf of Federal Highways Administration
Clean Water Act Section 404	Nationwide Permit	US Army Corps of Engineers
Porter-Cologne/Clean Water Act Section 401	401 Certification and/or Waste Discharge Requirements (WDR)	North Coast Regional Water Quality Control Board
National Historic Preservation Act	Letter of Concurrence	State Historic Preservation Office & Tribal Historic Preservation Office

Table 10. Anticipated Environmental Permitting and Compliance Requirements

A Preliminary Environmental Study (PES) Form will be developed during the PA&ED phase of the project. The following technical studies and plans are anticipated to be required:

- Natural Environmental Study (NES) of Biological Resources
- Wetland Delineation and Rare Plant Survey
- Initial Site Assessment (ISA)
- Visual Impact Assessment
- Floodplain Evaluation & Location Hydraulic Study
- Geotechnical Investigation
- Historic Property Survey Report (HPSR) and Archeological Survey Report (ARS)
- Stormwater Data Report

## 11. FUNDING

In 2017, HCAOG prepared its 2018 Regional Transportation Improvement Program (RTIP) for incorporation into the 2018 State Transportation Improvement Program (STIP) cycle. The RTIP identifies state highway and local agency projects proposed for funding from Fiscal Year (FY) 2018-19 through 2022-23 based on the amount of funding available to the region. In addition, HCAOG requested an advance of \$550,000 of funding for the US 101/Kenmar Road Interchange Project through the Advanced Project Development Element (APDE). The APDE is an advancement of future

regular regional RTIP funds which provides funding for environmental, permits, plans, specifications and estimates. The advanced funds were requested to be programmed in FY 2019-20. The California Transportation Commission will make a decision on HCAOG's RTIP at its meeting on March 21/22, 2018.

Funding for PS&E, right-of-way and construction has not been programmed. Potential funding sources for PS&E, right-of-way and construction include: the state Active Transportation Program (ATP), the federal TIGER program, future STIP cycles, and local funds.

It has been determined that this project is eligible for Federal-aid funding.

## **Capital Outlay Project Estimate**

**Table 11** presents a summary of the capital outlay estimates for the proposed alternatives. Detailed estimates for the various alternatives are presented in **Attachment I**.

	Range of Es	timate	Federal F	unds	Local Fu	nds
Alternative	Construction	Right- of-Way	Construction	Right-of- Way	Construction	Right-of- Way
Signal	\$14.8M	\$200K	TBD	TBD	TBD	TBD
Roundabout 1a	\$4.1M	\$300K	TBD	TBD	TBD	TBD
Roundabout 1b	\$4.0M	\$200K	TBD	TBD	TBD	TBD
Roundabout 2	\$5.4M	\$200K	TBD	TBD	TBD	TBD

 Table 11: Summary of Capital Outlay Estimate

The level of detail available to develop these capital outlay project estimates is only accurate to within the above ranges and is useful for long-range planning purposes only. The capital outlay project estimates should not be used to program or commit State-programmed capital outlay funds.

## **Capital Outlay Support Estimate**

Capital outlay support estimate for programming PA&ED in the 2018 STIP for this project: \$550,000.

# 12. DELIVERY SCHEDULE

The following section outlines the delivery schedule for the proposed alternative.

#### Table 10: Project Delivery Schedule

Project Milestones	Scheduled Delivery Date
Program Project	12/15/2017
Begin Environmental (PA&ED) Phase	07/01/2019
Circulate Draft Environmental Document	07/01/2020
Draft Project Report	12/1/2020
End Environmental Milestone	6/30/2021

The anticipated funding fiscal year for construction is 2025/26.

## 13. RISKS

A risk register and risk analysis will be complete for the project during the PA&ED phase of the project.

# 14. EXTERNAL AGENCY COORDINATION

This project does not anticipate coordination with the Federal Highway Administration (FHWA).

Coordination between the City of Fortuna, County of Humboldt and Caltrans will be required throughout all phases of this project. In addition, the project will require the following coordination:

<u>Caltrans</u> Encroachment Permit

<u>US Army Corps of Engineers</u> Department of the Army Permit for: Clean Water Act Section 404

<u>California Department of Fish and Wildlife</u> California Fish and Game Code Section 1602 Lake or Streambed Alteration Agreement

<u>California Coastal Commission and/or Local Coastal Program</u> California Public Resources Code Division 20 (California Coastal Act) Coastal Development Permit

Regional Water Quality Control Board Clean Water Act Section 401 Water Quality Certification

Railroads North Coast Railroad Authority

<u>California Public Utilities Commission</u> Modification to an Existing Rail Crossing, GO-88B

# 15. PROJECT REVIEWS

Caltrans District 1 has indicated that a formal review of this PSR is not required at this time. The City will engage Caltrans prior to proceeding with PA&ED at which time formal Caltrans reviews can occur, if requested.

## 16. PROJECT PERSONNEL

#### **City of Fortuna**

Merritt Perry, Director of Public Works/City Engineer, 707-725-1469 Kevin Carter, Deputy Director of Public Works, 707-725-1472 Mike Johnson, General Services Superintendent, 707-725-1466

#### **Caltrans District 1**

Kevin Tucker, Advanced Planning, 707-441-5770 Jesse Roberts, Transportation Planning, 707-441-4693

#### Humboldt County Association of Governments

Marcella Clem, Director, 707-444-8208

### **County of Humboldt**

Tony Seghetti, Deputy Director Engineering, 707-445-7377 Tom Mattson, Director of Public Works, 707-445-7491

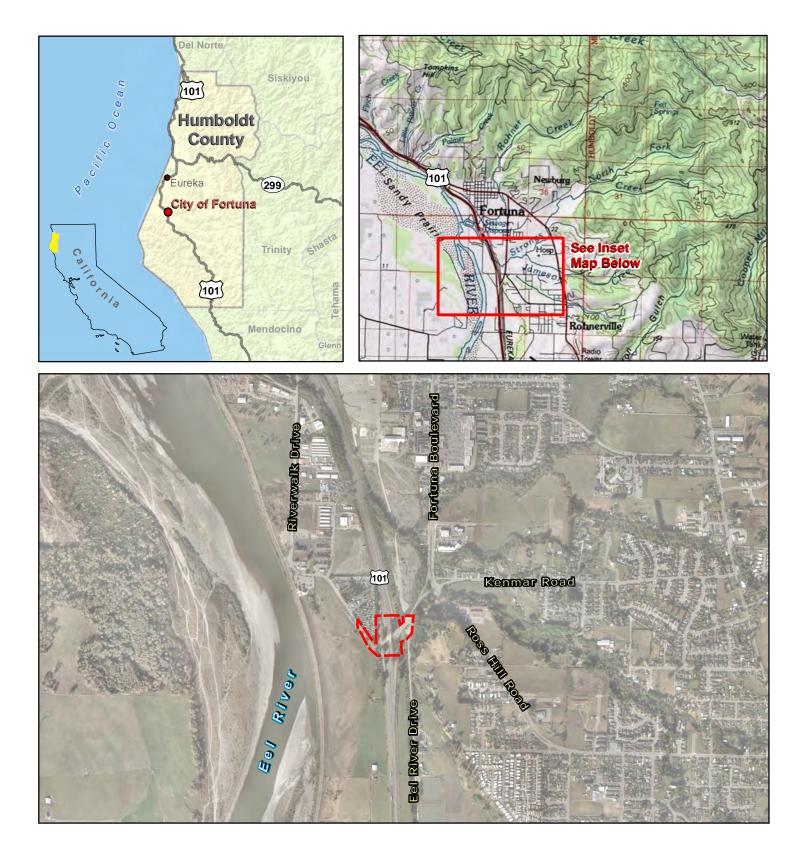
#### GHD (Consultant)

Josh Wolf, Project Manager, 707-443-8326

## 17. ATTACHMENTS

- L. Location map
- M. Traffic Counts and LOS Analysis
- N. Review of Geometric Design Standards
- O. Conceptual Design Drawings
- P. Truck Turning Analysis
- Q. Fast Path Exhibits
- R. Preliminary Structures Analysis
- S. Landscaping/Gateway Concepts
- T. Cost Estimates
- U. Right-of-Way and Property Ownership
- V. Environmental Constraints Analysis

**Attachment A - Location map** 





Paper Size 8.5" x 11" (ANSI A) City of Fortuna Job Number | 11109149 0 200400600800,000 Kenmar Road Interchange Improvements Revision A 06 Dec 2017 Date Feet Map Projection: Lambert Conformal Conic Horizontal Datum: North American 1983 Grid: NAD 1983 StatePlane California I FIPS 0401 Feet Vicinity Map Figure 1

C:\111\11109149 HCAOG Hwy 101 Fortuna Downtown-Riverwalk\08-GIS\Maps\Figures\Recon\_WetlandsHabitat\F1\_Vicinity\_171206.mxd © 2012. While every care has been taken to prepare this map, GHD (and DATA CUSTODIAN) make no representations or warranties about its accuracy, reliability, completeness or suitability for any particular purpose and cannot accept liability and responsibility of any way and for any reason. Data source: City of Fortuna Aerial, 2010; GHD data, 2013; USA Topo Maps; Streetmap USA. Created by:gldavidson W www.ghd.com

Attachment B - Traffic Counts and LOS Analysis

**Existing Conditions** 

6.7

## Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	0	50	32	189	110	0	0	0	0	177	1	7
Future Vol, veh/h	0	50	32	189	110	0	0	0	0	177	1	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	3	3	3	3	3	3	3	3	3	3	3	3
Mvmt Flow	0	53	34	199	116	0	0	0	0	186	1	7

Major/Minor			Major2			Minor2		
Conflicting Flow All			0	0	0	514	514	116
Stage 1			-	-	-	514	514	-
Stage 2			-	-	-	0	0	-
Critical Hdwy			4.13	-	-	7.13	6.53	6.23
Critical Hdwy Stg 1			-	-	-	6.13	5.53	-
Critical Hdwy Stg 2			-	-	-	-	-	-
Follow-up Hdwy			2.227	-	-			3.327
Pot Cap-1 Maneuver			-	-	0	469	463	934
Stage 1			-	-	0	541	534	-
Stage 2			-	-	0	-	-	-
Platoon blocked, %				-				
Mov Cap-1 Maneuver			-	-	-	469	463	934
Mov Cap-2 Maneuver			-	-	-	469	463	-
Stage 1			-	-	-	541	534	-
Stage 2			-	-	-	-	-	-
Approach			WB			SB		
HCM Control Delay, s						17.6		
HCM LOS						С		
Minor Lane/Major Mvmt	WBL	WBT SBLn1						
Capacity (veh/h)	-	- 478						
HCM Lana V/C Patia		0 407						

HCM Lane V/C Ratio	-	- 0.407	
HCM Control Delay (s)	-	- 17.6	
HCM Lane LOS	-	- C	
HCM 95th %tile Q(veh)	-	- 2	

## Intersection

Int Delay, s/veh

3.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	9	218	0	0	274	488	25	1	206	0	0	0
Future Vol, veh/h	9	218	0	0	274	488	25	1	206	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	Free	-	-	Yield	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	10	251	0	0	315	561	29	1	237	0	0	0

Major/Minor	Major1			Major2			Minor1			
Conflicting Flow All	315	0	-	-	-	0	586	586	251	
Stage 1	-	-	-	-	-	-	271	271	-	
Stage 2	-	-	-	-	-	-	315	315	-	
Critical Hdwy	4.12	-	-	-	-	-	6.42	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	5.42	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	5.42	5.52	-	
Follow-up Hdwy	2.218	-	-	-	-	-	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1245	-	0	0	-	0	473	422	788	
Stage 1	-	-	0	0	-	0	775	685	-	
Stage 2	-	-	0	0	-	0	740	656	-	
Platoon blocked, %		-			-					
Mov Cap-1 Maneuver	1245	-	-	-	-	-	469	0	788	
Mov Cap-2 Maneuver	-	-	-	-	-	-	469	0	-	
Stage 1	-	-	-	-	-	-	768	0	-	
Stage 2	-	-	-	-	-	-	740	0	-	

Approach	EB	WB	NB	
HCM Control Delay, s	0.3	0	10.8	
HCM LOS			В	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	WBT
Capacity (veh/h)	887	1245	-	-
HCM Lane V/C Ratio	0.301	0.008	-	-
HCM Control Delay (s)	10.8	7.9	0	-
HCM Lane LOS	В	А	Α	-
HCM 95th %tile Q(veh)	1.3	0	-	-

1.3

## Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	0	407	17	13	733	0	29	0	11	0	0	0
Future Vol, veh/h	0	407	17	13	733	0	29	0	11	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	473	20	15	852	0	34	0	13	0	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	852	0	0	493	0	0	1366	1366	483	1373	1376	852
Stage 1	-	-	-	-	-	-	483	483	-	883	883	-
Stage 2	-	-	-	-	-	-	883	883	-	490	493	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	787	-	-	1071	-	-	124	147	584	123	145	359
Stage 1	-	-	-	-	-	-	565	553	-	340	364	-
Stage 2	-	-	-	-	-	-	340	364	-	560	547	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	787	-	-	1071	-	-	121	143	584	118	141	359
Mov Cap-2 Maneuver	-	-	-	-	-	-	121	143	-	118	141	-
Stage 1	-	-	-	-	-	-	565	553	-	340	354	-
Stage 2	-	-	-	-	-	-	331	354	-	548	547	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.1	37.9	0
HCM LOS			E	А

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	BLn1
Capacity (veh/h)	155	787	-	-	1071	-	-	-
HCM Lane V/C Ratio	0.3	-	-	-	0.014	-	-	-
HCM Control Delay (s)	37.9	0	-	-	8.4	0	-	0
HCM Lane LOS	E	А	-	-	А	А	-	А
HCM 95th %tile Q(veh)	1.2	0	-	-	0	-	-	-

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्भ	1		र्भ	1	ሻ	<b>∱</b> }		ሻ	- 11	
Traffic Volume (veh/h)	216	48	154	23	172	87	315	278	9	21	171	259
Future Volume (veh/h)	216	48	154	23	172	87	315	278	9	21	171	259
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1863	1863	1900	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	257	57	183	27	205	104	375	331	11	25	204	0
Adj No. of Lanes	0	1	1	0	1	1	1	2	0	1	2	0
Peak Hour Factor	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	312	69	337	35	268	259	420	1160	38	49	434	0
Arrive On Green	0.21	0.21	0.21	0.16	0.16	0.16	0.24	0.33	0.33	0.03	0.12	0.00
Sat Flow, veh/h	1465	325	1583	216	1636	1583	1774	3496	116	1774	3632	0.00
Grp Volume(v), veh/h	314	0	183	232	0	104	375	167	175	25	204	0
Grp Sat Flow(s),veh/h/ln	1790	0	1583	1852	0	1583	1774	1770	1842	1774	1770	0
Q Serve(g_s), s	11.4	0.0	7.0	8.2	0.0	4.0	13.9	4.8	4.8	0.9	3.7	0.0
Cycle Q Clear(g_c), s	11.4	0.0	7.0	8.2	0.0	4.0	13.9	4.8	4.8	0.9	3.7	0.0
Prop In Lane	0.82		1.00	0.12		1.00	1.00		0.06	1.00		0.00
Lane Grp Cap(c), veh/h	381	0	337	303	0	259	420	587	611	49	434	0
V/C Ratio(X)	0.82	0.00	0.54	0.77	0.00	0.40	0.89	0.28	0.29	0.51	0.47	0.00
Avail Cap(c_a), veh/h	472	0	418	489	0	418	455	794	827	140	960	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	25.6	0.0	23.9	27.3	0.0	25.5	25.2	16.8	16.8	32.7	27.8	0.0
Incr Delay (d2), s/veh	9.4	0.0	1.4	4.0	0.0	1.0	18.6	0.3	0.3	7.9	0.8	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.6	0.0	3.2	4.5	0.0	1.8	8.9	2.4	2.5	0.6	1.8	0.0
LnGrp Delay(d),s/veh	35.0	0.0	25.2	31.3	0.0	26.5	43.8	17.1	17.1	40.6	28.6	0.0
LnGrp LOS	D	0.0	C	C	0.0	C	D	В	В	D	C	0.0
Approach Vol, veh/h		497			336			717			229	
Approach Delay, s/veh		31.4			29.8			31.0			29.9	
Approach LOS		C			C			C			C	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	6.4	27.1		4 19.0	20.7	12.9		15.7				
Change Period (Y+Rc), s	4.5	4.5		4.5	4.5	4.5		4.5				
Max Green Setting (Gmax), s	4.5 5.4	30.6		4.5	4.5	4.5		4.5				
Max Q Clear Time (g_c+I1), s	2.9	6.8		13.4	15.9	5.7		10.0				
Green Ext Time (p_c), s	2.9 0.0	0.0 3.4		13.4	0.2	2.7		1.0				
u = 71	0.0	J.4		1.1	0.2	2.1		1.0				
Intersection Summary												
HCM 2010 Ctrl Delay			30.8									
HCM 2010 LOS			С									

86.6

#### Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	0	126	22	332	107	0	0	0	0	352	0	19
Future Vol, veh/h	0	126	22	332	107	0	0	0	0	352	0	19
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	134	23	353	114	0	0	0	0	374	0	20

Major/Minor	Major2			Minor2		
Conflicting Flow All	0	0	0	820	820	114
Stage 1	-	-	-	820	820	-
Stage 2	-	-	-	0	0	-
Critical Hdwy	4.12	-	-	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	2.218	-	-	3.518	4.018	3.318
Pot Cap-1 Maneuver	-	-	0	~ 294	310	939
Stage 1	-	-	0	~ 369	389	-
Stage 2	-	-	0	-	-	-
Platoon blocked, %		-				
Mov Cap-1 Maneuver	-	-	-	~ 294	310	939
Mov Cap-2 Maneuver	-	-	-	~ 294	310	-
Stage 1	-	-	-	~ 369	389	-
Stage 2	-	-	-	-	-	-
Approach	WB			SB		
HCM Control Delay				180		

HCM Control Delay, s	189
HCM LOS	F

Minor Lane/Major Mvmt	WBL	WBT SBLn1	
Capacity (veh/h)	-	- 305	
HCM Lane V/C Ratio	-	- 1.294	
HCM Control Delay (s)	-	- 189	
HCM Lane LOS	-	- F	
HCM 95th %tile Q(veh)	-	- 19	
Notes			

~: Volume exceeds capacity \$:

\$: Delay exceeds 300s +: Computation Not Defined

\*: All major volume in platoon

### Intersection

Int Delay, s/veh

3.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	13	465	0	0	413	228	26	0	220	0	0	0
Future Vol, veh/h	13	465	0	0	413	228	26	0	220	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	Free	-	-	Yield	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	14	495	0	0	439	243	28	0	234	0	0	0

Major/Minor	Major1			Major2			Minor1			
Conflicting Flow All	439	0	-	-	-	0	961	961	495	
Stage 1	-	-	-	-	-	-	522	522	-	
Stage 2	-	-	-	-	-	-	439	439	-	
Critical Hdwy	4.12	-	-	-	-	-	6.42	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	5.42	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	5.42	5.52	-	
Follow-up Hdwy	2.218	-	-	-	-	-	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1121	-	0	0	-	0	284	256	575	
Stage 1	-	-	0	0	-	0	595	531	-	
Stage 2	-	-	0	0	-	0	650	578	-	
Platoon blocked, %		-			-					
Mov Cap-1 Maneuver	1121	-	-	-	-	-	279	0	575	
Mov Cap-2 Maneuver	-	-	-	-	-	-	279	0	-	
Stage 1	-	-	-	-	-	-	585	0	-	
Stage 2	-	-	-	-	-	-	650	0	-	

Approach	EB	WB	NB	
HCM Control Delay, s	0.2	0	14.4	
HCM LOS			В	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	WBT
Capacity (veh/h)	643	1121	-	-
HCM Lane V/C Ratio	0.407	0.012	-	-
HCM Control Delay (s)	14.4	8.3	0	-
HCM Lane LOS	В	А	Α	-
HCM 95th %tile Q(veh)	2	0	-	-

1.2

### Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	0	642	43	9	611	0	30	0	13	0	0	0
Future Vol, veh/h	0	642	43	9	611	0	30	0	13	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	1	1	1	1	1	1	1	1	1	1	1	1
Mvmt Flow	0	683	46	10	650	0	32	0	14	0	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	650	0	0	729	0	0	1375	1375	706	1382	1398	650
Stage 1	-	-	-	-	-	-	706	706	-	669	669	-
Stage 2	-	-	-	-	-	-	669	669	-	713	729	-
Critical Hdwy	4.11	-	-	4.11	-	-	7.11	6.51	6.21	7.11	6.51	6.21
Critical Hdwy Stg 1	-	-	-	-	-	-	6.11	5.51	-	6.11	5.51	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.11	5.51	-	6.11	5.51	-
Follow-up Hdwy	2.209	-	-	2.209	-	-	3.509	4.009	3.309	3.509	4.009	3.309
Pot Cap-1 Maneuver	941	-	-	879	-	-	123	146	438	122	141	471
Stage 1	-	-	-	-	-	-	428	440	-	449	457	-
Stage 2	-	-	-	-	-	-	449	457	-	424	430	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	941	-	-	879	-	-	121	143	438	117	138	471
Mov Cap-2 Maneuver	-	-	-	-	-	-	121	143	-	117	138	-
Stage 1	-	-	-	-	-	-	428	440	-	449	449	-
Stage 2	-	-	-	-	-	-	441	449	-	411	430	-
-												
Approach	ED			\//D			ND			CD		

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.1	37.7	0
HCM LOS			E	А

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	BLn1
Capacity (veh/h)	155	941	-	-	879	-	-	-
HCM Lane V/C Ratio	0.295	-	-	-	0.011	-	-	-
HCM Control Delay (s)	37.7	0	-	-	9.1	0	-	0
HCM Lane LOS	E	А	-	-	А	Α	-	А
HCM 95th %tile Q(veh)	1.2	0	-	-	0	-	-	-

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्भ	1		र्भ	1	7	<b>∱</b> î≽		5	<u></u>	
Traffic Volume (veh/h)	271	119	265	5	75	55	123	175	11	60	223	422
Future Volume (veh/h)	271	119	265	5	75	55	123	175	11	60	223	422
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1881	1881	1900	1881	1881	1881	1881	1900	1881	1881	1900
Adj Flow Rate, veh/h	274	120	268	5	76	56	124	177	11	61	225	0
Adj No. of Lanes	0	1	1	0	1	1	1	2	0	1	2	0
Peak Hour Factor	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Percent Heavy Veh, %	1	1	1	1	1	1	1	1	1	1	1	1
Cap, veh/h	359	157	454	10	157	143	165	612	38	105	520	0
Arrive On Green	0.28	0.28	0.28	0.09	0.09	0.09	0.09	0.18	0.18	0.06	0.15	0.00
Sat Flow, veh/h	1264	554	1599	116	1760	1599	1792	3420	211	1792	3668	0
Grp Volume(v), veh/h	394	0	268	81	0	56	124	92	96	61	225	0
Grp Sat Flow(s),veh/h/ln	1818	0	1599	1875	0	1599	1792	1787	1844	1792	1787	0
Q Serve(g_s), s	9.2	0.0	6.7	1.9	0.0	1.5	3.1	2.1	2.1	1.5	2.7	0.0
Cycle Q Clear(g_c), s	9.2	0.0	6.7	1.9	0.0	1.5	3.1	2.1	2.1	1.5	2.7	0.0
Prop In Lane	0.70	0.0	1.00	0.06	0.0	1.00	1.00		0.11	1.00		0.00
Lane Grp Cap(c), veh/h	517	0	454	168	0	143	165	320	330	105	520	0
V/C Ratio(X)	0.76	0.00	0.59	0.48	0.00	0.39	0.75	0.29	0.29	0.58	0.43	0.00
Avail Cap(c_a), veh/h	707	0	622	729	0	622	677	1181	1219	209	1428	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	15.1	0.0	14.2	20.1	0.0	19.9	20.5	16.5	16.5	21.2	18.0	0.0
Incr Delay (d2), s/veh	3.3	0.0	1.2	2.1	0.0	1.7	6.7	0.5	0.5	5.0	0.6	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.1	0.0	3.1	1.1	0.0	0.7	1.9	1.1	1.1	0.9	1.4	0.0
LnGrp Delay(d),s/veh	18.5	0.0	15.5	22.2	0.0	21.6	27.2	16.9	16.9	26.2	18.6	0.0
LnGrp LOS	B	0.0	B	C	0.0	C	C	B	B	C	B	0.0
Approach Vol, veh/h		662	_		137			312			286	
Approach Delay, s/veh		17.3			22.0			21.0			20.2	
Approach LOS		B			C			C			C	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	Ŭ	4	5	6		8				
Phs Duration (G+Y+Rc), s	7.2	12.8		17.7	8.8	11.2		8.6				
Change Period (Y+Rc), s	4.5	4.5		4.5	4.5	4.5		4.5				
Max Green Setting (Gmax), s	4.J 5.4	30.6		18.0	4.5	18.5		18.0				
Max Q Clear Time (g c+l1), s	3.5	4.1		11.2	5.1	4.7		3.9				
Green Ext Time (p_c), s	0.0	2.6		2.0	0.2	2.1		0.4				
u = 7:	0.0	2.0		2.0	0.2	2.1		0.4				
Intersection Summary			40.0									
HCM 2010 Ctrl Delay			19.2									_
HCM 2010 LOS			В									

# **Cumulative No Build Alternative**

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### Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	0	150	40	240	230	0	0	0	0	305	5	35
Future Vol, veh/h	0	150	40	240	230	0	0	0	0	305	5	35
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	3	3	3	3	3	3	3	3	3	3	3	3
Mvmt Flow	0	158	42	253	242	0	0	0	0	321	5	37

Major/Minor			Major2			Minor2		
Conflicting Flow All			0	0	0	747	747	242
Stage 1			-	-	-	747	747	-
Stage 2			-	-	-	0	0	-
Critical Hdwy			4.13	-	-	7.13	6.53	6.23
Critical Hdwy Stg 1			-	-	-	6.13	5.53	-
Critical Hdwy Stg 2			-	-	-	-	-	-
Follow-up Hdwy			2.227	-	-		4.027	
Pot Cap-1 Maneuver			-	-	0	328	340	794
Stage 1			-	-	0	403	419	-
Stage 2			-	-	0	-	-	-
Platoon blocked, %				-				
Mov Cap-1 Maneuver			-	-	-	328	340	794
Mov Cap-2 Maneuver			-	-	-	328	340	-
Stage 1			-	-	-	403	419	-
Stage 2			-	-	-	-	-	-
Approach			WB			SB		
HCM Control Delay, s						94.5		
HCM LOS						F		
Minor Lane/Major Mvmt	WBL	WBT SBLn1						

Capacity (veh/h)	-	- 349	
HCM Lane V/C Ratio	-	- 1.041	
HCM Control Delay (s)	-	- 94.5	
HCM Lane LOS	-	- F	
HCM 95th %tile Q(veh)	-	- 12.6	

### Intersection

Int Delay, s/veh

3.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	30	425	0	0	430	660	40	5	245	0	0	0
Future Vol, veh/h	30	425	0	0	430	660	40	5	245	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	Free	-	-	Yield	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	34	483	0	0	489	750	45	6	278	0	0	0

Major/Minor	Major1			Major2			Minor1			
Conflicting Flow All	489	0	-	-	-	0	1040	1040	483	
Stage 1	-	-	-	-	-	-	551	551	-	
Stage 2	-	-	-	-	-	-	489	489	-	
Critical Hdwy	4.12	-	-	-	-	-	6.42	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	5.42	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	5.42	5.52	-	
Follow-up Hdwy	2.218	-	-	-	-	-	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1074	-	0	0	-	0	255	230	584	
Stage 1	-	-	0	0	-	0	577	515	-	
Stage 2	-	-	0	0	-	0	616	549	-	
Platoon blocked, %		-			-					
Mov Cap-1 Maneuver	1074	-	-	-	-	-	244	0	584	
Mov Cap-2 Maneuver	-	-	-	-	-	-	244	0	-	
Stage 1	-	-	-	-	-	-	552	0	-	
Stage 2	-	-	-	-	-	-	616	0	-	

Approach	EB	WB	NB	
HCM Control Delay, s	0.6	0	14.9	
HCM LOS			В	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	WBT
Capacity (veh/h)	691	1074	-	-
HCM Lane V/C Ratio	0.477	0.032	-	-
HCM Control Delay (s)	14.9	8.5	0	-
HCM Lane LOS	В	А	А	-
HCM 95th %tile Q(veh)	2.6	0.1	-	-

4.6

### Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	0	650	20	15	1060	0	30	0	15	0	0	0
Future Vol, veh/h	0	650	20	15	1060	0	30	0	15	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	739	23	17	1205	0	34	0	17	0	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1205	0	0	761	0	0	1989	1989	750	1998	2000	1205
Stage 1	-	-	-	-	-	-	750	750	-	1239	1239	-
Stage 2	-	-	-	-	-	-	1239	1239	-	759	761	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	579	-	-	851	-	-	45	61	411	45	60	224
Stage 1	-	-	-	-	-	-	403	419	-	215	247	-
Stage 2	-	-	-	-	-	-	215	247	-	399	414	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	579	-	-	851	-	-	43	57	411	41	56	224
Mov Cap-2 Maneuver	-	-	-	-	-	-	43	57	-	41	56	-
Stage 1	-	-	-	-	-	-	403	419	-	215	232	-
Stage 2	-	-	-	-	-	_	202	232	-	382	414	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.1	181.2	0
HCM LOS			F	А

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	BLn1
Capacity (veh/h)	61	579	-	-	851	-	-	-
HCM Lane V/C Ratio	0.838	-	-	-	0.02	-	-	-
HCM Control Delay (s)	181.2	0	-	-	9.3	0	-	0
HCM Lane LOS	F	А	-	-	А	А	-	Α
HCM 95th %tile Q(veh)	3.8	0	-	-	0.1	-	-	-

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्भ	1		ર્સ	1	ሻ	<b>∱1</b> }-		ሻ	- <b>†</b> †	
Traffic Volume (veh/h)	280	75	305	25	230	242	485	415	10	55	245	360
Future Volume (veh/h)	280	75	305	25	230	242	485	415	10	55	245	360
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1863	1863	1900	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	318	85	347	28	261	275	551	472	11	62	278	0
Adj No. of Lanes	0	1	1	0	1	1	1	2	0	1	2	0
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	333	89	373	30	284	268	565	1418	33	80	453	0
Arrive On Green	0.24	0.24	0.24	0.17	0.17	0.17	0.32	0.40	0.40	0.05	0.13	0.00
Sat Flow, veh/h	1414	378	1583	180	1674	1583	1774	3536	82	1774	3632	0
Grp Volume(v), veh/h	403	0	347	289	0	275	551	236	247	62	278	0
Grp Sat Flow(s),veh/h/ln	1792	0	1583	1854	0	1583	1774	1770	1848	1774	1770	0
Q Serve(g_s), s	26.8	0.0	26.0	18.6	0.0	20.5	37.2	11.1	11.2	4.2	9.0	0.0
Cycle Q Clear(g_c), s	26.8	0.0	26.0	18.6	0.0	20.5	37.2	11.1	11.2	4.2	9.0	0.0
Prop In Lane	0.79	0.0	1.00	0.10	0.0	1.00	1.00		0.04	1.00	0.0	0.00
Lane Grp Cap(c), veh/h	422	0	373	314	0	268	565	710	741	80	453	0.00
V/C Ratio(X)	0.95	0.00	0.93	0.92	0.00	1.02	0.98	0.33	0.33	0.78	0.61	0.00
Avail Cap(c_a), veh/h	422	0.00	373	314	0.00	268	565	826	863	154	834	0.00
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	45.6	0.0	45.3	49.4	0.0	50.2	40.8	25.0	25.0	57.2	49.9	0.0
Incr Delay (d2), s/veh	32.3	0.0	29.5	31.0	0.0	61.5	31.7	0.3	0.3	14.8	1.4	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	17.1	0.0	14.4	12.2	0.0	13.6	23.2	5.5	5.7	2.4	4.5	0.0
LnGrp Delay(d),s/veh	77.9	0.0	74.8	80.4	0.0	111.8	72.5	25.3	25.3	71.9	51.3	0.0
LnGrp LOS	н.э Е	0.0	74.0 E	00.4 F	0.0	F	72.5 E	23.3 C	23.3 C	н.э Е	D	0.0
Approach Vol, veh/h	<u> </u>	750	<u> </u>	<u> </u>	564	<u> </u>	<u> </u>	1034	0	<u> </u>	340	
Approach Delay, s/veh		76.4			95.7			50.4			55.0	
Approach LOS		70.4 E			95.7 F			50.4 D			55.0 E	
Approach LOS		E			Г			D			E	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	9.9	53.0		33.0	43.0	20.0		25.0				
Change Period (Y+Rc), s	4.5	4.5		4.5	4.5	4.5		4.5				
Max Green Setting (Gmax), s	10.5	56.5		28.5	38.5	28.5		20.5				
Max Q Clear Time (g_c+I1), s	6.2	13.2		28.8	39.2	11.0		22.5				
Green Ext Time (p_c), s	0.0	5.5		0.0	0.0	4.5		0.0				
Intersection Summary												
HCM 2010 Ctrl Delay			67.8									
HCM 2010 LOS			E									
			-									

### Intersection: 1: US 101 SB On/US 101 NB Off & Kenmar Road

Movement	EB	WB	SB
Directions Served	TR	LT	LTR
Maximum Queue (ft)	130	4	251
Average Queue (ft)	59	0	134
95th Queue (ft)	102	2	238
Link Distance (ft)	191	222	214
Upstream Blk Time (%)			8
Queuing Penalty (veh)			0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

### Intersection: 2: US 101 NB Off/US 101 NB One & Kenmar Road/Kenmar Drive

Movement	EB	WB	NB
Directions Served	LT	TR	LTR
Maximum Queue (ft)	178	271	307
Average Queue (ft)	49	69	83
95th Queue (ft)	162	225	231
Link Distance (ft)	222	248	302
Upstream Blk Time (%)	1	1	2
Queuing Penalty (veh)	4	12	0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

### Intersection: 3: Atterberry lane/Eel River Drive & Kenmar Drive

Movement	EB	WB	NB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	318	594	313
Average Queue (ft)	86	106	141
95th Queue (ft)	317	411	322
Link Distance (ft)	248	513	346
Upstream Blk Time (%)	8	3	5
Queuing Penalty (veh)	55	30	0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

694.2

#### Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	0	450	55	410	350	0	0	0	0	595	0	75
Future Vol, veh/h	0	450	55	410	350	0	0	0	0	595	0	75
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	474	58	432	368	0	0	0	0	626	0	79

Major/Minor	Major2			Minor2		
Conflicting Flow All	0	0	0	1232	1232	368
Stage 1	-	-	-	1232	1232	-
Stage 2	-	-	-	0	0	-
Critical Hdwy	4.12	-	-	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	2.218	-	-	3.518	4.018	3.318
Pot Cap-1 Maneuver	-	-	0	~ 154	177	677
Stage 1	-	-	0	~ 217	249	-
Stage 2	-	-	0	-	-	-
Platoon blocked, %		-				
Mov Cap-1 Maneuver	-	-	-	~ 154	177	677
Mov Cap-2 Maneuver	-	-	-	~ 154	177	-
Stage 1	-	-	-	~ 217	249	-
Stage 2	-	-	-	-	-	-
-						

Approach	WB	SB
HCM Control Delay, s		\$ 1481.7
HCMLOS		F

Minor Lane/Major Mvmt	WBL	WBT SBLn1
Capacity (veh/h)	-	- 169
HCM Lane V/C Ratio	-	- 4.173
HCM Control Delay (s)	-	<b>\$</b> 1481.7
HCM Lane LOS	-	- F
HCM 95th %tile Q(veh)	-	- 70.8
Notes		

~: Volume exceeds capacity

\$: Delay exceeds 300s +: Computation Not Defined

\*: All major volume in platoon

#### Intersection

Int Delay, s/veh

23.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	85	960	0	0	705	455	50	0	310	0	0	0
Future Vol, veh/h	85	960	0	0	705	455	50	0	310	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	Free	-	-	Yield	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	89	1011	0	0	742	479	53	0	326	0	0	0

Major/Minor	Major1			Major2			Minor1			
Conflicting Flow All	742	0	-	-	-	0	1931	1931	1011	
Stage 1	-	-	-	-	-	-	1189	1189	-	
Stage 2	-	-	-	-	-	-	742	742	-	
Critical Hdwy	4.12	-	-	-	-	-	6.42	6.52	6.22	
Critical Hdwy Stg 1	-	-	-	-	-	-	5.42	5.52	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	5.42	5.52	-	
Follow-up Hdwy	2.218	-	-	-	-	-	3.518	4.018	3.318	
Pot Cap-1 Maneuver	865	-	0	0	-	0	73	66	~ 291	
Stage 1	-	-	0	0	-	0	289	261	-	
Stage 2	-	-	0	0	-	0	471	422	-	
Platoon blocked, %		-			-					
Mov Cap-1 Maneuver	865	-	-	-	-	-	56	0	~ 291	
Mov Cap-2 Maneuver	-	-	-	-	-	-	56	0	-	
Stage 1	-	-	-	-	-	-	221	0	-	
Stage 2	-	-	-	-	-	-	471	0	-	

Approach	EB	WB	NB	
HCM Control Delay, s	0.8	0	136.9	
HCM LOS			F	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	WBT
Capacity (veh/h)	326	865	-	-
HCM Lane V/C Ratio	1.162	0.103	-	-
HCM Control Delay (s)	136.9	9.6	0	-
HCM Lane LOS	F	А	А	-
HCM 95th %tile Q(veh)	15.7	0.3	-	-
Notes				

~: Volume exceeds capacity

\$: Delay exceeds 300s

+: Computation Not Defined \*: All major volume in platoon

13.1

### Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	0	1225	45	10	1130	0	30	0	15	0	0	0
Future Vol, veh/h	0	1225	45	10	1130	0	30	0	15	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	1289	47	11	1189	0	32	0	16	0	0	0

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1189	0	0	1337	0	0	2524	2524	1313	2532	2548	1189
Stage 1	-	-	-	-	-	-	1313	1313	-	1211	1211	-
Stage 2	-	-	-	-	-	-	1211	1211	-	1321	1337	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	587	-	-	516	-	-	~ 19	28	194	18	27	229
Stage 1	-	-	-	-	-	-	195	228	-	223	255	-
Stage 2	-	-	-	-	-	-	223	255	-	193	222	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	587	-	-	516	-	-	~ 18	26	194	16	25	229
Mov Cap-2 Maneuver	-	-	-	-	-	-	~ 18	26	-	16	25	-
Stage 1	-	-	-	-	-	-	195	228	-	223	239	-
Stage 2	-	-	-	-	-	-	209	239	-	177	222	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0	0.1	\$ 712.7	0
HCM LOS			F	А

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR SI	BLn1
Capacity (veh/h)	26	587	-	-	516	-	-	-
HCM Lane V/C Ratio	1.822	-	-	-	0.02	-	-	-
HCM Control Delay (s)	\$ 712.7	0	-	-	12.1	0	-	0
HCM Lane LOS	F	А	-	-	В	А	-	А
HCM 95th %tile Q(veh)	5.8	0	-	-	0.1	-	-	-
Notes								

~: Volume exceeds capacity

\$: Delay exceeds 300s +: Computation Not Defined \*: All

\*: All major volume in platoon

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्भ	1		र्स	1	ሻ	<b>∱1</b> }-		ሻ	- 11	
Traffic Volume (veh/h)	495	215	535	5	140	145	385	335	15	170	425	615
Future Volume (veh/h)	495	215	535	5	140	145	385	335	15	170	425	615
Number	7	4	14	3	8	18	5	2	12	1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	1900	1863	1863	1900	1863	1863	1863	1863	1900	1863	1863	1900
Adj Flow Rate, veh/h	500	217	540	5	141	146	389	338	15	172	429	0
Adj No. of Lanes	0	1	1	0	1	1	1	2	0	1	2	0
Peak Hour Factor	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	294	128	371	8	226	199	404	1159	51	125	631	0
Arrive On Green	0.23	0.23	0.23	0.13	0.13	0.13	0.23	0.34	0.34	0.07	0.18	0.00
Sat Flow, veh/h	1255	545	1583	64	1796	1583	1774	3453	153	1774	3632	0
Grp Volume(v), veh/h	717	0	540	146	0	146	389	173	180	172	429	0
Grp Sat Flow(s), veh/h/ln	1800	0	1583	1860	0	1583	1774	1770	1836	1774	1770	0
Q Serve(g_s), s	18.0	0.0	18.0	5.7	0.0	6.8	16.7	5.5	5.6	5.4	8.7	0.0
Cycle Q Clear(g_c), s	18.0	0.0	18.0	5.7	0.0	6.8	16.7	5.5	5.6	5.4	8.7	0.0
Prop In Lane	0.70	0.0	1.00	0.03	0.0	1.00	1.00	0.0	0.08	1.00	0.1	0.00
Lane Grp Cap(c), veh/h	422	0	371	234	0	199	404	594	616	125	631	0.00
V/C Ratio(X)	1.70	0.00	1.46	0.62	0.00	0.73	0.96	0.29	0.29	1.38	0.68	0.00
Avail Cap(c_a), veh/h	422	0.00	371	436	0	371	404	705	731	125	852	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	29.4	0.0	29.4	31.9	0.0	32.4	29.4	18.8	18.8	35.7	29.5	0.0
Incr Delay (d2), s/veh	325.4	0.0	219.7	2.7	0.0	5.2	35.1	0.3	0.3	213.0	1.3	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	46.9	0.0	30.4	3.1	0.0	3.3	12.0	2.7	2.8	10.0	4.3	0.0
LnGrp Delay(d),s/veh	354.8	0.0	249.2	34.6	0.0	37.5	64.5	19.1	19.1	248.7	30.9	0.0
LnGrp LOS	554.0 F	0.0	243.2 F	0.+0 C	0.0	57.5 D	04.5 E	B	B	240.7 F	50.5 C	0.0
Approach Vol, veh/h		1257	<u> </u>	0	292	U		742		1	601	
Approach Delay, s/veh		309.4			36.1			42.9			93.2	
Approach LOS		509.4 F			50.1 D			42.9 D			95.2 F	
Approach 203		Г			U			D			Г	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	9.9	30.3		22.5	22.0	18.2		14.2				
Change Period (Y+Rc), s	4.5	4.5		4.5	4.5	4.5		4.5				
Max Green Setting (Gmax), s	5.4	30.6		18.0	17.5	18.5		18.0				
Max Q Clear Time (g_c+l1), s	7.4	7.6		20.0	18.7	10.7		8.8				
Green Ext Time (p_c), s	0.0	5.2		0.0	0.0	3.0		0.8				
Intersection Summary												
HCM 2010 Ctrl Delay			168.5									
HCM 2010 LOS			F									

### Intersection: 1: US 101 SB On/US 101 NB Off & Kenmar Road

Movement	EB	WB	SB
Directions Served	TR	LT	LTR
Maximum Queue (ft)	236	76	258
Average Queue (ft)	211	3	232
95th Queue (ft)	227	45	247
Link Distance (ft)	191	222	214
Upstream Blk Time (%)	100		100
Queuing Penalty (veh)	0		0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

### Intersection: 2: US 101 NB Off/US 101 NB One & Kenmar Road/Kenmar Drive

Movement	EB	WB	NB
Directions Served	LT	TR	LTR
Maximum Queue (ft)	245	225	358
Average Queue (ft)	149	42	315
95th Queue (ft)	296	148	381
Link Distance (ft)	222	248	302
Upstream Blk Time (%)	14	0	93
Queuing Penalty (veh)	144	1	0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

### Intersection: 3: Atterberry lane/Eel River Drive & Kenmar Drive

Movement	EB	WB	NB
Directions Served	LTR	LTR	LTR
Maximum Queue (ft)	401	404	336
Average Queue (ft)	298	61	177
95th Queue (ft)	510	316	380
Link Distance (ft)	248	512	346
Upstream Blk Time (%)	40	2	20
Queuing Penalty (veh)	512	21	0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

# **Cumulative Signal Alternative**

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		ሻሻ	<b>↑</b>					ሻ	4	
Traffic Volume (veh/h)	0	150	40	240	230	0	0	0	0	305	5	35
Future Volume (veh/h)	0	150	40	240	230	0	0	0	0	305	5	35
Number	7	4	14	3	8	18				1	6	16
Initial Q (Qb), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00				1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	0	1845	1900	1845	1845	0				1845	1845	1900
Adj Flow Rate, veh/h	0	158	42	253	242	0				359	0	0
Adj No. of Lanes	0	1	0	2	1	0				2	1	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95				0.95	0.95	0.95
Percent Heavy Veh, %	0	3	3	3	3	0				3	3	3
Cap, veh/h	0	220	58	1843	1390	0				468	246	0
Arrive On Green	0.00	0.16	0.16	0.90	1.00	0.00				0.13	0.00	0.00
Sat Flow, veh/h	0	1405	374	3408	1845	0				3514	1845	0
Grp Volume(v), veh/h	0	0	200	253	242	0				359	0	0
Grp Sat Flow(s), veh/h/ln	0	0	1779	1704	1845	0				1757	1845	0
Q Serve(g_s), s	0.0	0.0	9.6	0.7	0.0	0.0				8.9	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	9.6	0.7	0.0	0.0				8.9	0.0	0.0
Prop In Lane	0.00	0.0	0.21	1.00	0.0	0.00				1.00	0.0	0.00
Lane Grp Cap(c), veh/h	0.00	0	278	1843	1390	0.00				468	246	0.00
V/C Ratio(X)	0.00	0.00	0.72	0.14	0.17	0.00				0.77	0.00	0.00
Avail Cap(c_a), veh/h	0.00	0.00	579	1843	1390	0.00				996	523	0.00
HCM Platoon Ratio	1.00	1.00	1.00	1.67	1.67	1.00				1.00	1.00	1.00
Upstream Filter(I)	0.00	0.00	1.00	0.99	0.99	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	36.1	2.0	0.0	0.0				37.7	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	3.5	0.0	0.3	0.0				2.7	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	0.0	5.0	0.4	0.0	0.0				4.5	0.0	0.0
LnGrp Delay(d),s/veh	0.0	0.0	39.6	2.1	0.3	0.0				40.3	0.0	0.0
LnGrp LOS	0.0	0.0	00.0 D	A	A	0.0				чо.о D	0.0	0.0
Approach Vol, veh/h		200			495						359	
Approach Delay, s/veh		39.6			1.2						40.3	
		59.0 D			A						40.5 D	
Approach LOS											U	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs			3	4		6		8				
Phs Duration (G+Y+Rc), s			53.8	19.2		17.1		72.9				
Change Period (Y+Rc), s			5.1	5.1		5.1		5.1				
Max Green Setting (Gmax), s			19.9	29.3		25.5		47.7				
Max Q Clear Time (g_c+11), s Green Ext Time (p_c), s			2.7 0.8	11.6 2.4		10.9 1.1		2.0 2.9				
u = 7.			0.0	۲.2		1.1		2.5				
Intersection Summary			04.0									
HCM 2010 Ctrl Delay			21.8									
HCM 2010 LOS			С									
Notes												

5:00 pm Baseline

Synchro 9 Report Page 1

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	۲.	<b>†</b> †			<b>^</b>	1		ર્ન	1				
Traffic Volume (veh/h)	30	425	0	0	430	660	40	5	245	0	0	0	
Future Volume (veh/h)	30	425	0	0	430	660	40	5	245	0	0	0	
Number	7	4	14	3	8	18	5	2	12				
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0				
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00				
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Adj Sat Flow, veh/h/ln	1863	1863	0	0	1863	1863	1900	1863	1863				
Adj Flow Rate, veh/h	34	483	0	0	489	0	45	6	278				
Adj No. of Lanes	1	2	0	0	2	1	0	1	1				
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88				
Percent Heavy Veh, %	2	2	0	0	2	2	2	2	2				
Cap, veh/h	56	2428	0	0	2138	956	316	42	318				
Arrive On Green	0.03	0.69	0.00	0.00	1.00	0.00	0.20	0.20	0.20				
Sat Flow, veh/h	1774	3632	0	0	3632	1583	1574	210	1583				
Grp Volume(v), veh/h	34	483	0	0	489	0	51	0	278				
Grp Sat Flow(s),veh/h/l	n1774	1770	0	0	1770	1583	1784	0	1583				
Q Serve(g_s), s	1.7	4.5	0.0	0.0	0.0	0.0	2.1	0.0	15.3				
Cycle Q Clear(g_c), s	1.7	4.5	0.0	0.0	0.0	0.0	2.1	0.0	15.3				
Prop In Lane	1.00		0.00	0.00		1.00	0.88		1.00				
Lane Grp Cap(c), veh/h	n 56	2428	0	0	2138	956	358	0	318				
V/C Ratio(X)	0.60	0.20	0.00	0.00	0.23	0.00	0.14	0.00	0.87				
Avail Cap(c_a), veh/h	187	2428	0	0	2138	956	494	0	438				
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.67	1.67	1.00	1.00	1.00				
Upstream Filter(I)	0.72	0.72	0.00	0.00	0.93	0.00	1.00	0.00	1.00				
Uniform Delay (d), s/vel	h 43.0	5.1	0.0	0.0	0.0	0.0	29.6	0.0	34.9				
Incr Delay (d2), s/veh	7.2	0.1	0.0	0.0	0.2	0.0	0.2	0.0	13.6				
Initial Q Delay(d3),s/vel	n 0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
%ile BackOfQ(50%),ve	h/lr0.9	2.2	0.0	0.0	0.1	0.0	1.1	0.0	7.9				
LnGrp Delay(d),s/veh	50.3	5.3	0.0	0.0	0.2	0.0	29.8	0.0	48.5				
LnGrp LOS	D	А			А		С		D				
Approach Vol, veh/h		517			489			329					
Approach Delay, s/veh		8.2			0.2			45.6					
Approach LOS		А			Α			D					
Timer	1	2	3	4	5	6	7	8					
Assigned Phs		2		4			7	8					
Phs Duration (G+Y+Rc	), s	23.2		66.8			7.4	59.5					
Change Period (Y+Rc),		5.1		5.1			4.5	5.1					
Max Green Setting (Gr		24.9		54.9			9.5	40.9					
Max Q Clear Time (g c		17.3		6.5			3.7	2.0					
Green Ext Time (p_c),		0.8		8.2			0.0	8.0					
Intersection Summary													
HCM 2010 Ctrl Delay			14.5										
HCM 2010 LOS			В										

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	٦	A		ኘ	<b>≜</b> †₽			4			4		
Traffic Volume (veh/h)	0	650	20	15	1060	0	30	0	15	0	0	0	
Future Volume (veh/h)	0	650	20	15	1060	0	30	0	15	0	0	0	
Number	5	2	12	1	6	16	3	8	18	7	4	14	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00	Ŭ	1.00	1.00	Ŭ	1.00	1.00	Ū	1.00	1.00	Ū	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Adj Sat Flow, veh/h/ln	1863	1863	1900	1863	1863	1900	1900	1863	1900	1900	1863	1900	
Adj Flow Rate, veh/h	0	739	23	17	1205	0	34	0	17	0	0	0	
Adj No. of Lanes	1	2	0	1	2	0	0	1	0	0	1	Ũ	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	
Percent Heavy Veh, %	2	2	2	0.00	2	0.00	2	0.00	0.00	2	2	2	
Cap, veh/h	2	2744	85	34	3020	0	46	0	23	0	2	0	
Arrive On Green	0.00	1.00	1.00	0.02	0.85	0.00	0.04	0.00	0.04	0.00	0.00	0.00	
Sat Flow, veh/h	1774	3504	100	1774	3632	0.00	1137	0.00	0.04 569	0.00	1863	0.00	
Grp Volume(v), veh/h	0	373	389	17	1205	0	51	0	0	0	0	0	
Grp Sat Flow(s),veh/h/li		1770	1844	1774	1770	0	1706	0	0	0	1863	0	
Q Serve(g_s), s	0.0	0.0	0.0	0.9	6.8	0.0	2.7	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear(g_c), s	0.0	0.0	0.0	0.9	6.8	0.0	2.7	0.0	0.0	0.0	0.0	0.0	
Prop In Lane	1.00		0.06	1.00		0.00	0.67		0.33	0.00		0.00	
Lane Grp Cap(c), veh/h		1386	1443	34	3020	0	68	0	0	0	2	0	
V/C Ratio(X)	0.00	0.27	0.27	0.50	0.40	0.00	0.75	0.00	0.00	0.00	0.00	0.00	
Avail Cap(c_a), veh/h	99	1386	1443	106	3020	0	104	0	0	0	103	0	
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	0.00	0.95	0.95	0.43	0.43	0.00	1.00	0.00	0.00	0.00	0.00	0.00	
Uniform Delay (d), s/vel	h 0.0	0.0	0.0	43.7	1.5	0.0	42.7	0.0	0.0	0.0	0.0	0.0	
Incr Delay (d2), s/veh	0.0	0.5	0.4	4.8	0.2	0.0	14.9	0.0	0.0	0.0	0.0	0.0	
Initial Q Delay(d3),s/vel	n 0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),vel	h/lr0.0	0.2	0.2	0.5	3.3	0.0	1.5	0.0	0.0	0.0	0.0	0.0	
LnGrp Delay(d),s/veh	0.0	0.5	0.4	48.5	1.6	0.0	57.6	0.0	0.0	0.0	0.0	0.0	
LnGrp LOS		А	А	D	А		Е						
Approach Vol, veh/h		762			1222			51			0		
Approach Delay, s/veh		0.4			2.3			57.6			0.0		
Approach LOS		A			Α			E			0.0		
	1		2	Λ		6	7						
Timer	1	2	3	4	5		1	8					
Assigned Phs		2		4	5	6		8					
Phs Duration (G+Y+Rc)		75.6		0.0	0.0	81.9		8.1					
Change Period (Y+Rc),		5.1		4.5	4.5	* 5.1		4.5					
Max Green Setting (Gr		55.4		5.0	5.0	* 57		5.5					
Max Q Clear Time (g_c		2.0		0.0	0.0	8.8		4.7					
Green Ext Time (p_c), s	s 0.0	23.7		0.0	0.0	22.7		0.0					
Intersection Summary													
HCM 2010 Ctrl Delay			3.0										
HCM 2010 LOS			А										
Notes													

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	ሻሻ		1	٦		1	ኘኘ	<b>≜</b> †₽		٦	††	1	
Traffic Volume (veh/h)	280	75	305	25	230	242	485	415	10	55	245	360	
Future Volume (veh/h)	280	75	305	25	230	242	485	415	10	55	245	360	
Number	7	4	14	3	8	18	5	2	12	1	6	16	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Adj Sat Flow, veh/h/ln	1863	1863	1863	1863	1863	1863	1863	1863	1900	1863	1863	1863	
Adj Flow Rate, veh/h	318	85	347	28	261	275	551	472	11	62	278	409	
Adj No. of Lanes	2	1	1	1	1	1	2	2	0	1	2	1	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2	
Cap, veh/h	413	436	672	119	337	286	654	1450	34	86	950	425	
Arrive On Green	0.12	0.23	0.23	0.07	0.18	0.18	0.19	0.41	0.41	0.05	0.27	0.27	
Sat Flow, veh/h	3442	1863	1583	1774	1863	1583	3442	3536	82	1774	3539	1583	
Grp Volume(v), veh/h	318	85	347	28	261	275	551	236	247	62	278	409	
Grp Sat Flow(s), veh/h/li		1863	1583	1774	1863	1583	1721	1770	1848	1774	1770	1583	
Q Serve(g_s), s	6.7	2.7	12.1	1.1	10.0	10.2	11.6	6.8	6.8	2.6	4.7	12.8	
Cycle Q Clear(g_c), s	6.7	2.7	12.1	1.1	10.0	10.2	11.6	6.8	6.8	2.6	4.7	12.8	
Prop In Lane	1.00	2.1	1.00	1.00	10.0	1.00	1.00	0.0	0.04	1.00	۰.۳	1.00	
Lane Grp Cap(c), veh/h		436	672	119	337	286	654	726	758	86	950	425	
V/C Ratio(X)	0.77	0.19	0.52	0.24	0.77	0.96	0.84	0.33	0.33	0.72	0.29	0.96	
Avail Cap(c_a), veh/h	437	543	762	135	448	381	759	0.33 844	882	209	1324	0.90 592	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Uniform Delay (d), s/vel		23.0	15.9	33.1	29.2	19.2	29.2	15.0	15.0	35.1	21.7	12.2	
Incr Delay (d2), s/veh	7.7	23.0	0.6	1.0	29.2 6.0	31.2	7.6	0.3	0.2	10.8	0.2	23.4	
Initial Q Delay(d3),s/vel		0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.2	23.4	
%ile BackOfQ(50%),vel		1.4	5.3	0.0	5.7	7.1	6.2	3.3	3.5	1.5	2.3	0.0 8.8	
LnGrp Delay(d),s/veh	39.7	23.2	16.5	34.1	35.2	50.4	36.8	15.3	15.3	45.9	2.3	35.6	
LnGrp LOS	59.7 D	23.2 C	10.5 B	04.1 C	55.2 D	50.4 D	30.0 D	15.5 B	15.5 B	45.9 D	21.9 C	55.0 D	
	U		D	0		U	U		D	U		D	
Approach Vol, veh/h		750			564			1034			749		
Approach Delay, s/veh		27.1			42.5			26.7			31.4		
Approach LOS		С			D			С			С		
Timer	1	2	3	4	5	6	7	8					
Assigned Phs	1	2	3	4	5	6	7	8					
Phs Duration (G+Y+Rc)	), s8.1	35.2	9.5	22.0	18.7	24.6	13.5	18.0					
Change Period (Y+Rc),		4.5	4.5	4.5	4.5	4.5	4.5	4.5					
Max Green Setting (Gr		35.7	5.7	21.8	16.5	28.0	9.5	18.0					
Max Q Clear Time (g_c		8.8	3.1	14.1	13.6	14.8	8.7	12.2					
Green Ext Time (p_c), s		7.0	0.0	1.9	0.7	5.3	0.3	1.3					
Intersection Summary													
HCM 2010 Ctrl Delay			30.8										
HCM 2010 LOS			С										
			Ŭ										

### Intersection: 1: US 101 SB On/US 101 NB Off & Kenmar Road

MovementEBWBWBSBSBDirections ServedTRLLTLL
Directions Served TR L L T L LTR
Maximum Queue (ft) 182 57 96 160 168 207
Average Queue (ft) 111 11 45 42 93 101
95th Queue (ft) 178 35 82 107 154 180
Link Distance (ft) 169 213 213 256
Upstream Blk Time (%) 2 0 0
Queuing Penalty (veh) 0 0 0
Storage Bay Dist (ft) 80 120
Storage Blk Time (%) 0 1 4 5
Queuing Penalty (veh) 0 1 8 8

### Intersection: 2: US 101 NB Off/US 101 NB One & Kenmar Road/Kenmar Drive

Movement	EB	EB	EB	WB	WB	WB	NB	NB
Directions Served	L	Т	Т	Т	Т	R	LT	R
Maximum Queue (ft)	122	145	137	97	112	204	80	109
Average Queue (ft)	24	55	44	26	32	11	28	45
95th Queue (ft)	67	120	103	67	83	84	64	81
Link Distance (ft)		213	213	236	236	236	289	
Upstream Blk Time (%)						0		
Queuing Penalty (veh)						0		
Storage Bay Dist (ft)	75							150
Storage Blk Time (%)	0	2						
Queuing Penalty (veh)	0	1						

### Intersection: 3: Atterberry lane/Eel River Drive & Kenmar Drive

Movement Lane Configurations Traffic Volume (veh/h) Future Volume (veh/h) Number Initial Q (Qb), veh Ped-Bike Adj(A_pbT) Dadking Dug Adji	EBL 0 0 7 0 1.00 1.00 1.00 0 0	EBT 450 450 4 0 1.00	EBR 55 55 14 0 1.00	WBL 410 410 3	WBT 1 350 350	WBR 0	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (veh/h) Future Volume (veh/h) Number Initial Q (Qb), veh Ped-Bike Adj(A_pbT)	0 7 1.00 1.00 0	450 450 4 0	55 14 0	410 410 3	350	0				×	Δ.	
Future Volume (veh/h) Number Initial Q (Qb), veh Ped-Bike Adj(A_pbT)	0 7 1.00 1.00 0	450 4 0	55 14 0	410 410 3		Ο				-		
Number Initial Q (Qb), veh Ped-Bike Adj(A_pbT)	7 0 1.00 1.00 0	4 0	14 0	3	350	0	0	0	0	595	0	75
Initial Q (Qb), veh Ped-Bike Adj(A_pbT)	0 1.00 1.00 0	0	0		000	0	0	0	0	595	0	75
Ped-Bike Adj(A_pbT)	1.00 1.00 0				8	18				1	6	16
Ped-Bike Adj(A_pbT)	1.00 0	1 00	1 00	0	0	0				0	0	0
Derking Due Ad	0	1 00	1.00	1.00		1.00				1.00		1.00
Parking Bus, Adj		1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Adj Sat Flow, veh/h/ln	٥	1845	1900	1845	1845	0				1845	1845	1900
Adj Flow Rate, veh/h	0	474	58	432	368	0				700	0	0
Adj No. of Lanes	0	1	0	2	1	0				2	1	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95				0.95	0.95	0.95
Percent Heavy Veh, %	0	3	3	3	3	0				3	3	3
Cap, veh/h	0	512	63	960	1209	0				812	426	0
Arrive On Green	0.00	0.32	0.32	0.47	1.00	0.00				0.23	0.00	0.00
Sat Flow, veh/h	0	1613	197	3408	1845	0				3514	1845	0
Grp Volume(v), veh/h	0	0	532	432	368	0				700	0	0
Grp Sat Flow(s), veh/h/ln	0	0	1810	1704	1845	0				1757	1845	0
Q Serve(g_s), s	0.0	0.0	25.6	7.7	0.0	0.0				17.2	0.0	0.0
Cycle Q Clear(g_c), s	0.0	0.0	25.6	7.7	0.0	0.0				17.2	0.0	0.0
Prop In Lane	0.00	0.0	0.11	1.00	0.0	0.0				1.00	0.0	0.0
•	0.00	٥	574	960	1209	0.00				812	426	0.00
Lane Grp Cap(c), veh/h	0.00	0 0.00	0.93	0.45	0.30	0.00				0.86	420	0.00
V/C Ratio(X)	0.00	0.00	0.93 589	0.45 960	1209	0.00				0.00 996	523	0.00
Avail Cap(c_a), veh/h												
HCM Platoon Ratio	1.00	1.00	1.00	1.67	1.67	1.00				1.00	1.00	1.00
Upstream Filter(I)	0.00	0.00	1.00	0.93	0.93	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	0.0	0.0	29.7	19.1	0.0	0.0				33.2	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	20.6	0.3	0.6	0.0				6.7	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.0	0.0	16.0	3.6	0.2	0.0				9.1	0.0	0.0
LnGrp Delay(d),s/veh	0.0	0.0	50.4	19.5	0.6	0.0				39.9	0.0	0.0
LnGrp LOS			D	В	Α					D		
Approach Vol, veh/h		532			800						700	
Approach Delay, s/veh		50.4			10.8						39.9	
Approach LOS		D			В						D	
Timer	1	2	3	4	5	6	7	8				
Assigned Phs			3	4		6		8				
Phs Duration (G+Y+Rc), s			30.5	33.7		25.9		64.1				
Change Period (Y+Rc), s			5.1	5.1		5.1		5.1				
Max Green Setting (Gmax), s			19.9	29.3		25.5		47.7				
Max Q Clear Time (g_c+l1), s			9.7	27.6		19.2		2.0				
Green Ext Time (p_c), s			1.2	1.0		1.6		7.1				
Intersection Summary												
HCM 2010 Ctrl Delay			31.2									
HCM 2010 LOS			С									
Notes												

5:00 pm Baseline

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	٦	<b>†</b> †			<u></u>	1		ર્સ	1				
Traffic Volume (veh/h)	85	960	0	0	705	455	50	0	310	0	0	0	
Future Volume (veh/h)	85	960	0	0	705	455	50	0	310	0	0	0	
Number	7	4	14	3	8	18	5	2	12				
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0				
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00				
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Adj Sat Flow, veh/h/ln	1845	1845	0	0	1845	1845	1900	1845	1845				
Adj Flow Rate, veh/h	89	1011	0	0	742	0	53	0	326				
Adj No. of Lanes	1	2	0	0	2	1	0	1	1				
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95				
Percent Heavy Veh, %	3	3	0	0	3	3	3	3	3				
Cap, veh/h	114	2297	0	0	1895	848	406	0	363				
Arrive On Green	0.06	0.66	0.00	0.00	1.00	0.00	0.23	0.00	0.23				
Sat Flow, veh/h	1757	3597	0	0	3597	1568	1757	0	1568				
Grp Volume(v), veh/h	89	1011	0	0	742	0	53	0	326				
Grp Sat Flow(s),veh/h/li	n1757	1752	0	0	1752	1568	1757	0	1568				
Q Serve(g_s), s	4.5	12.6	0.0	0.0	0.0	0.0	2.2	0.0	18.2				
Cycle Q Clear(g_c), s	4.5	12.6	0.0	0.0	0.0	0.0	2.2	0.0	18.2				
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00				
Lane Grp Cap(c), veh/h	114	2297	0	0	1895	848	406	0	363				
V/C Ratio(X)	0.78	0.44	0.00	0.00	0.39	0.00	0.13	0.00	0.90				
Avail Cap(c_a), veh/h	185	2297	0	0	1895	848	486	0	434				
HCM Platoon Ratio	1.00	1.00	1.00	1.00	2.00	2.00	1.00	1.00	1.00				
Upstream Filter(I)	0.34	0.34	0.00	0.00	0.93	0.00	1.00	0.00	1.00				
Uniform Delay (d), s/vel	h 41.5	7.5	0.0	0.0	0.0	0.0	27.4	0.0	33.6				
Incr Delay (d2), s/veh	4.0	0.2	0.0	0.0	0.6	0.0	0.1	0.0	19.0				
Initial Q Delay(d3),s/vel	n 0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
%ile BackOfQ(50%),vel	h/lr2.3	6.1	0.0	0.0	0.1	0.0	1.1	0.0	9.8				
LnGrp Delay(d),s/veh	45.4	7.7	0.0	0.0	0.6	0.0	27.6	0.0	52.6				
LnGrp LOS	D	Α			Α		С		D				
Approach Vol, veh/h		1100			742			379					
Approach Delay, s/veh		10.8			0.6			49.1					
Approach LOS		В			А			D					
Timer	1	2	3	4	5	6	7	8					
Assigned Phs		2		4			7	8					
Phs Duration (G+Y+Rc)	), s	25.9		64.1			10.3	53.8					
Change Period (Y+Rc),	S	5.1		5.1			4.5	5.1					
Max Green Setting (Gm	nax), s	24.9		54.9			9.5	40.9					
Max Q Clear Time (g_c	+l1), s	20.2		14.6			6.5	2.0					
Green Ext Time (p_c), s	5	0.7		18.5			0.0	18.2					
Intersection Summary													
HCM 2010 Ctrl Delay			13.9										
HCM 2010 LOS			В										

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	٦	_ <b>≜</b> †≱		5	<b>≜</b> †₽			4			4		
Traffic Volume (veh/h)	0	1225	45	10	1130	0	30	0	15	0	0	0	
Future Volume (veh/h)	0	1225	45	10	1130	0	30	0	15	0	0	0	
Number	5	2	12	1	6	16	3	8	18	7	4	14	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00	Ū	1.00	1.00	·	1.00	1.00	Ū	1.00	1.00	·	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
<b>v</b> ,	1845	1845	1900	1845	1845	1900	1900	1845	1900	1900	1845	1900	
Adj Flow Rate, veh/h	0	1289	47	11	1189	0	32	0	16	0	0	0	
Adj No. of Lanes	1	2	0	1	2	0	0	1	0	0	1	0	
	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Percent Heavy Veh, %	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
	3 2	د 1421	52	د 687	3014	0 0	3 44	0 0	22	0 0	2	3 0	
Cap, veh/h	0.00	0.82	52 0.82	0.39	0.86	0.00	44 0.04	0.00	0.04	0.00	2 0.00	0.00	
· · · · · · · · · · · · · · · · · · ·	1757	3449	126	1757	3597	0	1126	0	563	0	1845	0	
Grp Volume(v), veh/h	0	654	682	11	1189	0	48	0	0	0	0	0	
Grp Sat Flow(s),veh/h/ln		1752	1822	1757	1752	0	1689	0	0	0	1845	0	
Q Serve(g_s), s	0.0	23.3	23.5	0.3	6.5	0.0	2.5	0.0	0.0	0.0	0.0	0.0	
Cycle Q Clear(g_c), s	0.0	23.3	23.5	0.3	6.5	0.0	2.5	0.0	0.0	0.0	0.0	0.0	
Prop In Lane	1.00		0.07	1.00		0.00	0.67		0.33	0.00		0.00	
Lane Grp Cap(c), veh/h	2	722	751	687	3014	0	66	0	0	0	2	0	
V/C Ratio(X)	0.00	0.91	0.91	0.02	0.39	0.00	0.73	0.00	0.00	0.00	0.00	0.00	
Avail Cap(c_a), veh/h	98	1079	1122	687	3014	0	103	0	0	0	102	0	
HCM Platoon Ratio	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	0.00	0.85	0.85	0.54	0.54	0.00	1.00	0.00	0.00	0.00	0.00	0.00	
Uniform Delay (d), s/veh	0.0	6.7	6.7	16.8	1.3	0.0	42.8	0.0	0.0	0.0	0.0	0.0	
Incr Delay (d2), s/veh	0.0	15.1	14.8	0.0	0.2	0.0	14.5	0.0	0.0	0.0	0.0	0.0	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh		13.2	13.7	0.2	3.1	0.0	1.5	0.0	0.0	0.0	0.0	0.0	
LnGrp Delay(d),s/veh	0.0	21.8	21.5	16.8	1.5	0.0	57.3	0.0	0.0	0.0	0.0	0.0	
LnGrp LOS		C	C	B	A		E						
Approach Vol, veh/h		1336	<u> </u>		1200			48			0		
Approach Delay, s/veh		21.6			1200			57.3			0.0		
		21.0 C			A			57.5 E			0.0		
Approach LOS		U			A								
Timer	1	2	3	4	5	6	7	8					
Assigned Phs	1	2		4	5	6		8					
Phs Duration (G+Y+Rc),	<b>3</b> 9.8	42.2		0.0	0.0	82.0		8.0					
Change Period (Y+Rc),		5.1		4.5	4.5	* 4.6		4.5					
Max Green Setting (Gma		55.4		5.0	5.0	* 57		5.5					
Max Q Clear Time (g_c+		25.5		0.0	0.0	8.5		4.5					
Green Ext Time (p_c), s		11.6		0.0	0.0	12.2		0.0					
Intersection Summary													
HCM 2010 Ctrl Delay			13.0										
HCM 2010 LOS			B										
			-										
Notes													

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	ኘኘ	↑	1	۲.	<b>↑</b>	1	ሻሻ	<b>∱</b> î≽		۲.	<b>†</b> †	1	
Traffic Volume (veh/h)	495	215	535	5	140	145	385	335	15	170	425	615	
Future Volume (veh/h)	495	215	535	5	140	145	385	335	15	170	425	615	
Number	7	4	14	3	8	18	5	2	12	1	6	16	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Adj Sat Flow, veh/h/ln	1845	1845	1845	1845	1845	1845	1845	1845	1900	1845	1845	1845	
Adj Flow Rate, veh/h	500	217	540	5	141	146	389	338	15	172	429	621	
Adj No. of Lanes	2	1	1	1	1	1	2	2	0	1	2	1	
Peak Hour Factor	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	0.99	
Percent Heavy Veh, %	3	3	3	3	3	3	3	3	3	3	3	3	
Cap, veh/h	865	548	695	125	212	180	498	868	38	214	804	757	
Arrive On Green	0.25	0.30	0.30	0.07	0.11	0.11	0.15	0.25	0.25	0.12	0.23	0.23	
Sat Flow, veh/h	3408	1845	1568	1757	1845	1568	3408	3419	151	1757	3505	1568	
Grp Volume(v), veh/h	500	217	540	5	141	146	389	173	180	172	429	621	
Grp Sat Flow(s), veh/h/lr		1845	1568	1757	1845	1568	1704	1752	1818	1757	1752	1568	
Q Serve(g_s), s	9.0	6.6	20.6	0.2	5.2	4.6	7.7	5.7	5.8	6.7	7.6	9.7	
Cycle Q Clear(g_c), s	9.0	6.6	20.6	0.2	5.2	4.6	7.7	5.7	5.8	6.7	7.6	9.7	
Prop In Lane	1.00	0.0	1.00	1.00	•	1.00	1.00	•	0.08	1.00		1.00	
Lane Grp Cap(c), veh/h		548	695	125	212	180	498	445	462	214	804	757	
V/C Ratio(X)	0.58	0.40	0.78	0.04	0.67	0.81	0.78	0.39	0.39	0.81	0.53	0.82	
Avail Cap(c_a), veh/h	1013	548	695	450	472	401	659	511	530	315	972	833	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Uniform Delay (d), s/veł		19.7	16.6	30.4	29.8	15.7	28.9	21.7	21.7	30.1	23.8	4.4	
Incr Delay (d2), s/veh	0.6	0.5	5.5	0.1	3.6	8.5	4.4	0.6	0.5	9.1	0.6	6.1	
Initial Q Delay(d3),s/veh		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),vel		3.4	9.9	0.1	2.8	2.9	3.9	2.9	3.0	3.8	3.7	5.4	
LnGrp Delay(d),s/veh	23.6	20.1	22.1	30.5	33.4	24.1	33.3	22.3	22.3	39.2	24.3	10.5	
LnGrp LOS	С	С	С	С	С	С	С	С	С	D	С	В	
Approach Vol, veh/h		1257			292			742			1222		
Approach Delay, s/veh		22.4			28.7			28.1			19.4		
Approach LOS		22.4 C			C			C			B		
	4		2	4		C	7						
Timer Assigned Phs	1	2	<u>3</u> 3	4	5 5	6 6	7	8 8					
Phs Duration (G+Y+Rc)	1 43 0	2 22.3	3 9.5	4 25.4	с 14.8	0 20.6	22.3	o 12.6					
		4.5	9.5 4.5		4.5	20.6 4.5	22.3 4.5	4.5					
Change Period (Y+Rc),				4.5			4.5 20.9						
Max Green Setting (Gm		20.5	18.0 2.2	20.9 22.6	13.6 9.7	19.5	20.9	18.0 7.2					
Max Q Clear Time (g_c		7.8				11.7							
Green Ext Time (p_c), s	5 U.Z	6.1	0.0	0.0	0.5	4.4	4.0	0.9					
Intersection Summary			06.4										
HCM 2010 Ctrl Delay			23.1										
HCM 2010 LOS			С										

### Intersection: 1: US 101 SB On/US 101 NB Off & Kenmar Road

Movement	EB	WB	WB	WB	SB	SB
wovernent	ED	VVD	VVD	VVD	30	SD
Directions Served	TR	L	L	Т	L	LTR
Maximum Queue (ft)	342	140	225	220	273	296
Average Queue (ft)	275	87	111	103	173	170
95th Queue (ft)	373	155	187	199	261	259
Link Distance (ft)	303		214	214		446
Upstream Blk Time (%)	18		1	0		
Queuing Penalty (veh)	0		3	1		
Storage Bay Dist (ft)		80			275	
Storage Blk Time (%)		4	16		0	0
Queuing Penalty (veh)		9	32		0	1

### Intersection: 2: US 101 NB Off/US 101 NB One & Kenmar Road/Kenmar Drive

Movement	EB	EB	EB	WB	WB	WB	NB	NB
Directions Served	L	Т	Т	Т	Т	R	LT	R
Maximum Queue (ft)	123	220	209	193	196	105	102	226
Average Queue (ft)	51	90	79	86	72	6	31	83
95th Queue (ft)	103	188	160	173	149	64	76	163
Link Distance (ft)		214	214	236	236		289	
Upstream Blk Time (%)		1	0		0			
Queuing Penalty (veh)		3	0		0			
Storage Bay Dist (ft)	75					150		150
Storage Blk Time (%)	3	5			1	0		2
Queuing Penalty (veh)	14	4			3	1		1

### Intersection: 3: Atterberry lane/Eel River Drive & Kenmar Drive

NA						ND
Movement	EB	EB	WB	WB	WB	NB
Directions Served	Т	TR	L	Т	TR	LTR
Maximum Queue (ft)	240	223	36	73	165	64
Average Queue (ft)	51	39	7	22	49	29
95th Queue (ft)	149	130	24	64	129	60
Link Distance (ft)	236	236		498	498	322
Upstream Blk Time (%)	0	0				
Queuing Penalty (veh)	2	1				
Storage Bay Dist (ft)			75			
Storage Blk Time (%)	1			0		
Queuing Penalty (veh)	0			0		

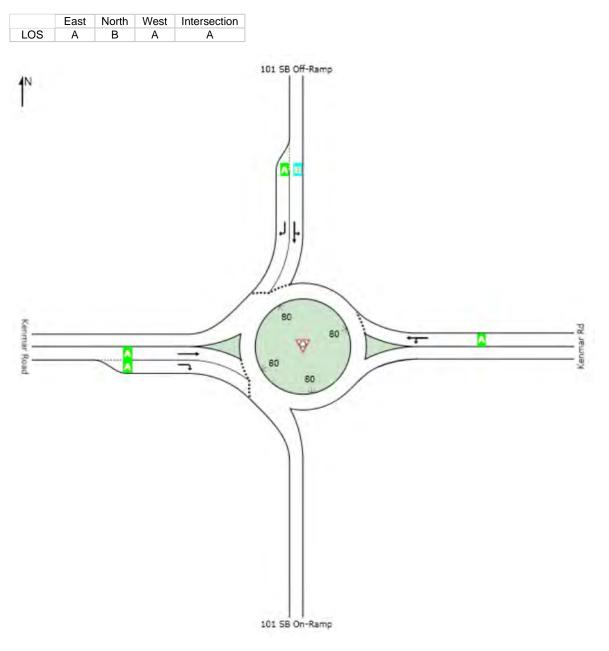
# **Cumulative Roundabout Alternative**

## LEVEL OF SERVICE

# 😵 Site: Kenmar Road/ SB Ramps

Kenmar Road Interchange Roundabout Concept - Option 1a, 1b, & 2 Cumulative AM Roundabout

#### **All Movement Classes**



Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

### LANE SUMMARY

# 😵 Site: Kenmar Road/ SB Ramps

Kenmar Road Interchange Roundabout Concept - Option 1a, 1b, & 2 Cumulative AM Roundabout

Lane Use a	nd Perforr	nance	<b>;</b>										
	Demand F Total	Flows HV	Cap.	Deg. Satn	Lane Util.	Average Delay	Level of Service	95% Back o Veh	f Queue Dist	Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	veh/h	%	veh/h	v/c	%	sec			ft		ft	%	%
East: Kenma	r Rd												
Lane 1 <sup>d</sup>	500	3.0	1377	0.363	100	6.9	LOS A	0.0	0.0	Full	1600	0.0	0.0
Approach	500	3.0		0.363		6.9	LOS A	0.0	0.0				
North: 101 S	B Off-Ramp												
Lane 1 <sup>d</sup>	326	3.0	1196	0.273	100	11.7	LOS B	1.6	40.9	Full	1600	0.0	0.0
Lane 2	37	3.0	786	0.047	100	7.4	LOS A	0.2	5.5	Short	200	0.0	NA
Approach	363	3.0		0.273		11.2	LOS B	1.6	40.9				
West: Kenma	ar Road												
Lane 1 <sup>d</sup>	158	3.0	1073	0.147	100	6.7	LOS A	0.9	22.9	Full	1600	0.0	0.0
Lane 2	42	3.0	719	0.059	100	7.9	LOS A	0.3	7.6	Short	200	0.0	NA
Approach	200	3.0		0.147		7.0	LOS A	0.9	22.9				
Intersection	1063	3.0		0.363		8.4	LOS A	1.6	40.9				

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

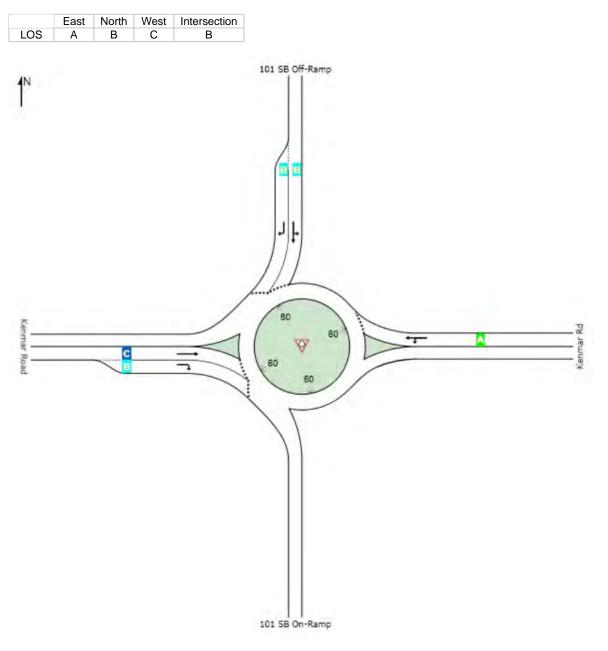
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# LEVEL OF SERVICE

# 😵 Site: Kenmar Road/SB Ramps PM

Kenmar Road Interchange Roundabout Concept - Option 1a & 1b Cumulative PM Roundabout

#### **All Movement Classes**



Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

### LANE SUMMARY

# 🕅 Site: Kenmar Road/SB Ramps PM

Kenmar Road Interchange Roundabout Concept - Option 1a & 1b Cumulative PM Roundabout

Lane Use a	Lane Use and Performance													
	Demand F		0	Deg.	Lane	Average	Level of	95% Back o		Lane	Lane	Cap.	Prob.	
	Total veh/h	HV	Cap.	Satn v/c	Util. %	Delay	Service	Veh	Dist	Config	Length	Adj. %	Block.	
East: Kenma		%	veh/h	V/C	70	Sec	_		ft	_	ft	70	%	
Lane 1 <sup>d</sup>	800	3.0	1377	0.581	100	7.1	LOS A	0.0	0.0	Full	1600	0.0	0.0	
Approach	800	3.0		0.581		7.1	LOS A	0.0	0.0					
North: 101 St	B Off-Ramp													
Lane 1 <sup>d</sup>	627	3.0	985	0.637	100	19.0	LOS B	7.3	187.2	Full	1600	0.0	0.0	
Lane 2	79	3.0	661	0.119	100	10.2	LOS B	0.6	16.0	Short	200	0.0	NA	
Approach	706	3.0		0.637		18.0	LOS B	7.3	187.2					
West: Kenma	ar Road													
Lane 1 <sup>d</sup>	474	3.0	607	0.781	100	31.0	LOS C	12.7	324.1	Full	1600	0.0	0.0	
Lane 2	58	3.0	422	0.137	100	14.0	LOS B	0.9	23.0	Short	200	0.0	NA	
Approach	532	3.0		0.781		29.2	LOS C	12.7	324.1					
Intersection	2038	3.0		0.781		16.6	LOS B	12.7	324.1					

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

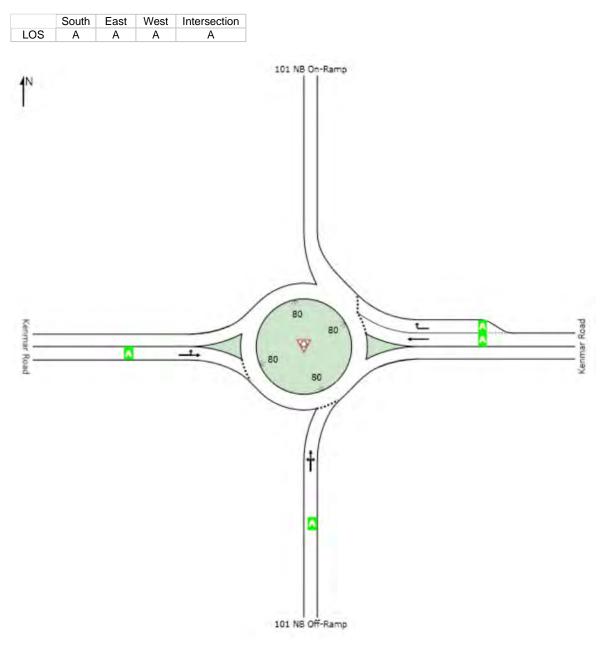
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## LEVEL OF SERVICE

# 🕅 Site: Kenmar Road/ NB Ramps

Kenmar Road Interchange Roundabout Concept - Option 1a & 1b Cumulative AM Roundabout

#### **All Movement Classes**



Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

### LANE SUMMARY

# Site: Kenmar Road/ NB Ramps

Kenmar Road Interchange Roundabout Concept - Option 1a & 1b Cumulative AM Roundabout

Lane Use and Performance													
	Demand F Total veh/h	Flows HV %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Average Delay sec	Level of Service	95% Back o Veh	f Queue Dist ft	Lane Config	Lane Length ft	Cap. Adj. %	Prob. Block. %
South: 101 NB Off-Ramp													
Lane 1 <sup>d</sup>	330	3.0	952	0.346	100	8.1	LOS A	2.0	51.6	Full	1600	0.0	0.0
Approach	330	3.0		0.346		8.1	LOS A	2.0	51.6				
East: Kenmar Road													
Lane 1	489	3.0	1276	0.383	100	4.8	LOS A	2.6	66.4	Full	1600	0.0	0.0
Lane 2 <sup>d</sup>	750	3.0	1506	0.498	100	4.9	LOS A	4.0	102.4	Short	200	0.0	NA
Approach	1239	3.0		0.498		4.8	LOS A	4.0	102.4				
West: Kenmar Road													
Lane 1 <sup>d</sup>	511	3.0	1377	0.371	100	4.6	LOS A	0.0	0.0	Full	1600	0.0	0.0
Approach	511	3.0		0.371		4.6	LOS A	0.0	0.0				
Intersection	2080	3.0		0.498		5.3	LOS A	4.0	102.4				

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

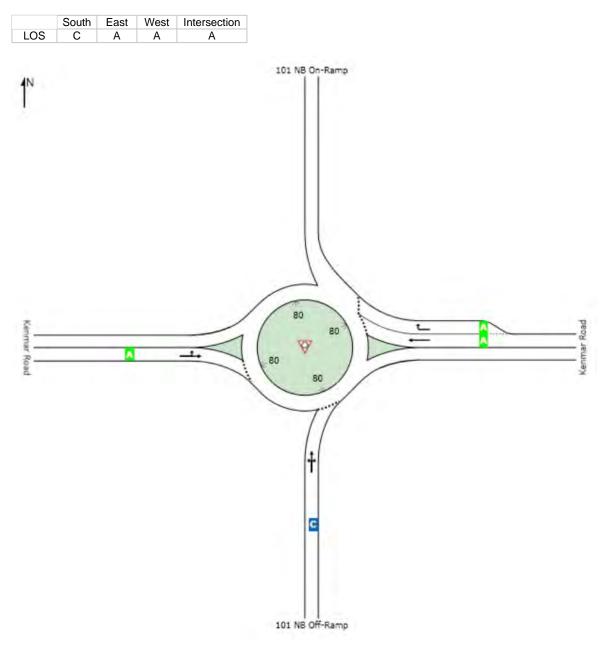
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# LEVEL OF SERVICE

# 😵 Site: Kenmar Road/NB Ramps PM

Kenmar Road Interchange Roundabout Concept - Option 1a & 1b Cumulative PM Roundabout

#### **All Movement Classes**



Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

### LANE SUMMARY

# 🕅 Site: Kenmar Road/NB Ramps PM

Kenmar Road Interchange Roundabout Concept - Option 1a & 1b Cumulative PM Roundabout

Lane Use and Performance													
	Demand Flows		Deg.		Lane	Average Level of		95% Back of Queue		Lane	Lane	Cap.	Prob.
	Total	ΗV	Cap.	Satn	Util.	Delay	Service	Veh	Dist	Config	Length	Adj.	Block.
	veh/h	%	veh/h	v/c	%	sec			ft		ft	%	%
South: 101 NB Off-Ramp													
Lane 1 <sup>d</sup>	380	3.0	569	0.668	100	28.8	LOS C	8.0	204.8	Full	1600	0.0	0.0
Approach	380	3.0		0.668		28.8	LOS C	8.0	204.8				
East: Kenmar Road													
Lane 1 <sup>d</sup>	742	3.0	1445	0.514	100	5.1	LOS A	4.1	104.5	Full	1600	0.0	0.0
Lane 2	479	3.0	1210	0.396	100	5.3	LOS A	2.7	67.8	Short	200	0.0	NA
Approach	1221	3.0		0.514		5.2	LOS A	4.1	104.5				
West: Kenmar Road													
Lane 1 <sup>d</sup>	1100	3.0	1377	0.799	100	4.7	LOS A	0.0	0.0	Full	1600	0.0	0.0
Approach	1100	3.0		0.799		4.7	LOS A	0.0	0.0				
Intersection	2701	3.0		0.799		8.3	LOS A	8.0	204.8				

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

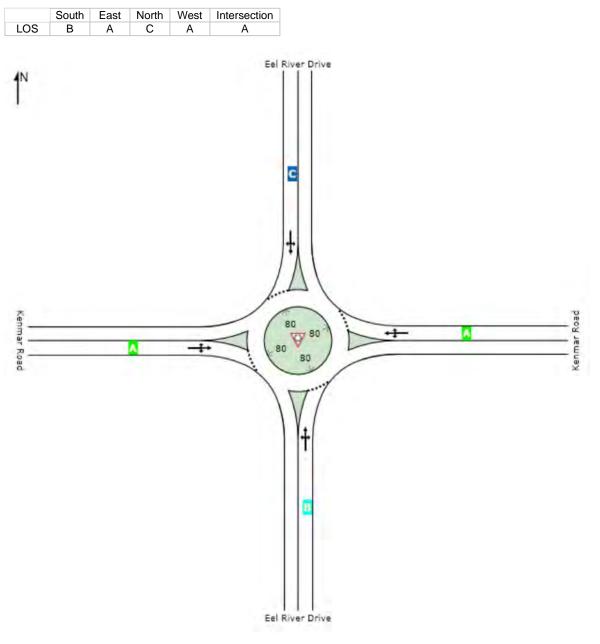
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# LEVEL OF SERVICE

# Site: Kenmar Road/Eel River Drive AM

Kenmar Road Interchange Roundabout Concept - Option 1a Cumulative AM Roundabout

#### **All Movement Classes**



Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

### LANE SUMMARY

## 😵 Site: Kenmar Road/Eel River Drive AM

Kenmar Road Interchange Roundabout Concept - Option 1a Cumulative AM Roundabout

Lane Use an	d Perfor	nance	;										
	Demand F Total veh/h	Flows HV %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Average Delay sec	Level of Service	95% Back o Veh	f Queue Dist ft	Lane Config	Lane Length ft	Cap. Adj. %	Prob. Block. %
South: Eel Riv	er Drive												
Lane 1 <sup>d</sup>	52	3.0	763	0.069	100	11.8	LOS B	0.4	9.8	Full	1600	0.0	0.0
Approach	52	3.0		0.069		11.8	LOS B	0.4	9.8				
East: Kenmar	Road												
Lane 1 <sup>d</sup>	1223	3.0	1324	0.924	100	5.7	LOS A	33.2	848.7	Full	1600	0.0	0.0
Approach	1223	3.0		0.924		5.7	LOS A	33.2	848.7				
North: Eel Riv	er Drive												
Lane 1 <sup>d</sup>	3	3.0	248	0.014	100	21.6	LOS C	0.1	2.6	Full	1600	0.0	0.0
Approach	3	3.0		0.014		21.6	LOS C	0.1	2.6				
West: Kenmar	Road												
Lane 1 <sup>d</sup>	762	3.0	1346	0.566	100	4.4	LOS A	6.2	157.9	Full	1600	0.0	0.0
Approach	762	3.0		0.566		4.4	LOS A	6.2	157.9				
Intersection	2041	3.0		0.924		5.4	LOS A	33.2	848.7				

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

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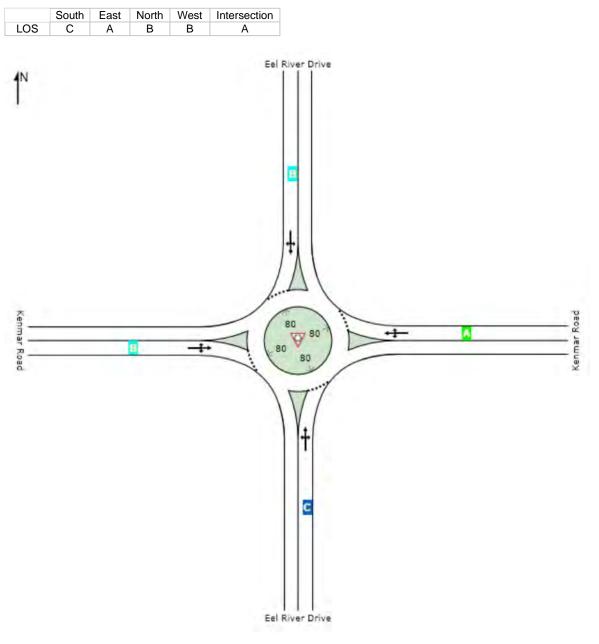
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## LEVEL OF SERVICE

## Site: Kenmar Road/Eel River Drive PM

Kenmar Road Interchange Roundabout Concept - Option 1a Cumulative PM Roundabout

#### **All Movement Classes**



Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

### LANE SUMMARY

## 😵 Site: Kenmar Road/Eel River Drive PM

Kenmar Road Interchange Roundabout Concept - Option 1a Cumulative PM Roundabout

Lane Use ar	nd Perfor	nance	;										
	Demand F		0.00	Deg.	Lane	Average	Level of	95% Back c		Lane	Lane	Cap.	Prob.
	Total veh/h	HV %	Cap. veh/h	Satn v/c	Util. %	Delay sec	Service	Veh	Dist ft	Config	Length ft	Adj. %	Block. %
South: Eel Riv		/0	ven/m	V/C	/0	360			п		11	/0	/0
Lane 1 <sup>d</sup>	48	3.0	216	0.225	100	26.8	LOS C	1.8	45.0	Full	1600	0.0	0.0
Approach	48	3.0		0.225		26.8	LOS C	1.8	45.0				
East: Kenmar	Road												
Lane 1 <sup>d</sup>	1201	3.0	1323	0.908	100	5.4	LOS A	31.4	803.7	Full	1600	0.0	0.0
Approach	1201	3.0		0.908		5.4	LOS A	31.4	803.7				
North: Eel Riv	er Drive												
Lane 1 <sup>d</sup>	3	3.0	275	0.011	100	19.6	LOS B	0.1	2.2	Full	1600	0.0	0.0
Approach	3	3.0		0.011		19.6	LOS B	0.1	2.2				
West: Kenma	r Road												
Lane 1 <sup>d</sup>	1338	3.0	1358	0.986	100	10.2	LOS B	148.8	3810.3	Full	1600	0.0	<mark>45.4</mark>
Approach	1338	3.0		0.986		10.2	LOS B	148.8	3810.3				
Intersection	2591	3.0		0.986		8.3	LOS A	148.8	3810.3				

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

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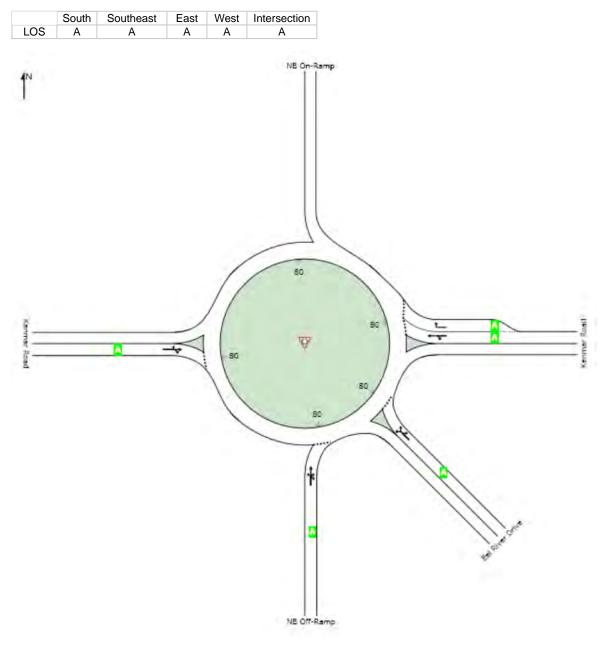
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## LEVEL OF SERVICE

## V Site: Kenmar Road/Eel River Drive/NB Ramps AM

Kenmar Road Interchange Roundabout Concept - Option 2 Cumulative AM Roundabout

#### **All Movement Classes**



Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

### LANE SUMMARY

## 😵 Site: Kenmar Road/Eel River Drive/NB Ramps AM

Kenmar Road Interchange Roundabout Concept - Option 2 Cumulative AM Roundabout

Lane Use a	nd Perfor	nance	<b>;</b>										
	Demand F Total veh/h	Flows HV %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Average Delay sec	Level of Service	95% Back of Veh	Queue Dist ft	Lane Config	Lane Length ft	Cap. Adj. %	Prob. Block. %
South: NB Of	ff-Ramp												
Lane 1 <sup>d</sup>	305	3.0	951	0.321	100	7.8	LOS A	1.9	48.0	Full	1600	0.0	0.0
Approach	305	3.0		0.321		7.8	LOS A	1.9	48.0				
SouthEast: E	el River Dri	ve											
Lane 1 <sup>d</sup>	47	3.0	720	0.066	100	9.9	LOS A	0.4	9.8	Full	1600	0.0	0.0
Approach	47	3.0		0.066		9.9	LOS A	0.4	9.8				
East: Kenma	r Road												
Lane 1	468	3.0	1258	0.372	100	5.1	LOS A	2.4	61.3	Full	1600	0.0	0.0
Lane 2 <sup>d</sup>	695	3.0	1484	0.468	100	5.0	LOS A	3.5	88.3	Short	200	0.0	NA
Approach	1163	3.0		0.468		5.0	LOS A	3.5	88.3				
West: Kenma	ar Road												
Lane 1 <sup>d</sup>	474	3.0	1356	0.349	100	4.7	LOS A	2.3	59.8	Full	1600	0.0	0.0
Approach	474	3.0		0.349		4.7	LOS A	2.3	59.8				
Intersection	1989	3.0		0.468		5.5	LOS A	3.5	88.3				

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

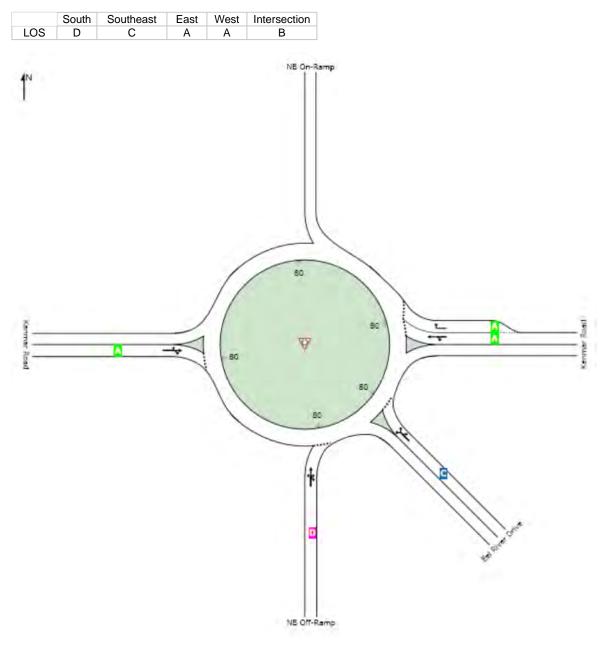
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## LEVEL OF SERVICE

## 😵 Site: Kenmar/Eel River Drive/NB Ramps PM

Kenmar Road Interchange Roundabout Concept - Option 2 Cumulative PM Roundabout

#### **All Movement Classes**



Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

### LANE SUMMARY

## V Site: Kenmar/Eel River Drive/NB Ramps PM

Kenmar Road Interchange Roundabout Concept - Option 2 Cumulative PM Roundabout

Lane Use a	nd Perform	nance	9										
	Demand F Total veh/h	Flows HV %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Average Delay sec	Level of Service	95% Back o Veh	f Queue Dist ft	Lane Config	Lane Length ft	Cap. Adj. %	Prob. Block. %
South: NB O	ff-Ramp												
Lane 1 <sup>d</sup>	380	3.0	495	0.768	100	37.3	LOS D	10.8	276.2	Full	1600	0.0	0.0
Approach	380	3.0		0.768		37.3	LOS D	10.8	276.2				
SouthEast: E	el River Dri	ve											
Lane 1 <sup>d</sup>	47	3.0	250	0.189	100	32.1	LOS C	1.3	33.6	Full	1600	0.0	0.0
Approach	47	3.0		0.189		32.1	LOS C	1.3	33.6				
East: Kenma	r Road												
Lane 1 <sup>d</sup>	753	3.0	1418	0.531	100	5.4	LOS A	4.2	107.2	Full	1600	0.0	0.0
Lane 2	479	3.0	1179	0.406	100	5.5	LOS A	2.7	68.7	Short	200	0.0	NA
Approach	1232	3.0		0.531		5.4	LOS A	4.2	107.2				
West: Kenma	ar Road												
Lane 1 <sup>d</sup>	1100	3.0	1362	0.808	100	4.9	LOS A	16.1	413.2	Full	1600	0.0	0.0
Approach	1100	3.0		0.808		4.9	LOS A	16.1	413.2				
Intersection	2759	3.0		0.808		10.0	LOS B	16.1	413.2				

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

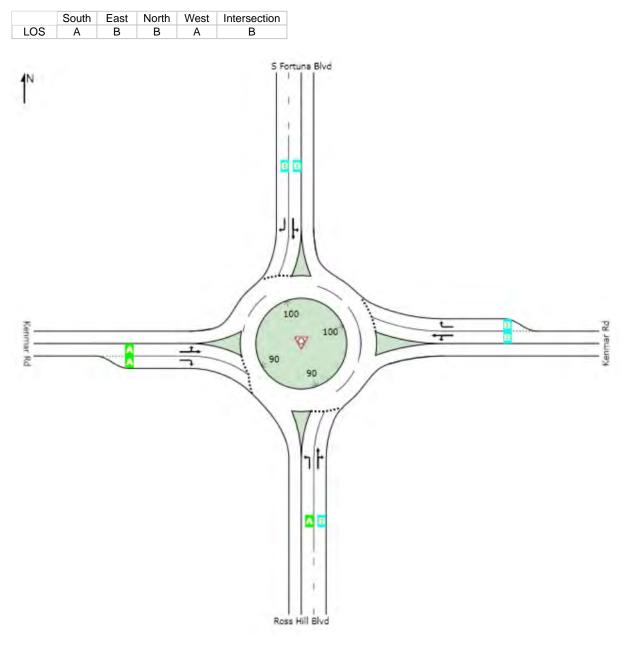
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## LEVEL OF SERVICE

## Site: Kenmar Road/Ross Hill Road/Fortuna Boulevard AM

Kenmar Road Interchange Roundabout Concept Cumulative AM Roundabout

#### **All Movement Classes**



Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies.

### LANE SUMMARY

😵 Site: Kenmar Road/Ross Hill Road/Fortuna Boulevard AM

Kenmar Road Interchange Roundabout Concept Cumulative AM Roundabout

Lane Use and Performance													
	Demand F Total veh/h	lows= HV %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Average Delay sec	Level of Service	95% Back of Veh	f Queue Dist ft	Lane Config	Lane Length ft	Cap. Adj. %	Prob. Block. %
South: Ross H		/0	VEH/H	V/C	/0	360			п		11	/0	/0
Lane 1 <sup>d</sup>	551	2.0	1121	0.492	100	8.7	LOS A	4.0	100.5	Full	1600	0.0	0.0
Lane 2	483	2.0	922	0.524	100	10.7	LOS B	4.4	111.7	Full	1600	0.0	0.0
Approach	1034	2.0		0.524		9.7	LOS A	4.4	111.7				
East: Kenmar	Rd												
Lane 1 <sup>d</sup>	290	2.0	675	0.429	100	11.4	LOS B	2.6	64.9	Full	1600	0.0	0.0
Lane 2	275	2.0	525	0.524	100	16.8	LOS B	3.1	79.1	Short	200	0.0	NA
Approach	565	2.0		0.524		14.0	LOS B	3.1	79.1				
North: S Fortu	na Blvd												
Lane 1	341	2.0	635	0.536	100	14.7	LOS B	4.9	125.1	Full	1600	0.0	0.0
Lane 2 <sup>d</sup>	409	2.0	860	0.476	100	10.3	LOS B	4.4	111.8	Full	1600	0.0	0.0
Approach	750	2.0		0.536		12.3	LOS B	4.9	125.1				
West: Kenmar	Rd												
Lane 1 <sup>d</sup>	403	2.0	1172	0.344	100	6.4	LOS A	2.6	66.3	Full	1600	0.0	0.0
Lane 2	347	2.0	979	0.354	100	7.5	LOS A	2.5	64.6	Short	200	0.0	NA
Approach	750	2.0		0.354		6.9	LOS A	2.6	66.3				
Intersection	3099	2.0		0.536		10.4	LOS B	4.9	125.1				

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Roundabout Capacity Model: SIDRA Standard.

HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

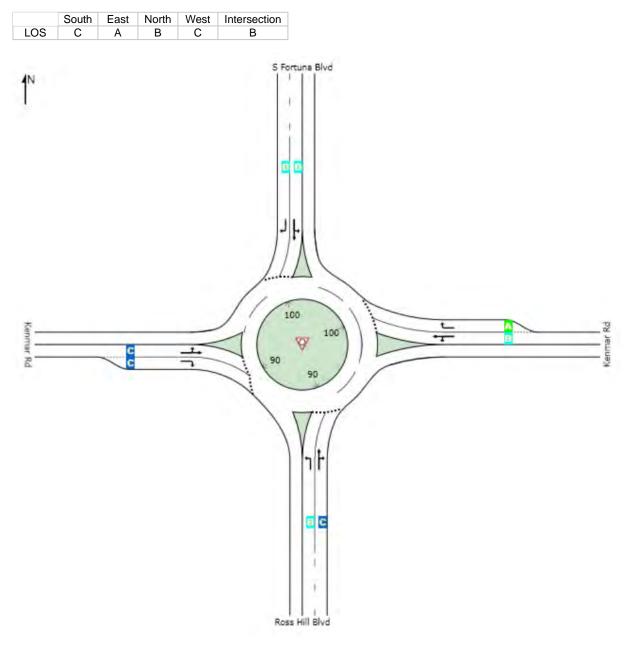
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## LEVEL OF SERVICE

## Site: Kenmar Road/Ross Hill Road/Fortuna Boulevard PM

Kenmar Road Interchange Roundabout Concept Cumulative AM Roundabout

#### **All Movement Classes**



Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies.

### LANE SUMMARY

😵 Site: Kenmar Road/Ross Hill Road/Fortuna Boulevard PM

Kenmar Road Interchange Roundabout Concept Cumulative AM Roundabout

Lane Use and Performance													
	Demand F Total	lows <sup>-</sup> Iows	Cap.	Deg. Satn	Lane Util.	Average Delav	Level of Service	95% Back o Veh	f Queue Dist	Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	veh/h	%	veh/h	v/c	%	Sec	0011100	Volt	ft	Conng	ft	%	%
South: Ross	Hill Blvd												
Lane 1 <sup>d</sup>	389	2.0	652	0.596	100	16.3	LOS B	6.7	169.4	Full	1600	0.0	0.0
Lane 2	354	2.0	501	0.706	100	26.2	LOS C	8.2	207.2	Full	1600	0.0	0.0
Approach	742	2.0		0.706		21.0	LOS C	8.2	207.2				
East: Kenma	r Rd												
Lane 1	146	2.0	539	0.272	100	10.5	LOS B	1.4	34.9	Full	1600	0.0	0.0
Lane 2 <sup>d</sup>	146	2.0	673	0.217	100	7.9	LOS A	1.2	29.8	Short	200	0.0	NA
Approach	293	2.0		0.272		9.2	LOS A	1.4	34.9				
North: S Fort	una Blvd												
Lane 1	601	2.0	849	0.708	100	17.4	LOS B	8.7	221.9	Full	1600	0.0	0.0
Lane 2 <sup>d</sup>	621	2.0	1045	0.594	100	11.3	LOS B	6.2	157.9	Full	1600	0.0	0.0
Approach	1222	2.0		0.708		14.3	LOS B	8.7	221.9				
West: Kenma	ar Rd												
Lane 1 <sup>d</sup>	717	2.0	903	0.794	100	21.4	LOS C	12.8	324.5	Full	1600	0.0	0.0
Lane 2	540	2.0	717	0.754	100	22.5	LOS C	10.0	253.5	Short	200	0.0	NA
Approach	1258	2.0		0.794		21.9	LOS C	12.8	324.5				
Intersection	3515	2.0		0.794		18.0	LOS B	12.8	324.5				

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > irrespective of lane delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

Roundabout Capacity Model: SIDRA Standard.

HCM Delay Formula option is used. Control Delay does not include Geometric Delay since Exclude Geometric Delay option applies. Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

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Attachment C - Review of Geometric Design Standards



## Memorandum

#### June 22, 2016

Project:	Fortuna Highway 101/Riverwalk Connectivity Study		
Subject:	Review of Safety and Design Standards		
Client:	Humboldt County Association of Governments	Job no.	: 11109149
Prepared by:	David Caisse, P.E and Josh Wolf, P.E.	Tel:	(707) 443-8326

### Introduction / Objective

This memo is intended to provide a brief summary of the existing conditions and identify potential nonstandard features for the Highway 101 interchanges at 12<sup>th</sup> Street and Kenmar Road. Existing facilities were compared against the applicable standards and guidelines for the roadway being analyzed. For example, standards for the Highway 101 on and off ramps and other State owned facilities are based on the Caltrans Highway Design Manual. Local facilities are based on the local agency or Federal guidance or standards (generally whichever are more stringent). Local facilities located within the State right-of-way crossing over or under a freeway or expressway and connecting to the state facility are based on the State's design standards. Below is a list of public standards which are commonly used.

### **Common Public Design Standards and Guidelines**

#### State of California

*Caltrans Highway Design Manual* – This manual was developed by the California Department of Transportation (Caltrans) to establish uniform policies and procedures to carry out the State highway design functions of the department. Design standards include items such as roadway geometry, pavement engineering, drainage, bicycle transportation and other miscellaneous design standards.

*California Manual on Uniform Traffic Control Devices (CA MUTCD)* – This manual provides uniform standards and specifications for all official traffic control devices in California. Design standards include items such as signs, markings, signal and temporary traffic control for vehicular, rail and bicycle facilities. The CA MUTCD is based on Federal Highway Administrations (FHWA) *2009 National Manual on Uniform Traffic Control Devices* with California revisions and amendments.

#### Federal

AASHTO Geometric Design of Highways and Streets – Similar to the Highway Design Manual, these guidelines are intended to provide roadway design standards with operational efficiency, comfort, safety and convenience of the motorist in mind. Design standards include items such as highway function, design controls and elements of design for various functional classifications of roadways (freeways, arterials, collectors, local roads, etc.).



US Department of Justice's ADA Standards for Accessible Design – These standards are based on the Americans with Disabilities Act of 1990 (ADA) and provide standards to prohibit discrimination and ensure equal opportunity for persons with disabilities. Design elements include standards for accessible routes, general site and building elements (residential and commercial) and recreational facilities.

AASHTO Guide for the Development of Bicycle Facilities – This manual was developed to provide an overview of planning and design considerations, as well as recommendations for operation and maintenance of various types of bicycle facilities.

*NACTO Urban Bikeway Design Guide* – Similar to the AASHTO Guide for the Development of Bicycle Facilities, this manual was developed to provide guidance for the planning and design of bicycle facilities. This manual however, was developed by cities for cities based on the experience of the best cycling cities in the world.

As discussed later in this memo, there are currently no pedestrian or bicycle facilities in the immediate vicinity of the interchanges, therefore the later three of the manuals identified above were not used when evaluating the existing conditions.

### **Condition Assessment**

A reconnaissance level condition assessment was performed and used to identify fundamental deficiencies as compared to the current design standards. The results presented below are based on a preliminary level characterization to provide background information and guidance for evaluating the existing conditions. For example, the characterization is based on notable qualitative characteristics visually observed and/or measured during a site walk, rather than a detailed investigation or survey of the existing conditions.

The characteristics noted are based on observable features that are relevant to the evaluation of the current layout. The characterization is intended to serve as a planning tool to provide additional information to be considered when recommending improvement alternatives. The primary factors being investigated are items such as horizontal alignment, geometric cross section, design vehicles, clearances, and sight distance.

### Results

The results of the condition assessment for each segment of roadway can be found on the following pages.



#### 12th Street and US Highway 101 Interchange

12<sup>th</sup> Street/Riverwalk Drive

Roadway Segment: Riverwalk Dr/	12th St			
	Existing Roadway Characteristics			
Facility Type	Local Facility		/	
Functional Classification	Minor Arterial / Major Collector			12-5
No. of Lanes	2			Hans Bank III A
Rural/Urban	Rural	Design	Meets	Second and the second second
Bike Facilities (Y/N)	N	Standards	Standards	Reference to Standard
Pedestrian Facilities (Y/N)	N		$(\sqrt{y} = yes)$	
Posted Speed/Design Speed (mph)	30/35	45 <sup>1</sup>		HDM Index 101.1
Lane Width (ft)	12	12	1	HDM Index 301.1 / AASHTO
Overcrossing Width (ft)	28	32		HDM Index 308.1
Right Shoulder Width (ft)	Var. 2' - 8' (between NB and SB ramps)	4		HDM Index 302.1 & 308.1 / AASHTO
Curve Radii (ft)	300+/-	425		HDM Index 203.2
Decision Sight Distance (ft)	400 <sup>2</sup>	525		HDM Index 201.7
Intersection Spacing (ft)	0' (Between SB ramps and Dinsmore Dr) & 300' (between NB ramps and Newburg Rd)	500		HDM Index 504.3
Horizontal Clearance (ft)	4'+ (w/out curb) & 1.5'+ (w/ curb)	4' (w/out curb) & 1.5' (w/ curb)	J	HDM Index 309.1
Vertical Clearance (ft - in)	15' - 5" <sup>3</sup>	16' - 6"		HDM Index 309.2
Stopping Sight Distance (ft)	250+	250	1	HDM Index 201.1
Design Vehicle	Cal Legal - 50 <sup>4</sup>	Cal Legal - 50		HDM Index 404.4

1 Design Standard applies to connections to freeways or expressways

<sup>2</sup> The roadway geometry could probably accommodate the minimum Decision Sight Distance, but some trees might need trimming or be removed. This location is an overcrossing so the vertical clearance shown here is for US Hwy 101.

<sup>4</sup> A Cal Legal - 50 Truck could probably navigate the turns, but may be required to travel outside the lane slightly and use the gore area or adjacent shoulder.

- Posted Speed/Design Speed When feasible, the design speed of local facilities connecting to a ٠ freeway or expressway should be 45 mph, but shall be a minimum of 35 mph.
- Overcrossing Width The adjacent sections of roadway approaching the overcrossing are urban ٠ in nature and contain 12' travel lanes with 8' shoulders. At the overcrossing, the section narrows to 12' travel lanes, 2' paved shoulders and a concrete curb/vehicular railing which begins at the edge of the shoulder.



- Right Shoulder Width The shoulder width decreases to only 2' within the overcrossing area and again on the Strongs Creek Bridge (which is located at the southern extents of this segment). All other portions of this segment have shoulders which meet the minimum design standard of 4' wide. The widths vary, but are generally around 8' in width.
- Intersection Spacing -
  - At the southern extent of this segment, Dinsmore Road intersects Riverwalk Drive immediately adjacent to the SB ramps. Due to its proximity and configuration, Dinsmore Road appears more like a 5<sup>th</sup> leg of the Riverwalk Drive and SB ramp intersection rather than its own. Drivers appear to be confused and have been observed traveling directly from Dinsmore Drive to the SB ramp or northward towards the downtown area.
  - At the northern extent of the segment, the NB on and off ramps are located approximately 300' south of the Newburg Road and 12<sup>th</sup> Street intersection. The preferred distance between intersections (from curb return to curb return) is 500', but shall be a minimum of 400'.
- Curve Radii 12<sup>th</sup> Street is a relatively straight section of road, but contains a few curves near the interchange. The first curve heading south towards Highway 101 is slightly smaller than recommended based on the speed of the roadway through that section.
- Decision Sight Distance Near the overcrossing, there are a number of large conifers that restrict visibility. The sight distance could be improved and would likely meet the standards if the trees were trimmed or removed.
- Vertical Clearance Since this is an overcrossing, the vertical clearance described here is for the vehicles on Highway 101.
- Design Vehicle In all cases, it appears as though a Cal Legal-50 truck could navigate the turns and stay within the paved roadway area; however, due to the tight radii entering and exiting the ramps and turning on and off the side streets (Dinsmore Drive and Newburg Road), large trucks would need to encroach slightly into the oncoming travel lane and or gore area.



#### Newburg Road

Roadway Segment: Newburg Rd				
	Existing Roadway Characteristics			
Facility Type	Local Facility			
Functional Classification	Major Collector			
No. of Lanes	2			The Property of the second
Rural/Urban	Rural		Meets	
Bike Facilities (Y/N)	N	Design Standards	Standards	Reference to Standard
Pedestrian Facilities (Y/N)	Y (north side only)		$(\sqrt{y} = yes)$	
Posted Speed/Design Speed (mph)	25/30	25	J	AASHTO
Lane Width (ft)	12	12	J	HDM Index 301.1 / AASHTO
Right Shoulder Width (ft)	8/4		J	HDM Index 302.1 & 308.1 / AASHTO
Curve Radii (ft)	300+	300	J	HDM Index 203.2
Decision Sight Distance (ft)	450+	450	J	HDM Index 201.7
Angle of Intersection (Degree)	45	75		HDM Index 403.3
Horizontal Clearance (ft)	3'+/- (in areas w/out curb)	4' (w/out curb) & 1.5' (w/ curb)		HDM Index 309.1
Stopping Sight Distance (ft)	200+	200	J	NDM Index 201.1
Design Vehicle	Cal Legal - 50 <sup>1</sup>	Cal Legal - 50		HDM Index 404.4

<sup>1</sup> A Cal Legal - 50 Truck could probably navigate the turns, but would be required travel outside its lane.

- Angle of Intersection Newberg Intersects 12<sup>th</sup> Street at a 45 degree angle. Provided there are no physical constraints, the interior angle should be 90 degrees or as close to 90 degrees as practical, but should not be less than 75 degrees.
  - Horizontal Clearance The southern half of the roadway contains a number of utility poles that are very close to the edge of the travel lane.
- Design Vehicle Newberg Road intersects 12<sup>th</sup> Street at an acute angle. Due to the angle and tight radii, large trucks need to encroach into oncoming travel lane to navigate the turns and stay within the existing pavement.



#### **Dinsmore Drive**

Roadway Segment: Dinsmore Dr		_		
	Existing Roadway Characteristics			De AL
Facility Type	Local Facility			
Functional Classification	Local Road			
No. of Lanes	2		/	
Rural/Urban	Rural	Design	Meets	
Bike Facilities (Y/N)	N	Standards	Standards	Reference to Standard
Pedestrian Facilities (Y/N)	N	Standarus	$(\sqrt{y} = yes)$	
Posted Speed/Design Speed (mph)	25/30	25	1	AASHTO
Lane Width (ft)	12	12	1	HDM Index 301.1 / AASHTO
Right Shoulder Width (ft)	2	X	1	HDM Index 302.1 & 308.1 / AASHTO
Curve Radii (ft)	300+	300	1	HDM Index 203.2
Decision Sight Distance (ft)	450+	450	1	HDM Index 201.7
Horizontal Clearance (ft)	4'+/- 1	4'	X	HDM Index 309.1
Stopping Sight Distance (ft)	200+	200		HDM Index 201.1
Design Vehicle	Cal Legal - 50 <sup>2</sup>	Cal Legal - 50		NDM Index 404.4

<sup>1</sup> Power poles are very close to the edge of the pavepient.

<sup>2</sup> A Cal Legal - 50 Truck could probably navigate the turns, but would be required travel outside its lane.

• Design Vehicle Dinsmore Drive intersects 12<sup>th</sup> Street as one of the five legs of this intersection. As a result, the intersection is tight and confusing. Due to the tight radius and close proximity of the bridge to the intersection, large trucks heading or coming from the south leg of the intersection are required to swing wide and encroach into oncoming travel lanes.



### US Highway 101 Northbound Ramp

Roadway Segment: US Hwy 101 Vo	orthbound (On and off ram)	9 <b>5)</b>		
	Existing Roadway Sharacteristics			JAN AND
Facility Type	Freeway Expressway			
Functional Classification	Freeway / Expressway			
No. of Lanes	1			THE PARTY
Rural/Urban	Rural	X	Meets	the statement of the statement
Bike Facilities (Y/N)	N	Design	Standards	Reference to Standard
Pedestrian Facilities (Y/N)	N	Standards	$(\sqrt{y} = yes)$	
Posted Speed/Design Speed (mph)	35/40	25/50 <sup>1</sup>		HDM Index 504.3
Lane Width (ft)	12	12	1	HDM Index 301.1
Right Shoulder Width (ft)	8	8		HDM Index 302.1 & 308.1 / AASHTO
Curve Radii (ft)	400 / 600	550		NDM Index 203.2
Decision Sight Distance (ft)	425+/-	600		HDM Index 201.7
Horizontal Clearance (ft)	4'+ (w/out curb) & 1.5'+ (w/ curb)	4' (w/out curb) & 1.5' (w/ curb)	J	HDM Index 309.1
Stopping Sight Distance (ft)	300+	300	1	HDM Index 201.1
Design Vehicle	STAA <sup>2</sup>	STAA		HDM Index 404.4

<sup>1</sup> Design speed should be 25 mph when traffic is expected to make a turning movement at the terminus and 50 mph when entering, exiting a ramp or when a "through" movement is provided at the terminus.

<sup>2</sup> An STAA truck could probably navigate the turns, but would be required travel outside its lane.

- Posted Speed/Design Speed The design speed of ramp can vary depending on the alignment and controls at each end. An acceptable approach is to set 25 mph and 50 mph design speeds for the ramp terminus and exit nose, respectively. The NB off ramp terminates at an intersection where traffic is expected to make a turning movement; therefore, the design speed should be 25 mph nearing this portion of the ramp.
- Curve Radii The design standard for the minimum curve radius of the northbound on and off
  ramps are based on the posted speed limit entering the on ramp from Highway 101. The curve
  radius identified below as not meeting the standard are is located on the northbound on ramp just
  before entering Highway 101. This particular section of road has no posted speed limit, but traffic
  entering Highway 101 at this location is accelerating and approaching speeds in excess of 40
  mph. If considerations are made for improvements to this interchange, this radius should be
  increased to meet the current design standards.
- Decision Sight Distance Similar to the 12<sup>th</sup> Street overcrossing, there are a number of large



conifers (Redwoods) along the right side of the off ramp that restrict visibility. The sight distance could be improved and would likely meet the standards if the trees were trimmed or removed.

 Design Vehicle – In all cases, it appears as though an STAA truck could navigate the turns and stay within the pavement; however, due to the tight radii entering and exiting the ramps and turning on and off 12<sup>th</sup> Street, large trucks would need to encroach slightly into the oncoming travel lane or gore area.



#### JS Highway 101 Southbound Ramp Roadway Segment: US Hwy 101 Southbound (On and off ramps) **Existing Roadway** Characteristics Facility Type Freeway / Expressway Functional Classification Freeway / Expressway No. of Lanes Rural/Urban Rural Meets Design Bike Facilities (Y/N) N Standards **Reference to Standard** Standards Pedestrian Facilities (Y/N) $(\sqrt{-\text{yes}})$ N 25/50 1 Posted Speed/Design Speed (mph) 25/30 HDM Index 504.3 Lane Width (ft) HDM Index 301.1 12 12 HDM Index 302.1 & 308.1 / Right Shoulder Width (ft) 8 8 AASHTO Curve Radii (ft) 300/650 300 HDM Index 203.2 \* Decision Sight Distance (ft) HDM Index 201.7 450+ 450 1 4'+ (w/out curb) & 1.5'+ 4' (w/out curb) & Horizontal Clearance (ft) HDM Index 309.1 (w/ curb) 1.5' (w/ curb) J HDM Index 201.1 Stopping Sight Distance (ft) 200 +200 1 STAA<sup>2</sup> Design Vehicle HDM Index 404.4 STAA

<sup>1</sup> Design speed should be 25 mph when traffic is expected to make a turning movement at the terminus and 50 mph when entering, exiting a ramp or when a "through" movement is provided at the terminus.

<sup>2</sup> An STAA truck could probably navigate the turns, but would be required travel outside its lane.

- Posted Speed/Design Speed The design speed of ramp can vary depending on the alignment and controls at each end. An acceptable approach is to set 25 mph and 50 mph design speeds for the ramp terminus and exit nose, respectively. The SB off ramp terminates at an intersection where traffic is expected to make a turning movement; therefore, the design speed should be 25 mph nearing this portion of the ramp.
- Design Vehicle In all cases, it appears as though an STAA truck could navigate the turns and stay within the pavement; however, due to the tight radii entering and exiting the ramps and turning on and off 12<sup>th</sup> Street or Dinsmore Drive, large trucks would need to encroach slightly into the oncoming travel lanes or gore area.



#### Kenmar Road and US Highway 101 Interchange

#### Kenmar Road

Roadway Segment: <mark>Kenmar Rd</mark>	1			
	Existing Roadway Characteristics			
Facility Type	Local Facility			LI AL
Functional Classification	Other Principal Arterial / Major Collector			1103
No. of Lanes	2			
Rural/Urban	Rural		Meets	
Bike Facilities (Y/N)	N	Design Standards	Standards	Reference to Standard
Pedestrian Facilities (Y/N)	N		$(\sqrt{y} = yes)$	
Posted Speed/Design Speed (mph)	35/40	45 <sup>1</sup>		HDM Index 101.1
Lane Width (ft)	12	12	1	HDM Index 301.1 / AASHTO
Right Shoulder Width (ft)	8	4	J	HDM Index 302.1 & 308.1 / AASHTO
Curve Radii (ft)	600 / 75	550	11	HDM Index 203.2
Decision Sight Distance (ft)	230+/-	600		HDM Index 201.7
Horizontal Clearance (ft)	4'+ (w/out curb) & 1.5'+ (w/ curb) or shielded	4' (w/out curb) & 1.5' (w/ curb)	J	HDM Index 309.1
Vertical Clearance (ft - in)	14' - 10" <sup>2</sup>	15	1	HDM Index 309.2
Stopping Sight Distance (ft)	125+/-	300		HDM Index 201.1
Design Vehicle	Cal Legal - 50 <sup>3</sup>	Cal Legal - 50	1	HDM Index 404.4

<sup>1</sup> Design Standard applies to connections to freeways or expressways

<sup>2</sup> This location is an undercrossing so the vertical clearance shown here is for Kenmar Rd.

<sup>3</sup> A Cal Legal - 50 Truck could probably navigate the turns, but would be required travel outside its lane.

- Posted Speed/Design Speed When feasible, the design speed of local facilities connecting to a freeway or expressway should be 45 mph, but shall be a minimum of 35 mph.
- Curve Radii Most of Kenmar is relatively straight, but near the southern portion of the interchange there is a tight radius. The curve radius here is significantly smaller than recommended based on the speed of the roadway through that section.
- Decision Sight Distance As a result of the tight radius identified above and dense vegetation growing outside of the right of way, visibility is obstructed.
- Vertical Clearance Kenmar Road is an undercrossing at this location so the vertical clearance described here is for the vehicles on Kenmar Road.
- Stopping Sight Distance Similar to Decision Sight Distance, the tight radius and dense vegetation obstructs visibility reducing the available stopping sight distance.



• Design Vehicle – In all cases, it appears as though a Cal Legal-50 truck could navigate the turns and stay within the paved roadway area; however, due to the tight radii entering and exiting the ramps and small curve radius identified above, large trucks would need to make wide turns and encroach slightly into the oncoming travel lane or gore area.



### Eel River Drive

Roadway Segment: <mark>Eel River Dr</mark>		6		
	Existing Roadway Characteristics Local Facility			
Facility Type				
Functional Classification	Major Collector			
No. of Lanes	2			
Rural/Urban	Rural		Meets Standards $(\sqrt{-} yes)$	Reference to Standard
Bike Facilities (Y/N)	N	Design Standards		
Pedestrian Facilities (Y/N)	N			
Posted Speed/Design Speed (mph)	30/35	30	1	AASHTO
Lane Width (ft)	11	9	1	HDM Index 301.1 / AASHTO
Right Shoulder Width (ft)	2+	2	J	HDM Index 302.1 & 308.1 / AASHTO
Curve Radii (ft)	85	425		HDM Index 203.2
Decision Sight Distance (ft)	525+	525	1	HDM Index 201.7
Intersection Spacing (ft)	150' (Between NB ramps and Eel River Dr)	500'		HDM Index 504.3
Horizontal Clearance (ft)	4'+/-	4' (w/out curb) & 1.5' (w/ curb)	J	HDM Index 309.1
Stopping Sight Distance (ft)	250+	250	J	HDM Index 201.1
Design Vehicle	Cal Legal - 50 <sup>1</sup>	Cal Legal - 50		HDM Index 404.4

- Curve Radii Most of Eel River Drive is relatively straight, but near its intersection with Kenmar Road there is a tight radius. The curve radius here is significantly smaller than recommended based on the speed of the roadway through that section; however, at this point the road is approaching the STOP sign so speeds would be lower. If considerations are made for improvements to this interchange, the curve radius or approach angle should be evaluated.
- Intersection Spacing The SB on and off ramps are located approximately 150' south of the Eel River Drive and Kenmar Road intersection. The preferred distance between intersections (from curb return to curb return) is 500', but shall be a minimum of 400'.
- Design Vehicle Eel River Drive intersects Kenmar Road at an acute angle. Due to the angle and tight radii, large trucks need to encroach into the oncoming travel lane to navigate the turns and stay within the existing paved roadway.



#### US Highway 101 Northbound Ramp

Roadway Segment: US Hwy 101 No	orthbound (On and off ramp Existing Roadway Characteristics	s)		
Facility Type	Freeway / Expressway			
Functional Classification	Freeway / Expressway			4 1 2 1
No. of Lanes	1			
Rural/Urban	Rural	Design Standards	$Meets$ Standards $(\sqrt{y} = yes)$	Reference to Standard
Bike Facilities (Y/N)	N			
Pedestrian Facilities (Y/N)	N			
Posted Speed/Design Speed (mph)	35/40	25/50 <sup>1</sup>		HDM Index 504.3
Lane Width (ft)	12	12	1	HDM Index 301.1
Right Shoulder Width (ft)	8	8	J	HDM Index 302.1 & 308.1 / AASHTO
Curve Radii (ft)	N/A	550	1	HDM Index 203.2
Decision Sight Distance (ft)	600+	600	1	HDM Index 201.7
Horizontal Clearance (ft)	4'+ (w/out curb) & 1.5'+ (w/ curb)	4' (w/out curb) & 1.5' (w/ curb)	J	HDM Index 309.1
Stopping Sight Distance (ft)	300+	300	1	HDM Index 201.1
Design Vehicle	STAA <sup>2</sup>	STAA		HDM Index 404.4

<sup>1</sup> Design speed should be 25 mph when traffic is expected to make a turning movement at the terminus and 50 mph when entering, exiting a ramp or when a "through" movement is provided at the terminus.

<sup>2</sup> An STAA truck could probably navigate the turns, but would be required travel outside its lane.

- Posted Speed/Design Speed The design speed of ramp can vary depending on the alignment and controls at each end. An acceptable approach is to set 25 mph and 50 mph design speeds for the ramp terminus and exit nose, respectively. The NB off ramp terminates at an intersection where traffic is expected to make a turning movement; therefore, the design speed should be 25 mph nearing this portion of the ramp.
- Design Vehicle In all cases, it appears as though an STAA truck could navigate the turns and stay within the pavement; however, due to the tight radii entering and exiting the ramps and turning on and off Kenmar Road, large trucks would need to encroach slightly into the oncoming travel lane or gore area.



#### US Highway 101 Southbound Ramp

Roadway Segment: US Hwy 101 So	uthbound (On and off ramp	5)		
	Existing Roadway Characteristics			
Facility Type	Freeway / Expressway			and the second
Functional Classification	Freeway / Expressway			
No. of Lanes	1			
Rural/Urban	Rural	Design Standards	Meets Standards $(\sqrt{y} = yes)$	Reference to Standard
Bike Facilities (Y/N)	N			
Pedestrian Facilities (Y/N)	N			
Posted Speed/Design Speed (mph)	35/40 <sup>1</sup>	25/50 <sup>2</sup>		HDM Index 504.3
Lane Width (ft)	12	12	1	HDM Index 301.1
Right Shoulder Width (ft)	8	8	J	HDM Index 302.1 & 308.1 / AASHTO
Curve Radii (ft)	N/A	550	1	HDM Index 203.2
Decision Sight Distance (ft)	600+	600	1	HDM Index 201.7
Horizontal Clearance (ft)	4'+ (w/out curb) & 1.5'+ (w/ curb)	4' (w/out curb) & 1.5' (w/ curb)	J	HDM Index 309.1
Stopping Sight Distance (ft)	300+	300	J	HDM Index 201.1
Design Vehicle	STAA <sup>3</sup>	STAA	1.1.1.1	HDM Index 404.4

<sup>1</sup> The southbound on ramp didn't have a speed limit sign, but was assumed to be 35 mph based on the northbound on ramp and ramp geometry.

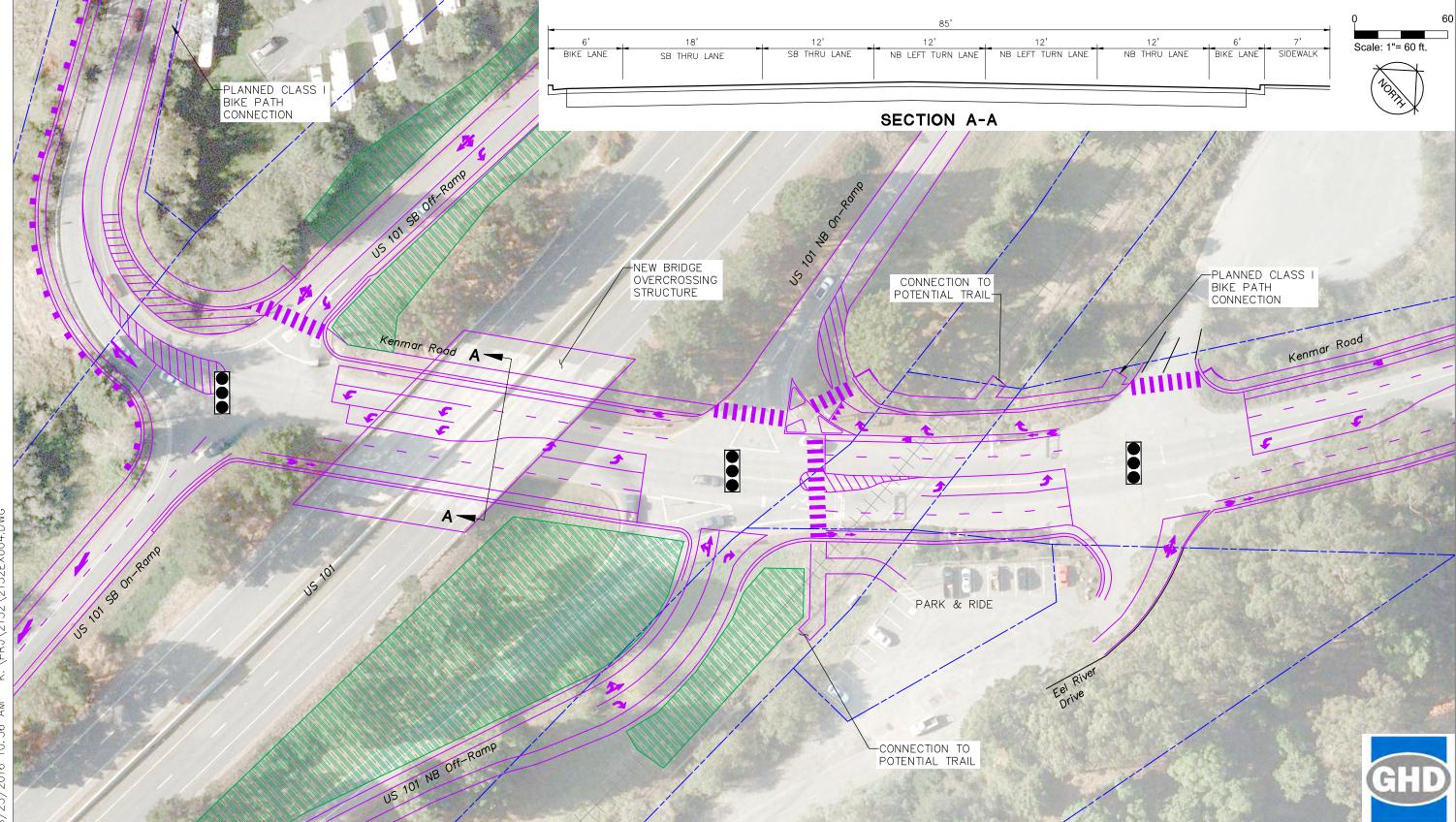
<sup>2</sup> Design speed should be 25 mph when traffic is expected to make a turning movement at the terminus and 50 mph when entering, exiting a ramp or when a "through" movement is provided at the terminus.

<sup>3</sup> An STAA truck could probably navigate the turns, but would be required travel outside its lane.

- Posted Speed/Design Speed The design speed of ramp can vary depending on the alignment and controls at each end. An acceptable approach is to set 25 mph and 50 mph design speeds for the ramp terminus and exit nose, respectively. The SB off ramp terminates at an intersection where traffic is expected to make a turning movement; therefore, the design speed should be 25 mph nearing this portion of the ramp.
- Design Vehicle In all cases, it appears as though an STAA truck could navigate the turns and stay within the pavement; however, due to the tight radii entering and exiting the ramps and turning on and off Kenmar Road, large trucks would need to encroach into the oncoming travel lane or gore area.

Attachment D - Conceptual Design Drawings

# **KENMAR Road INTERCHANGE SIGNAL CONCEPT**

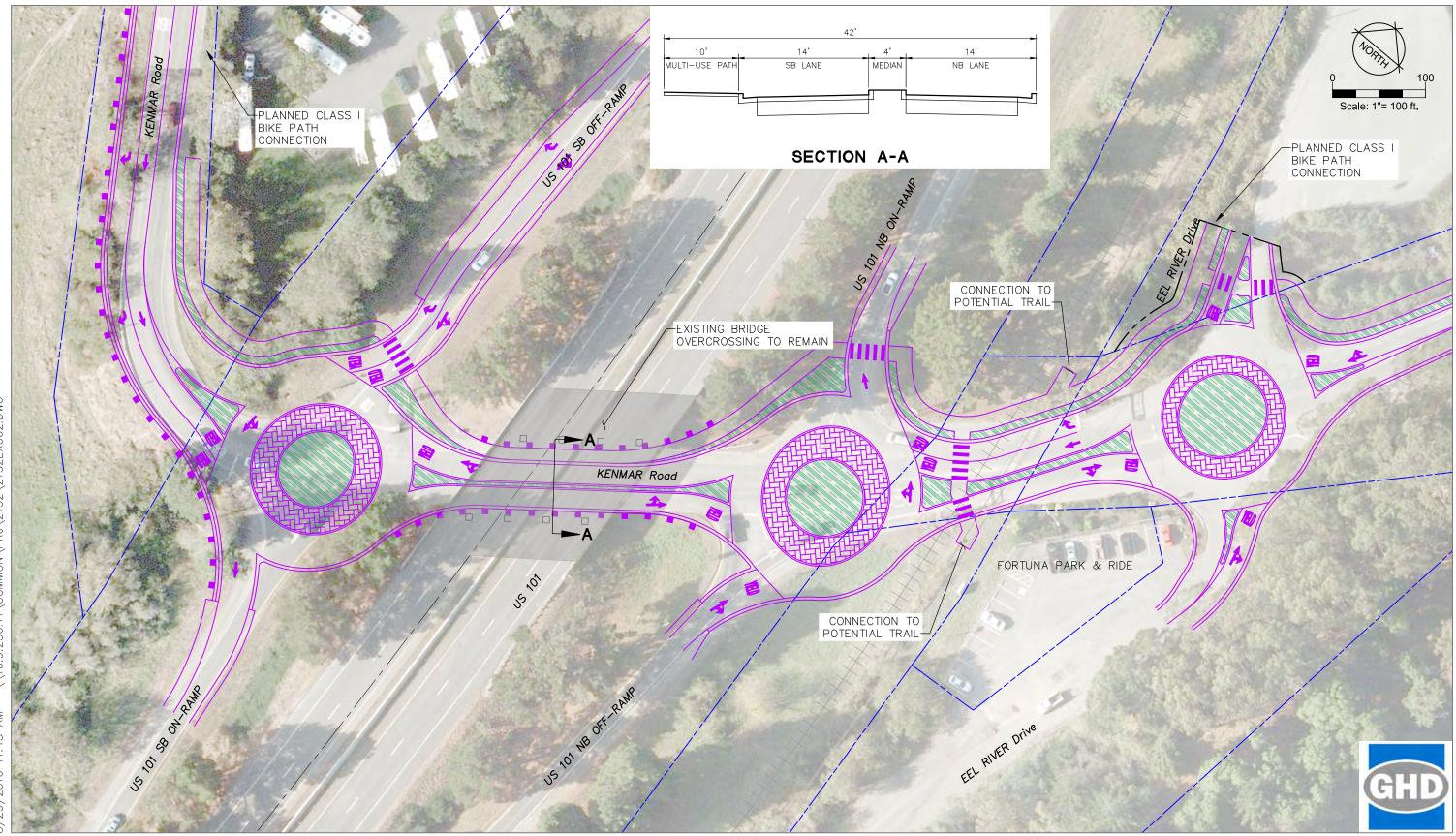


# **US 101/RIVERWALK AREA CONNECTIVITY PROJECT** Fortuna, California



August 26, 2016 2132EX004.dwg

# **KENMAR Rd. INTERCHANGE ROUNDABOUT CONCEPTS - Option 1a**

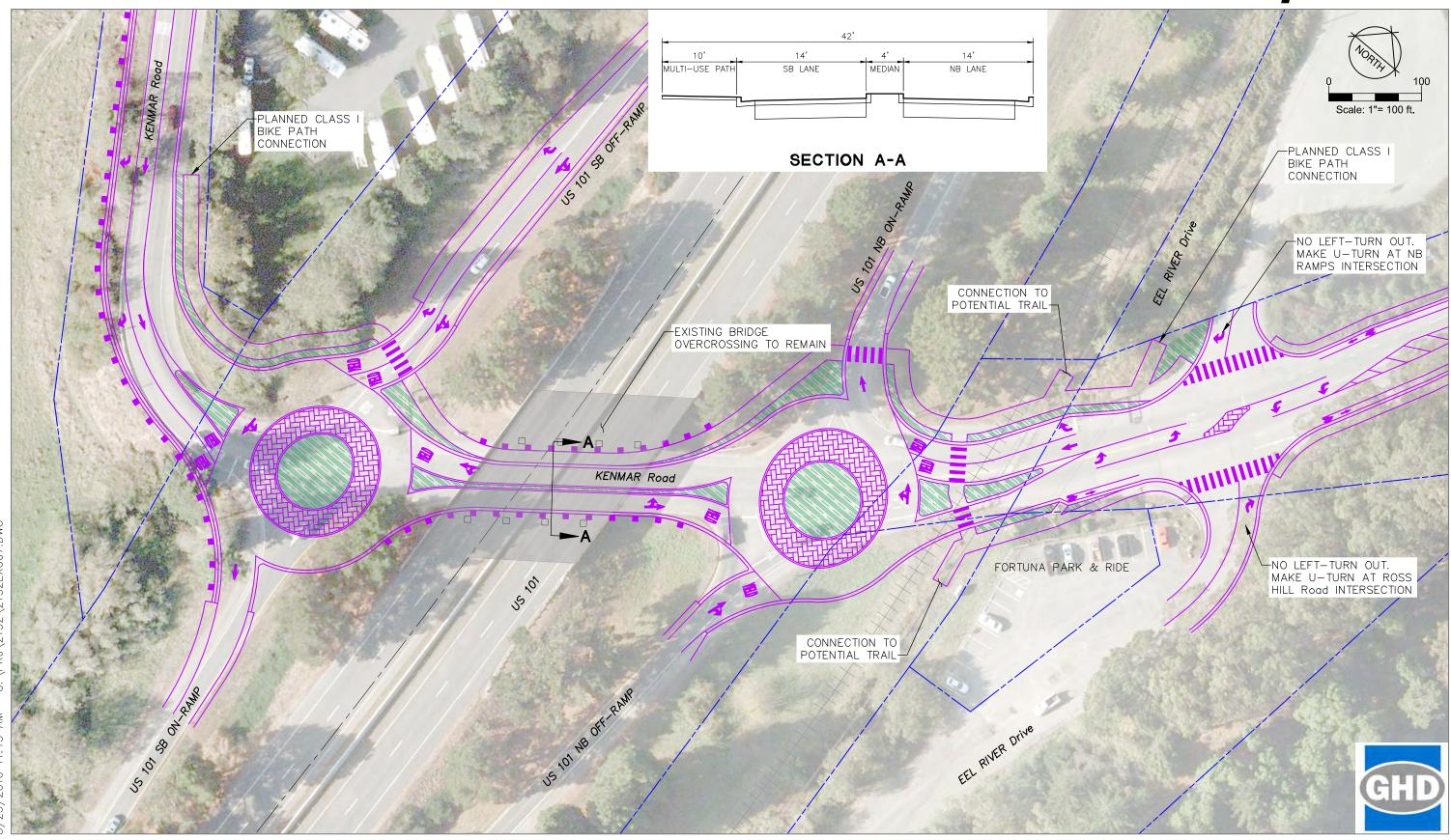


# **US 101/RIVERWALK AREA CONNECTIVITY PROJECT** Fortuna, California



August 26, 2016 2132EX002.dwg

# **KENMAR Rd. INTERCHANGE ROUNDABOUT CONCEPTS - Option 1b**



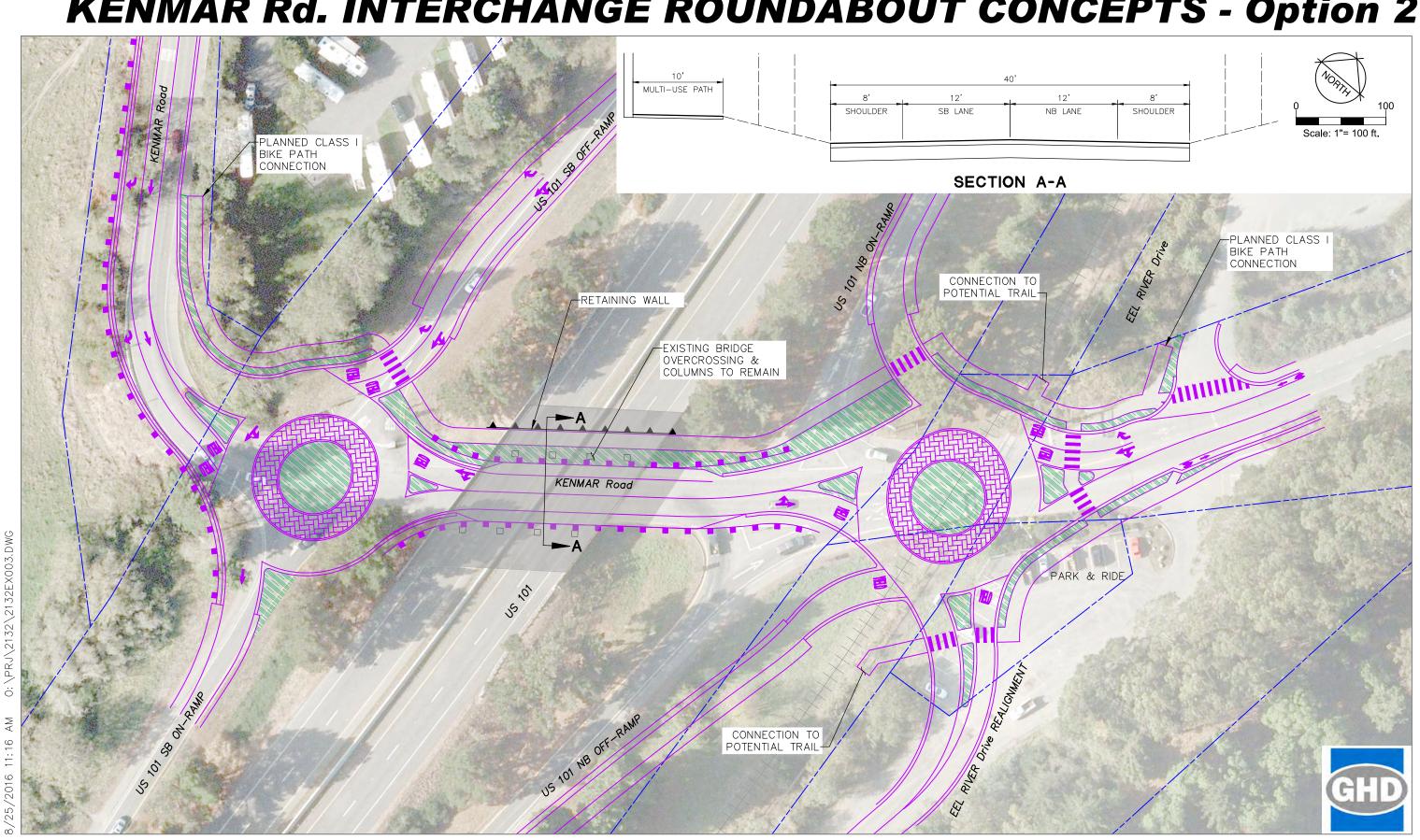
# **US 101/RIVERWALK AREA CONNECTIVITY PROJECT** Fortuna, California





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# **KENMAR Rd. INTERCHANGE ROUNDABOUT CONCEPTS - Option 2**



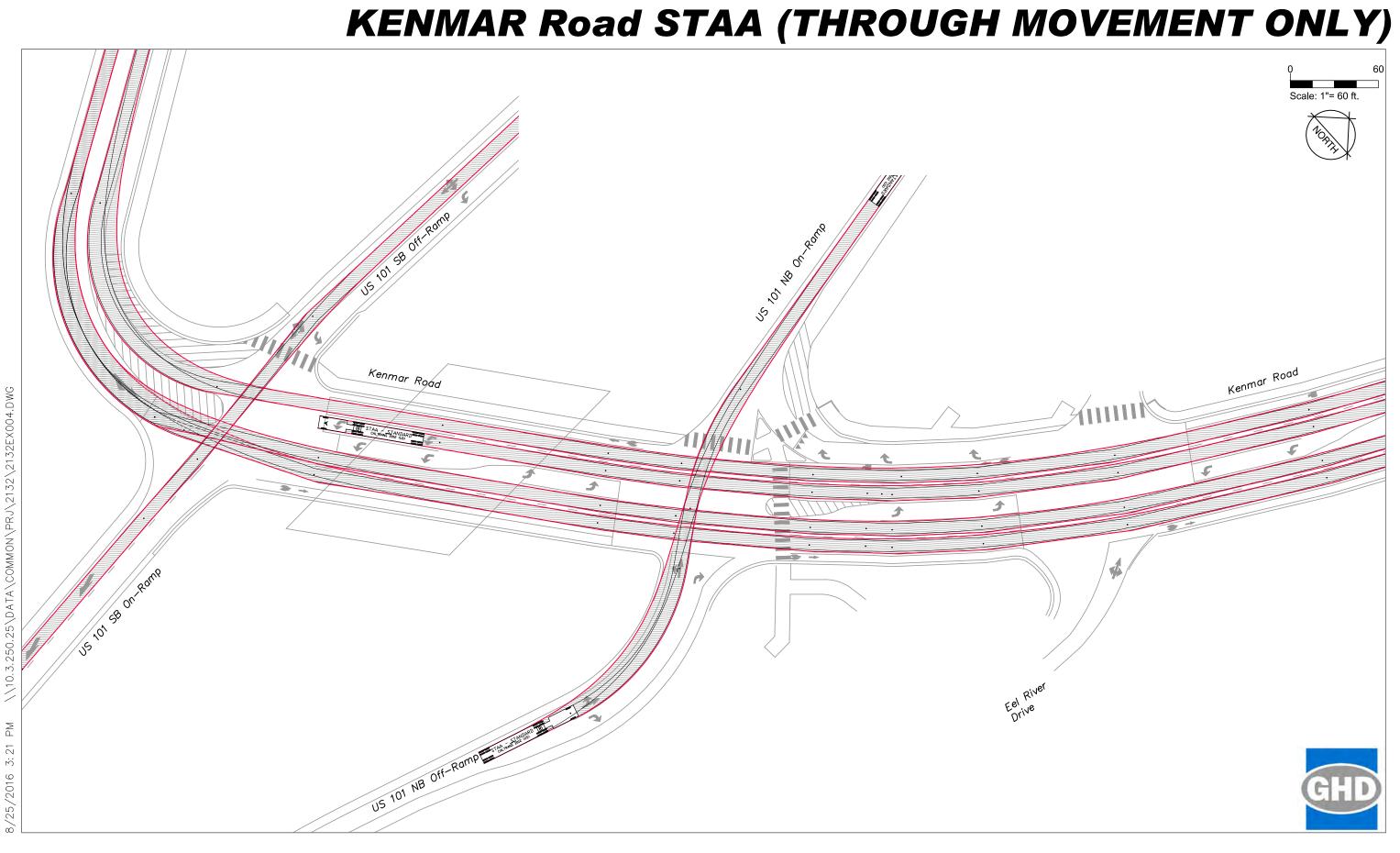
## US 101/RIVERWALK AREA CONNECTIVITY PROJECT Fortuna, California





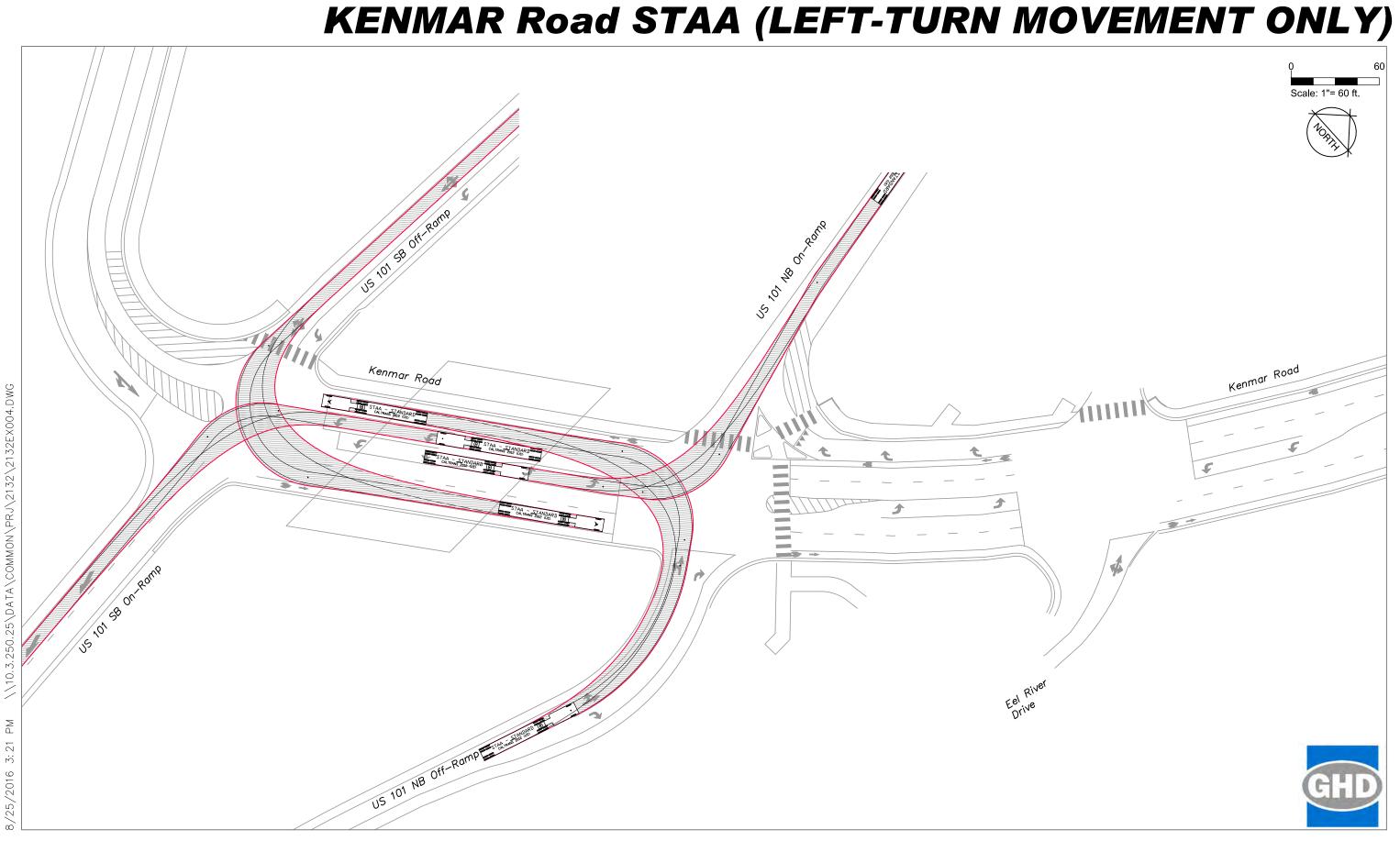
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Attachment E - Truck Turning Analysis



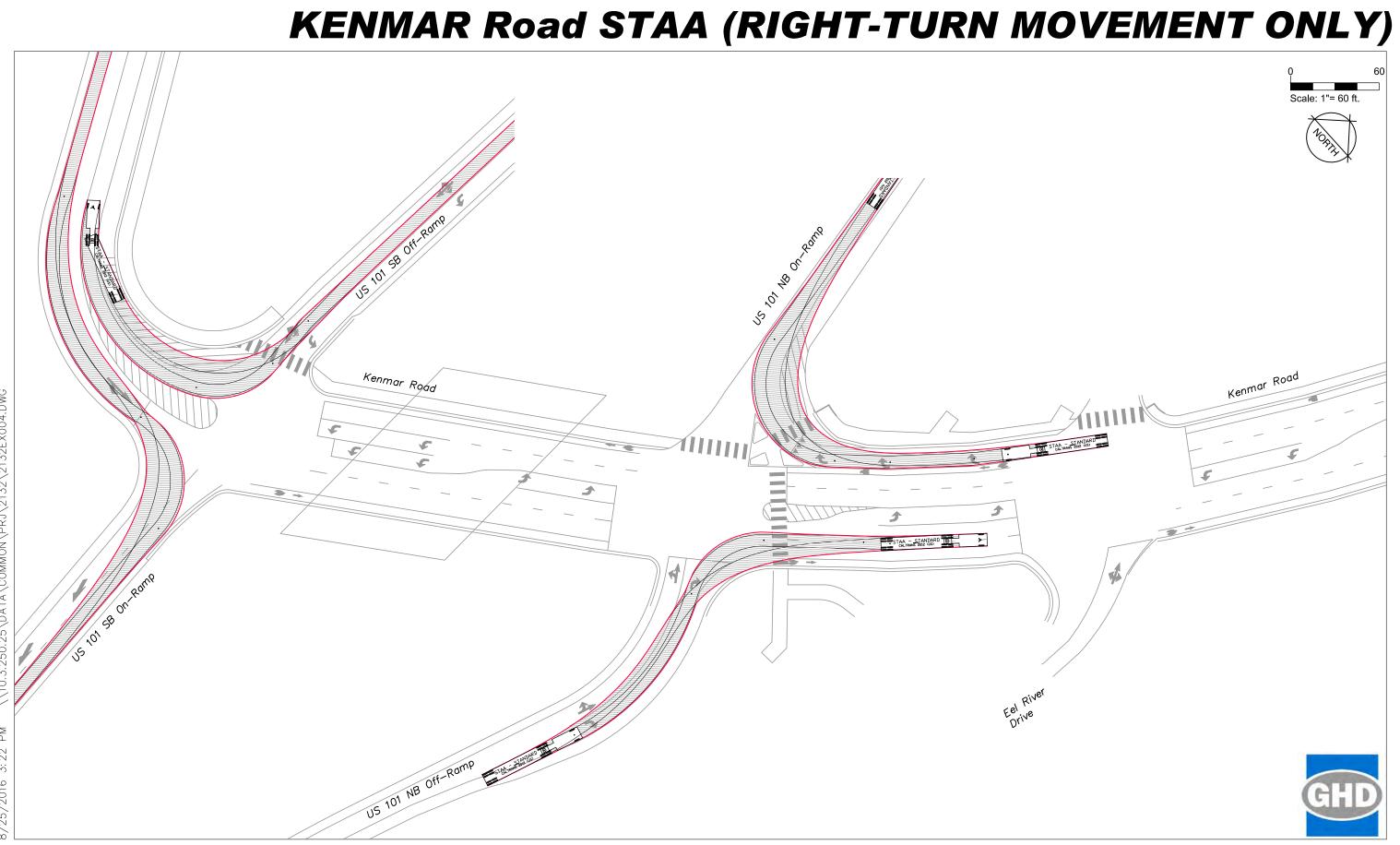
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Figure B1



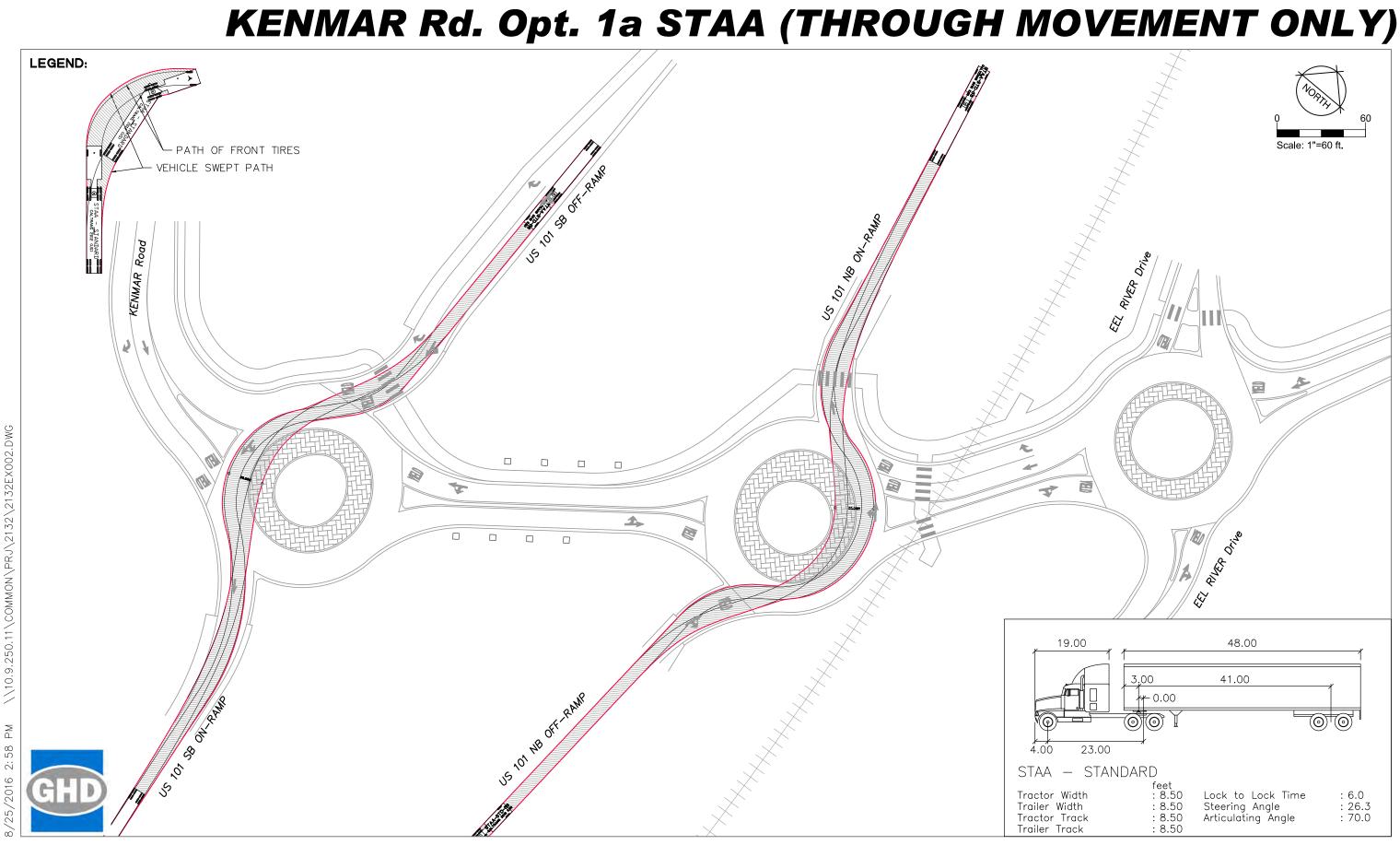


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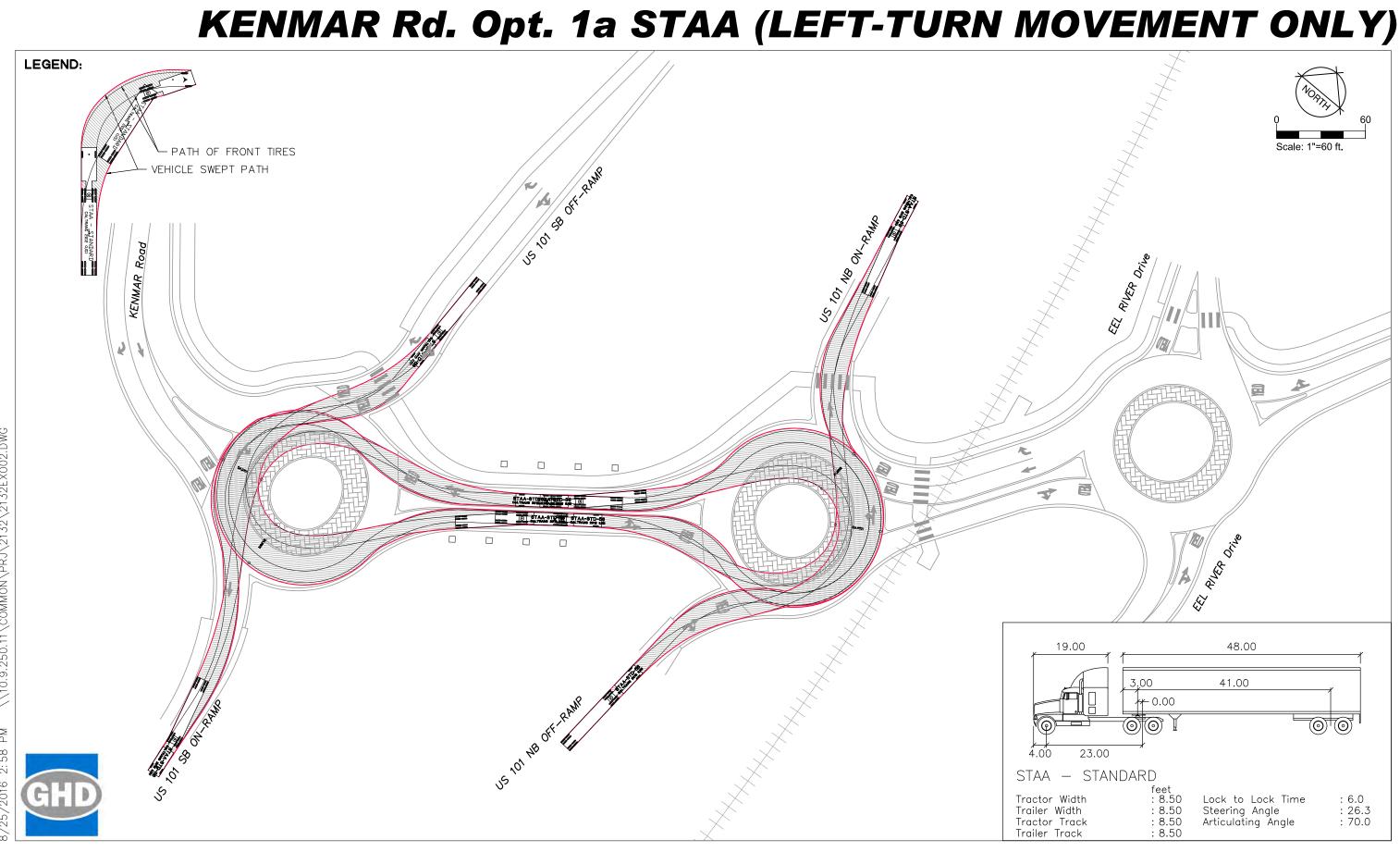
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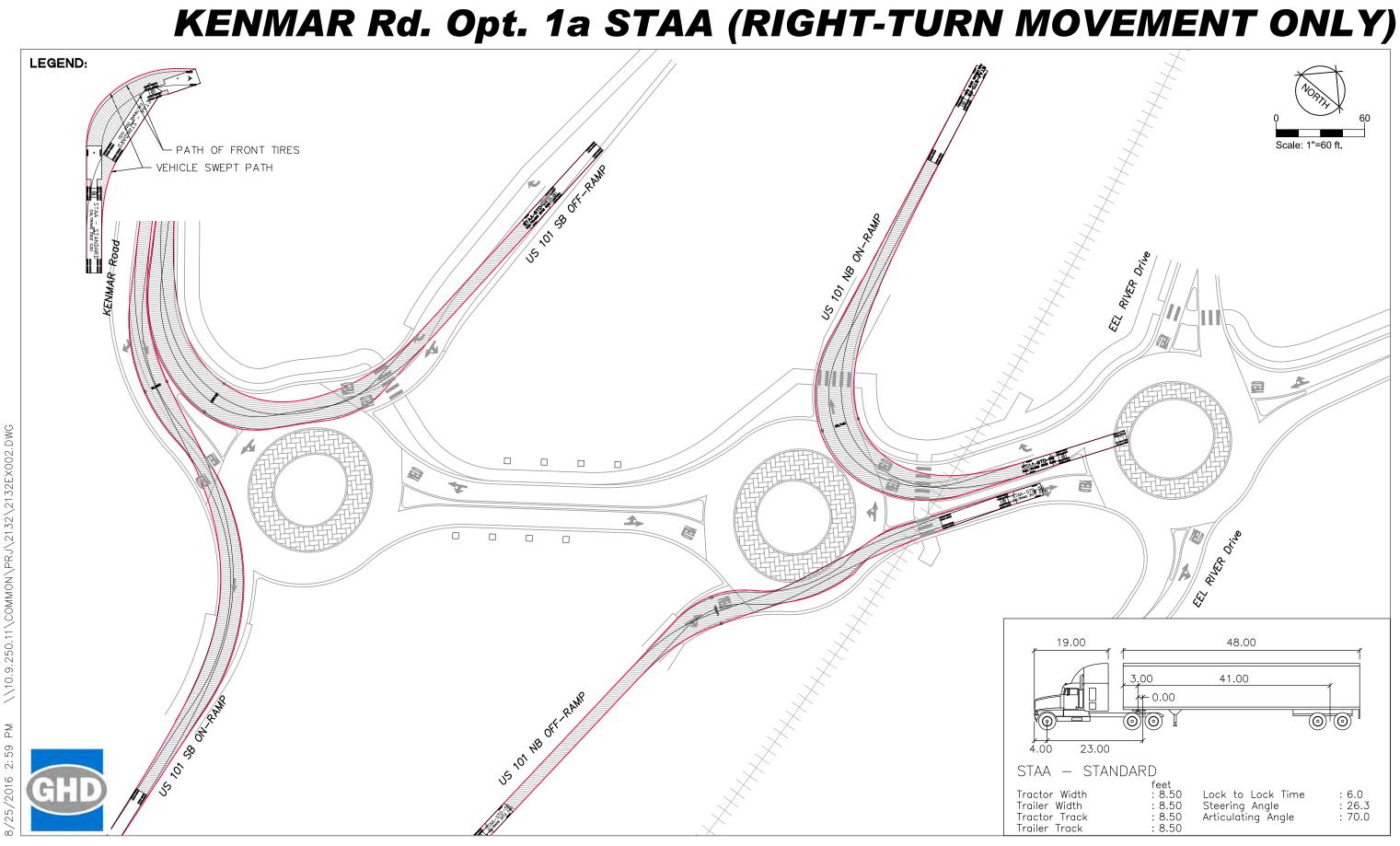




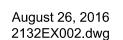


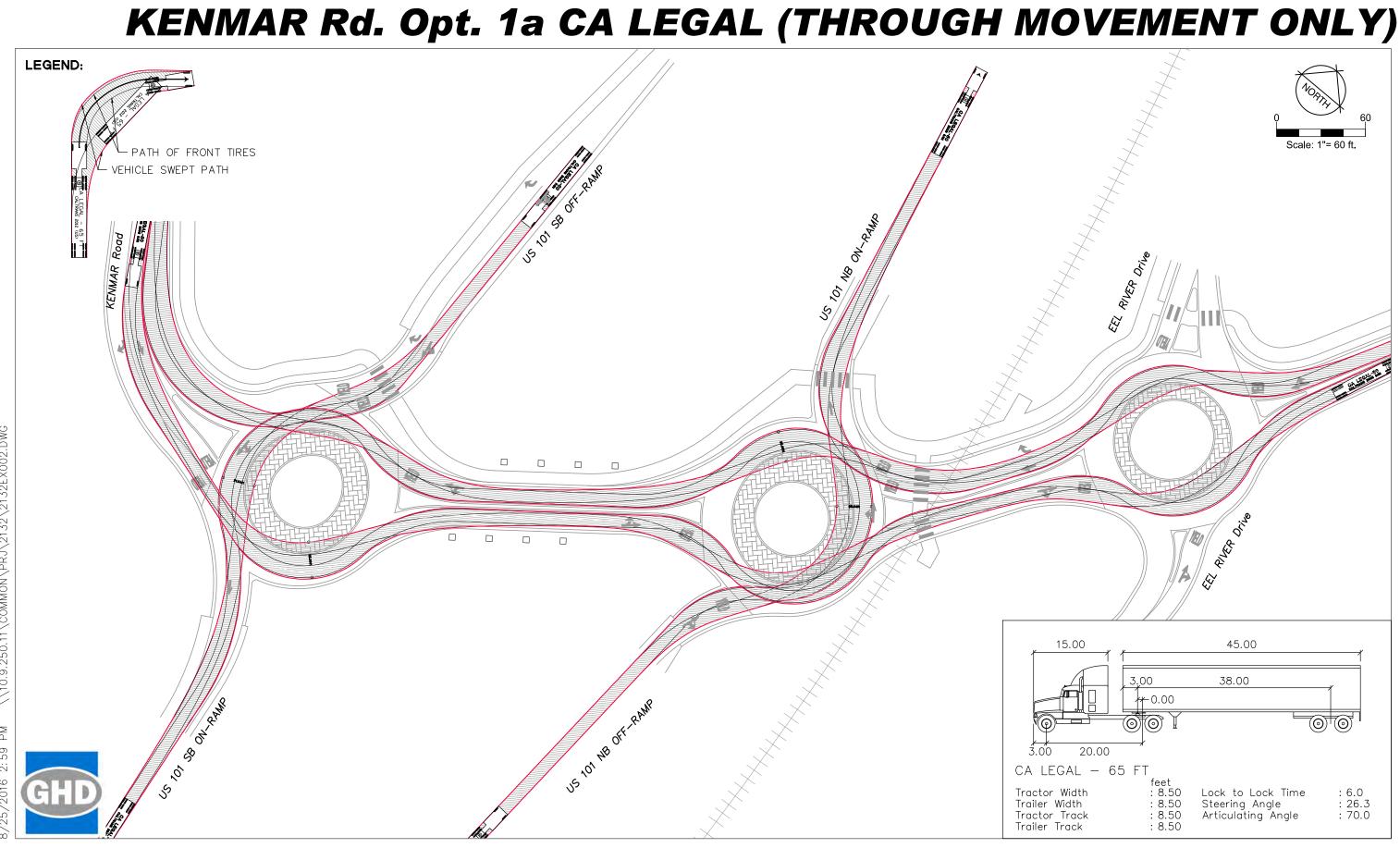
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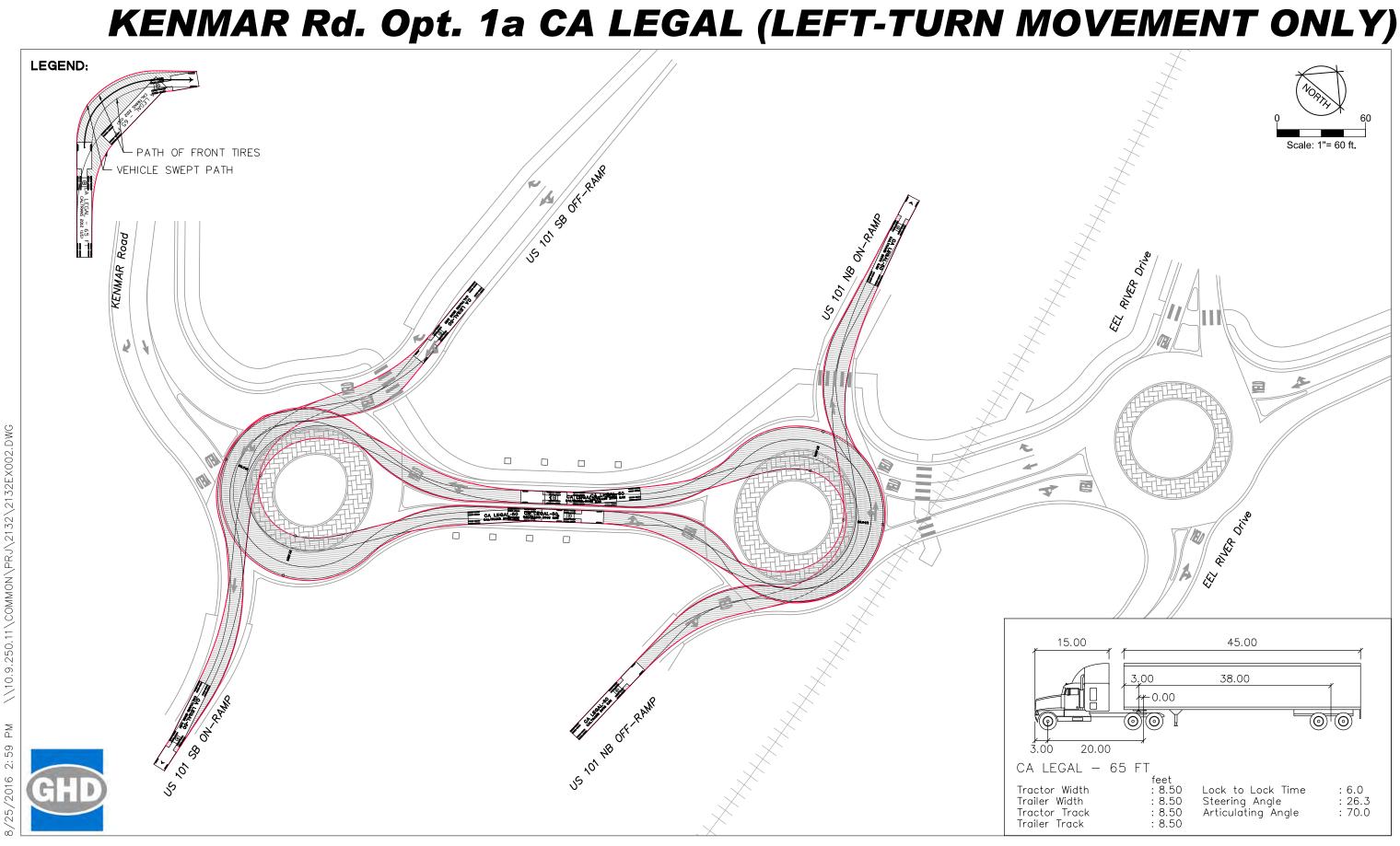




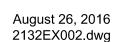


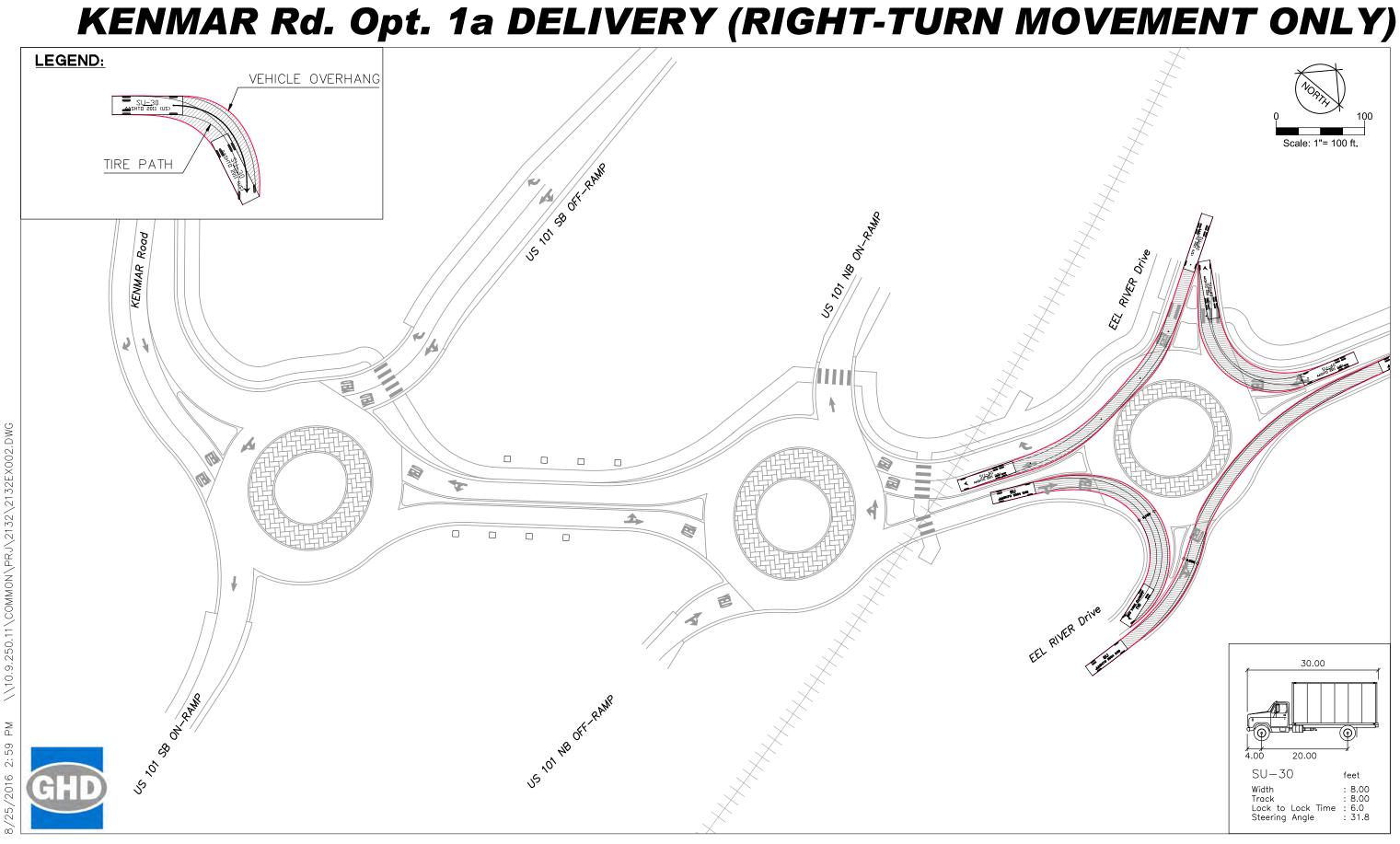


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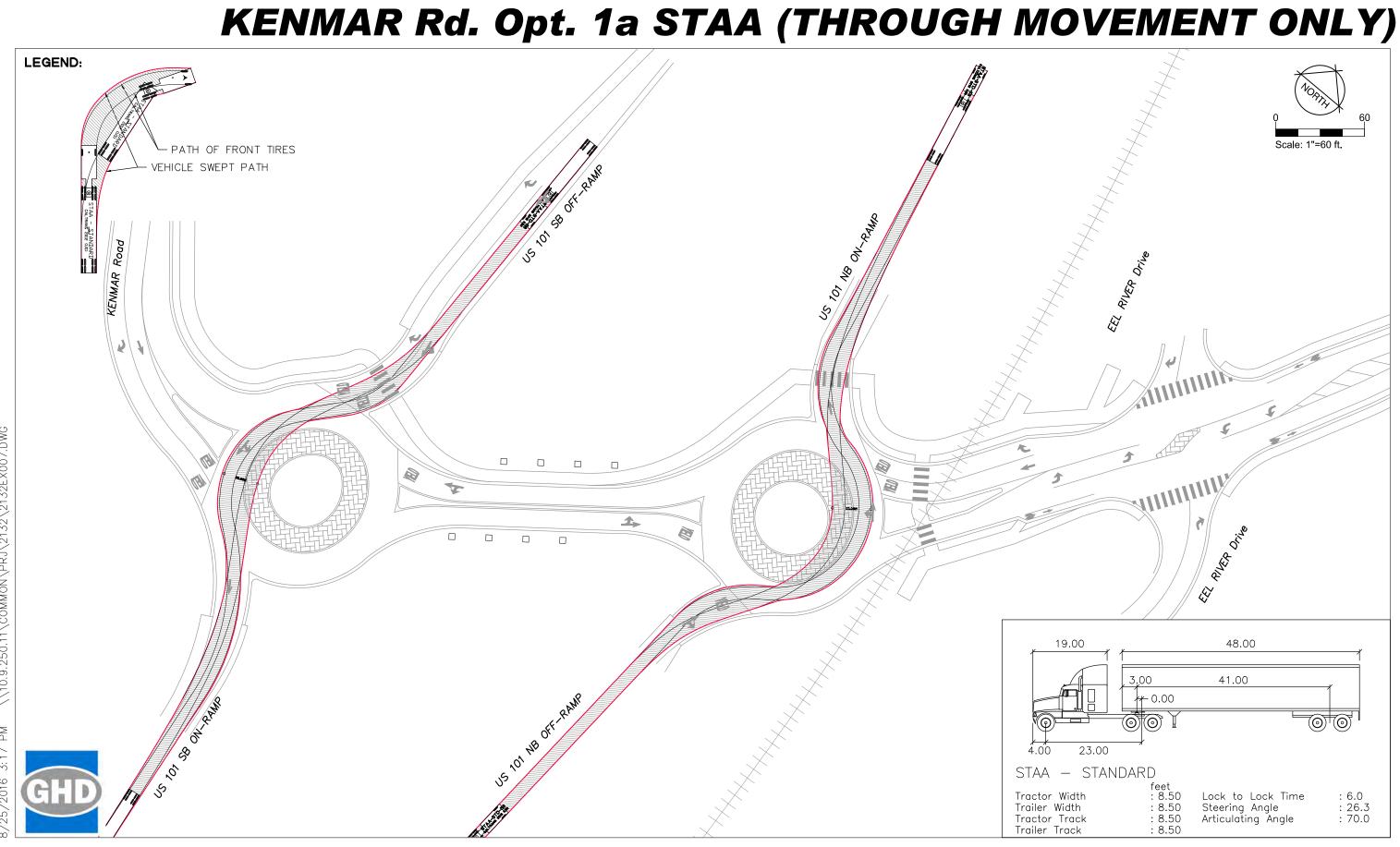




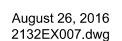


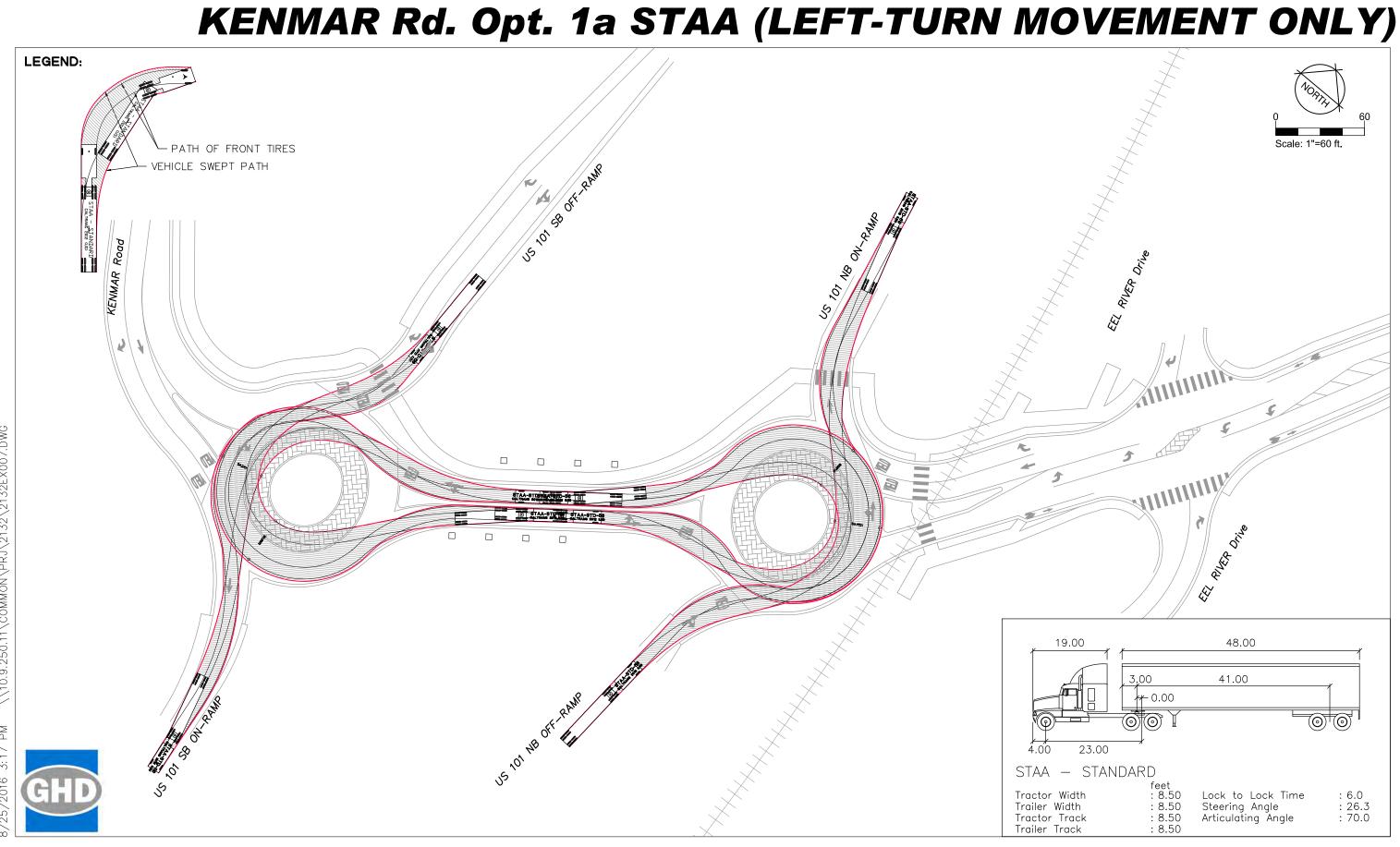


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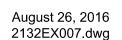


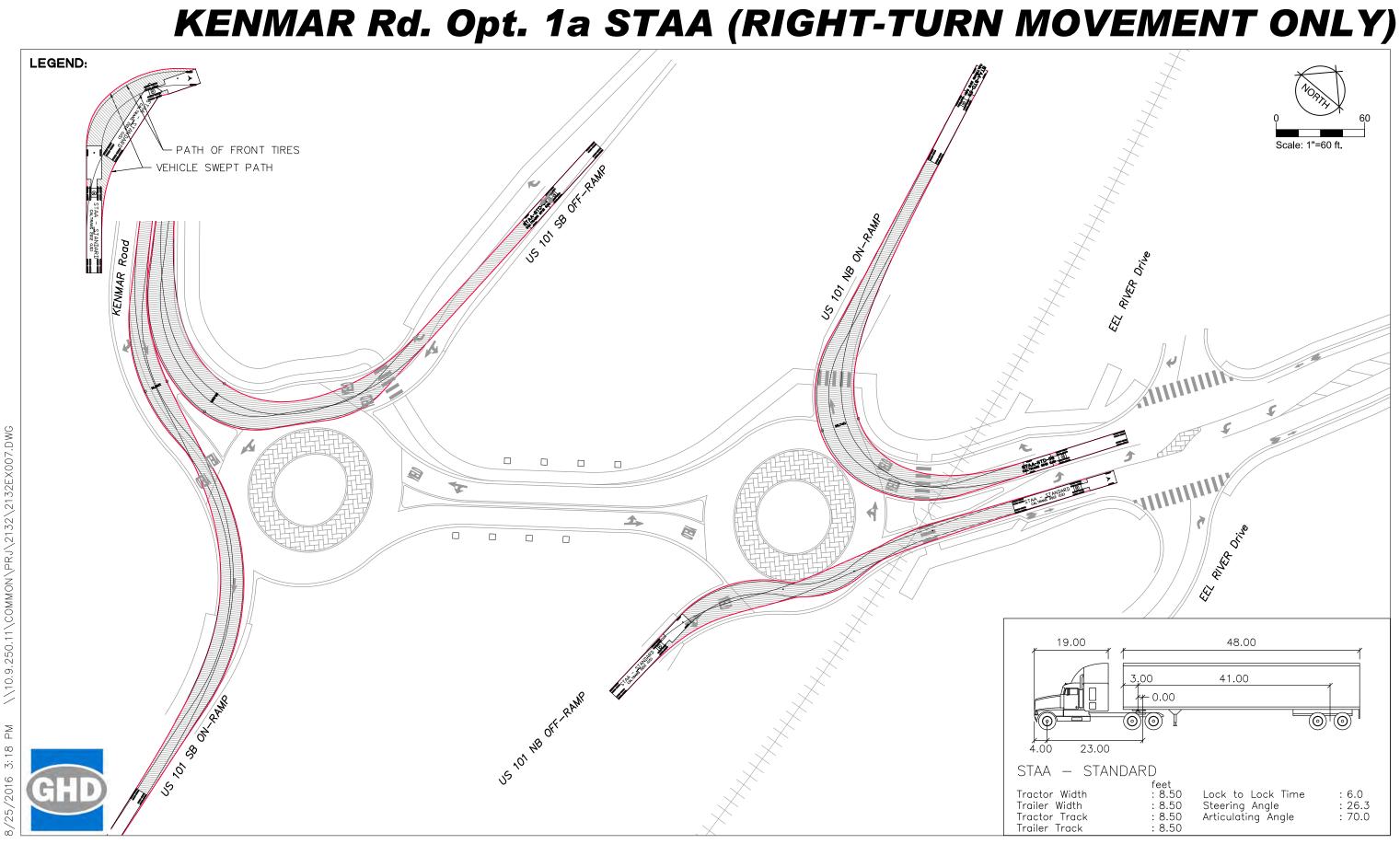




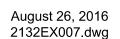


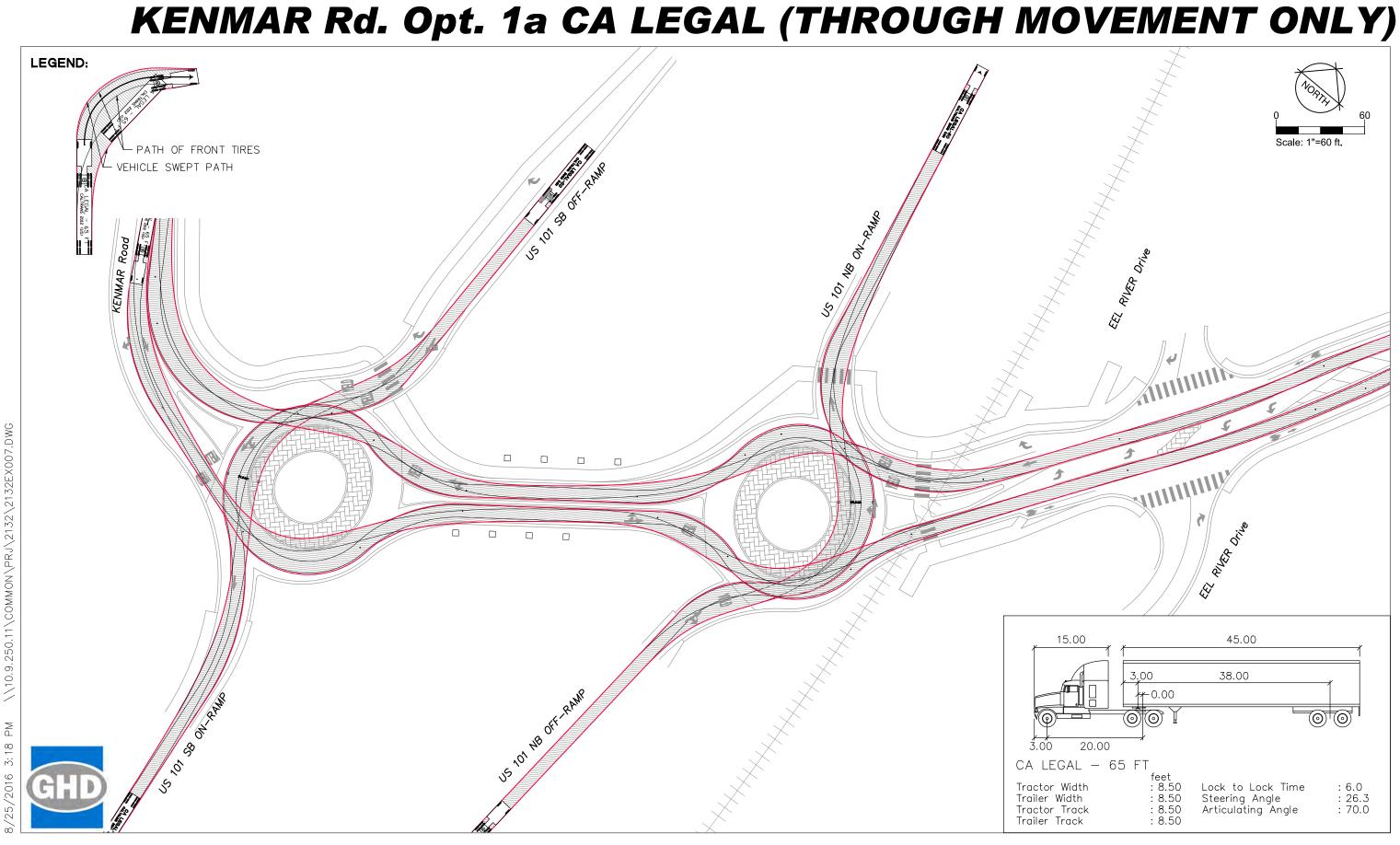




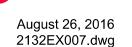


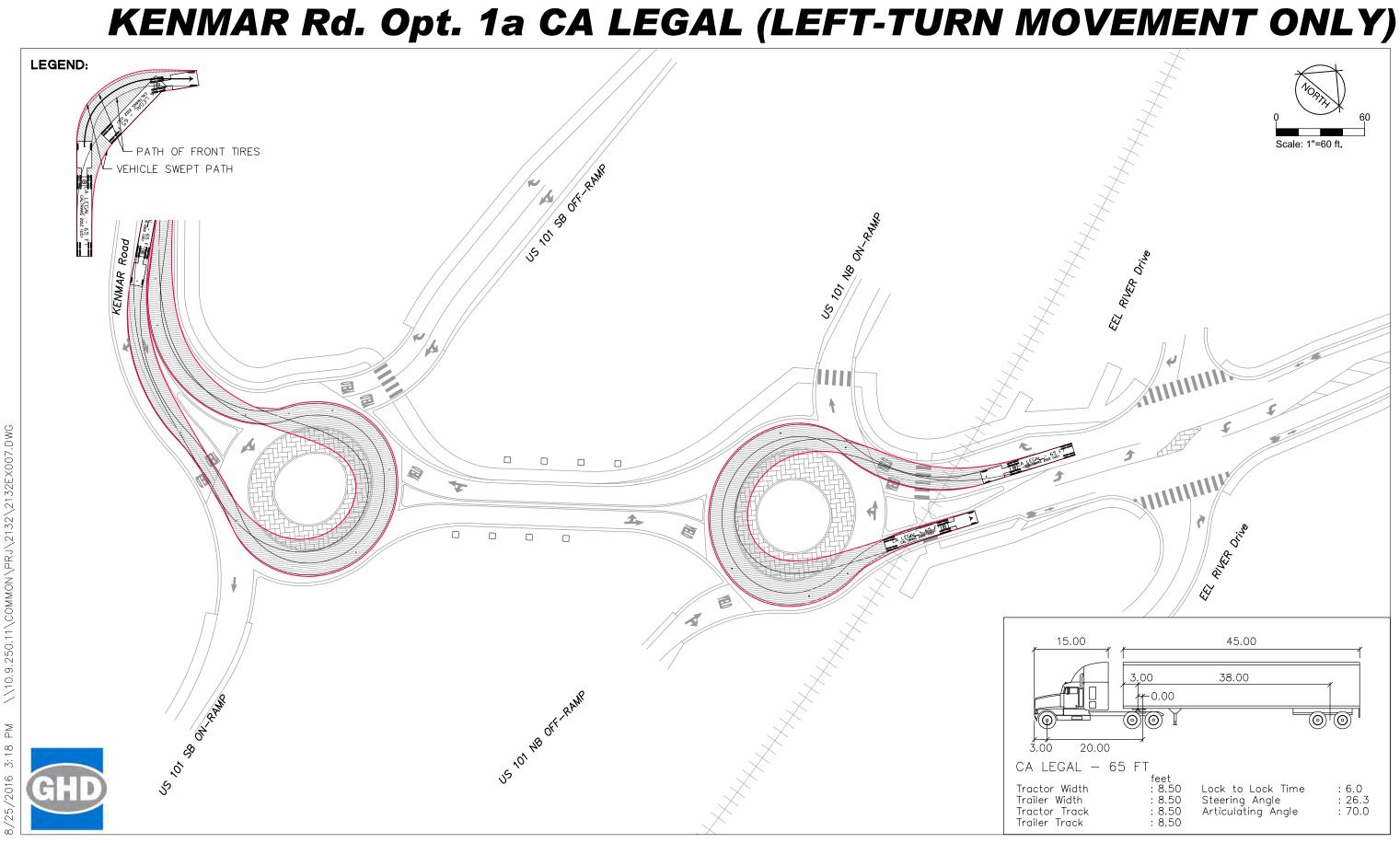




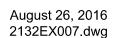


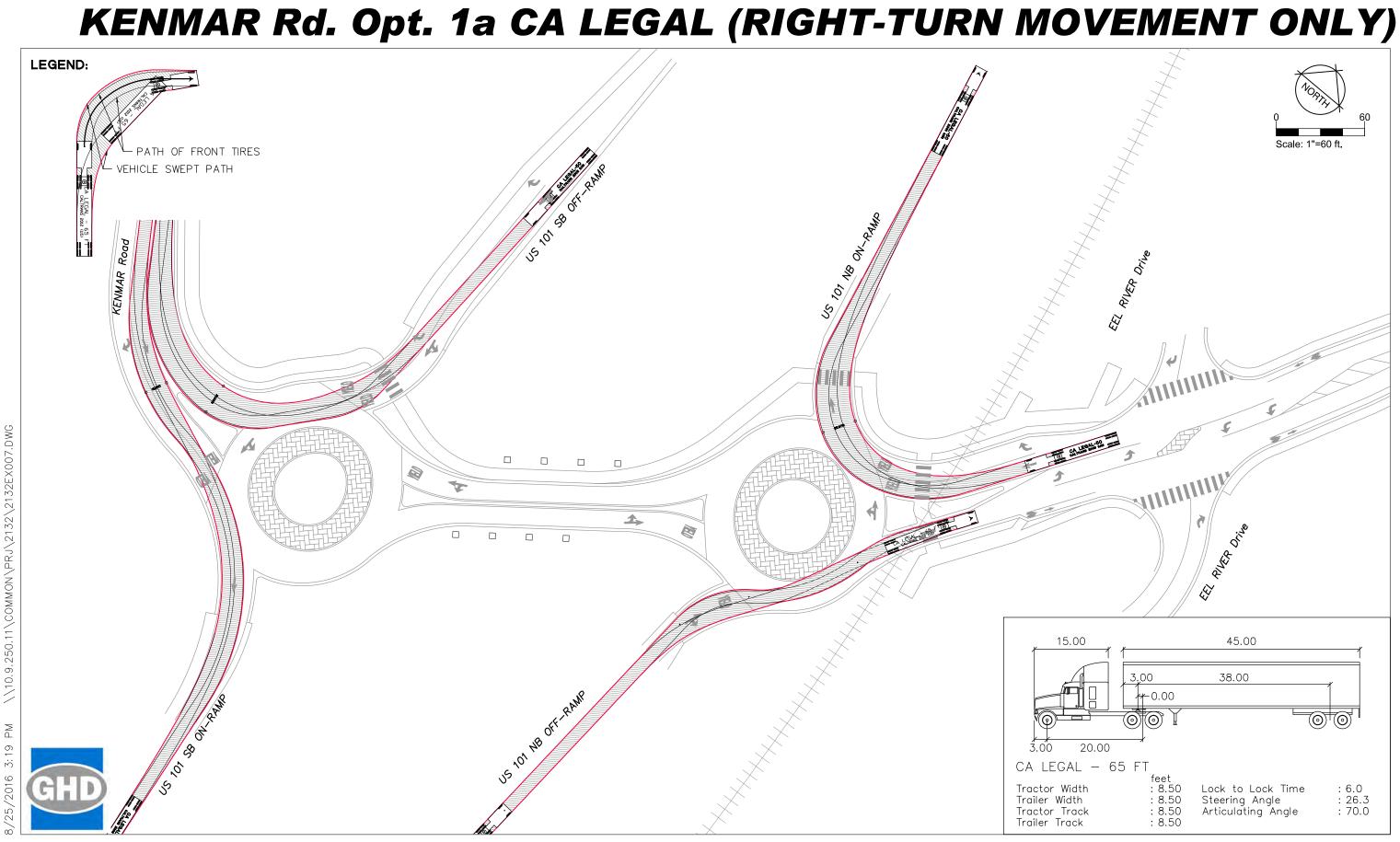




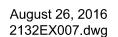


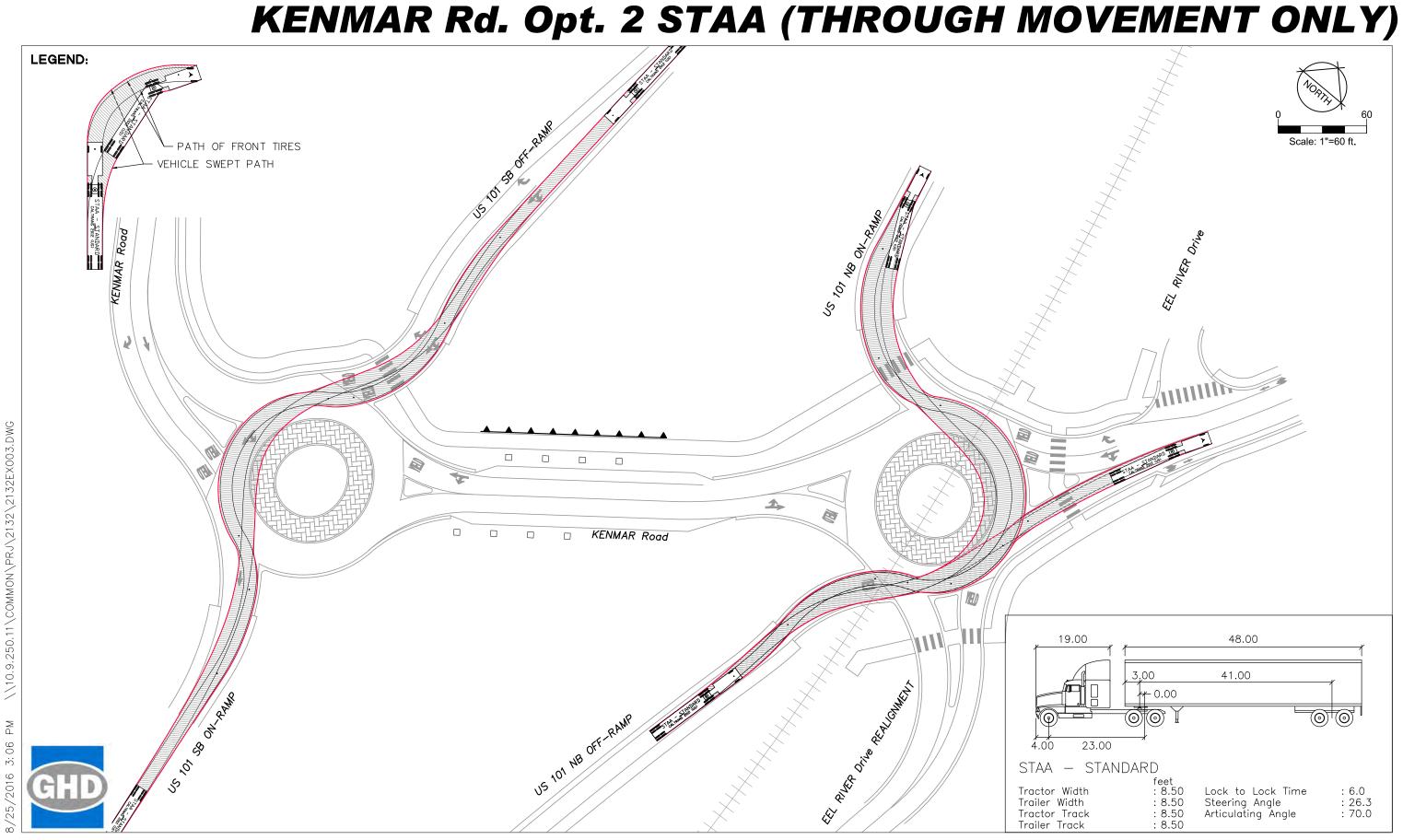




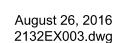


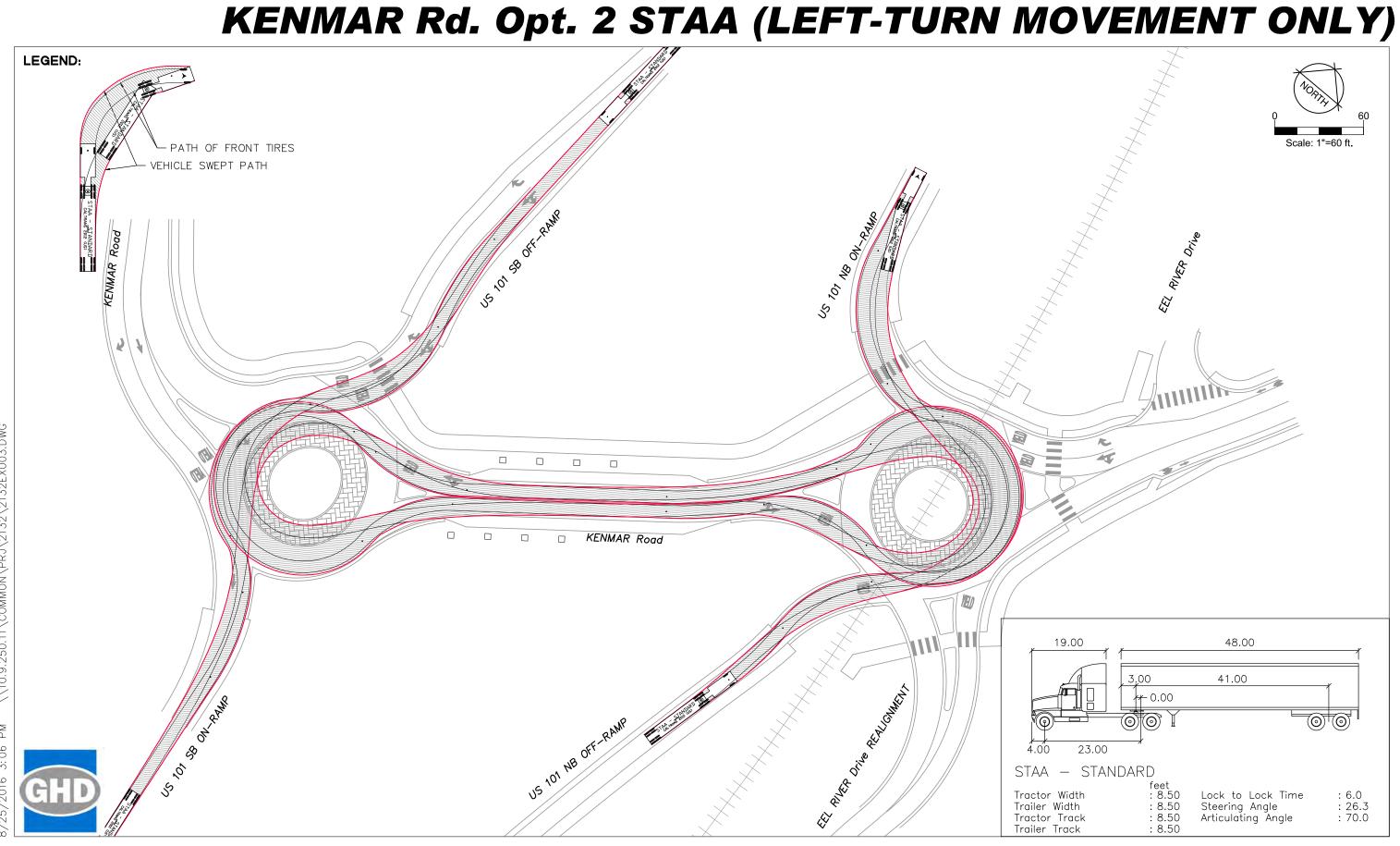




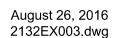


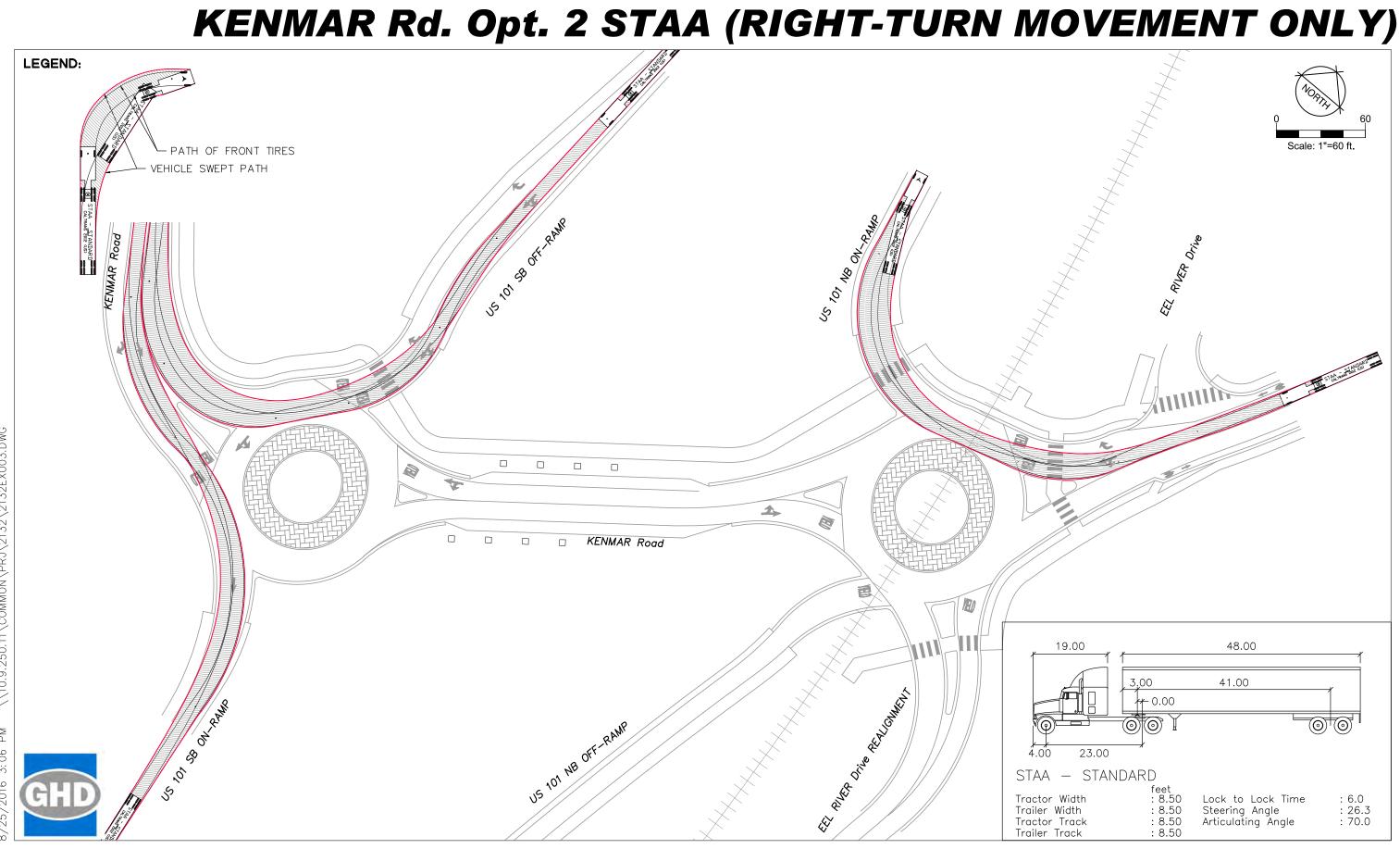




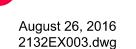


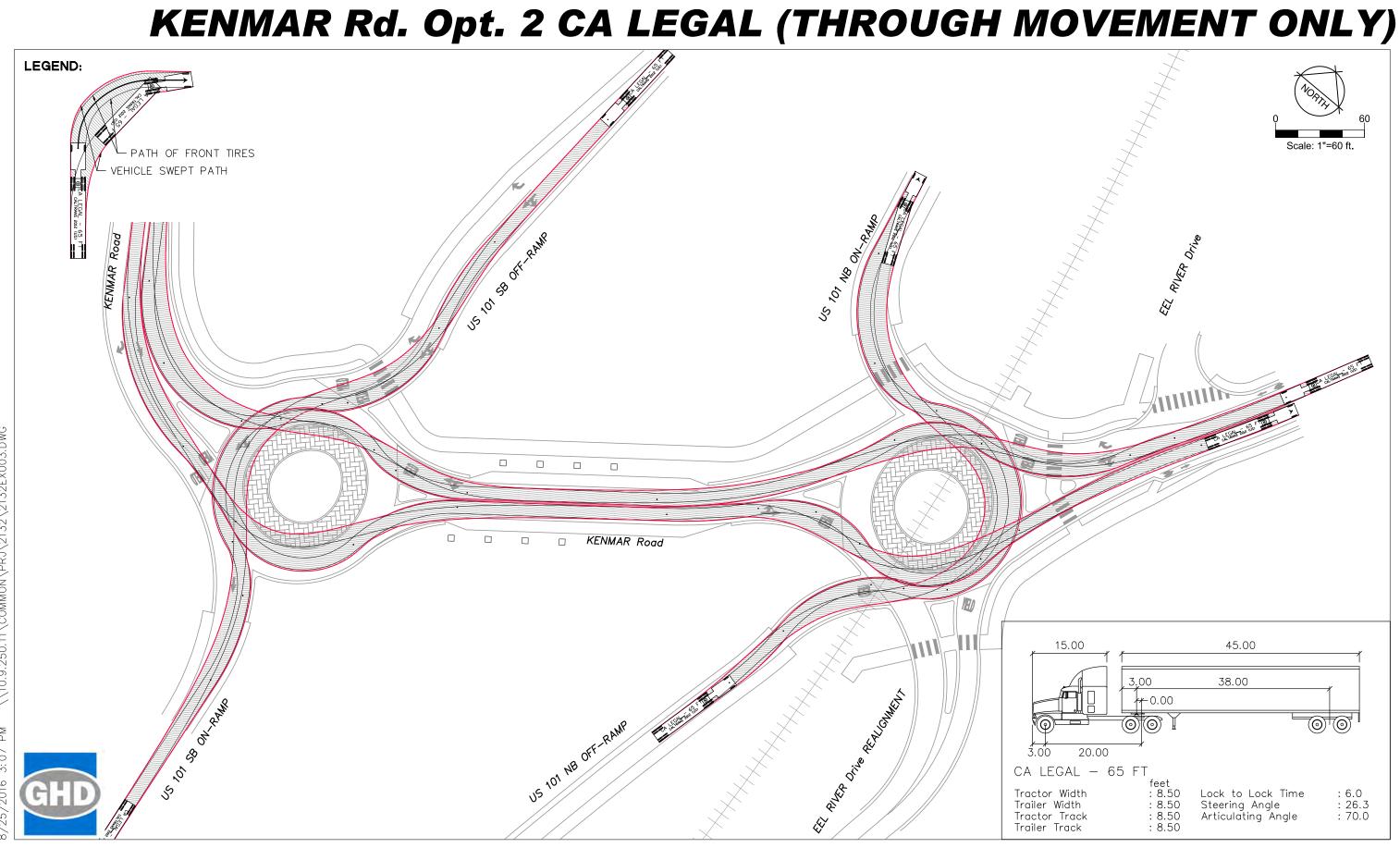




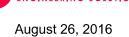






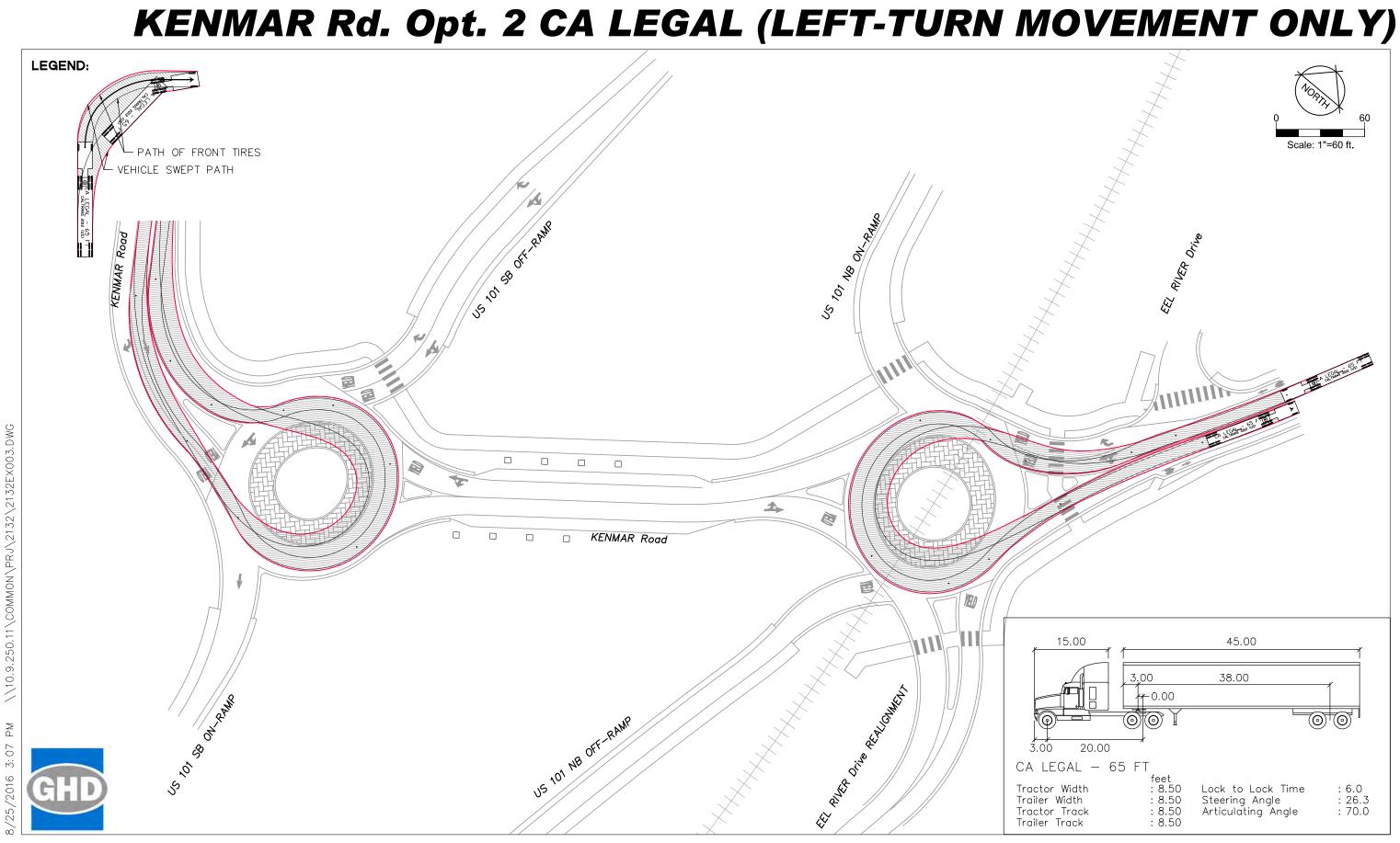




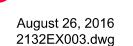


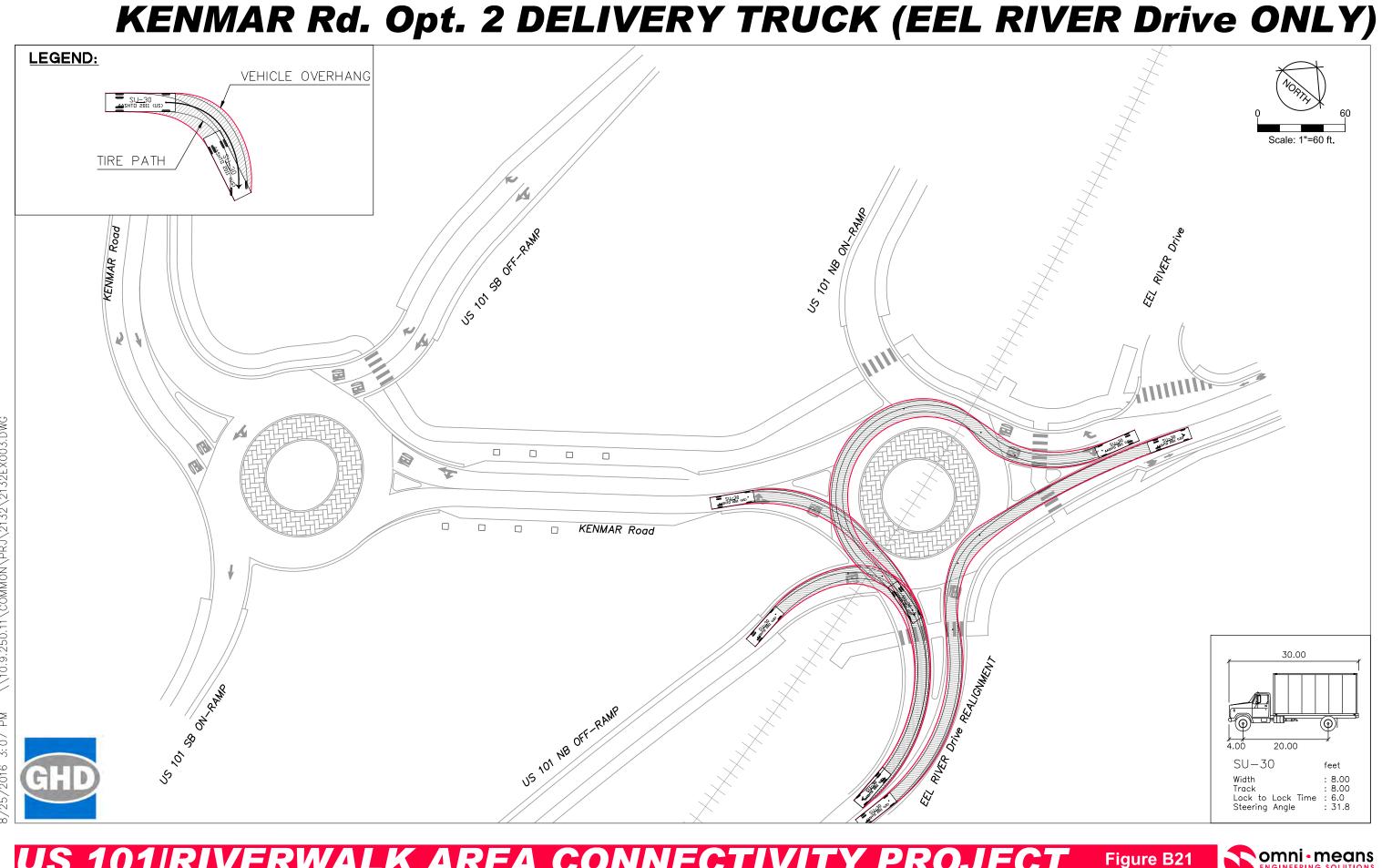
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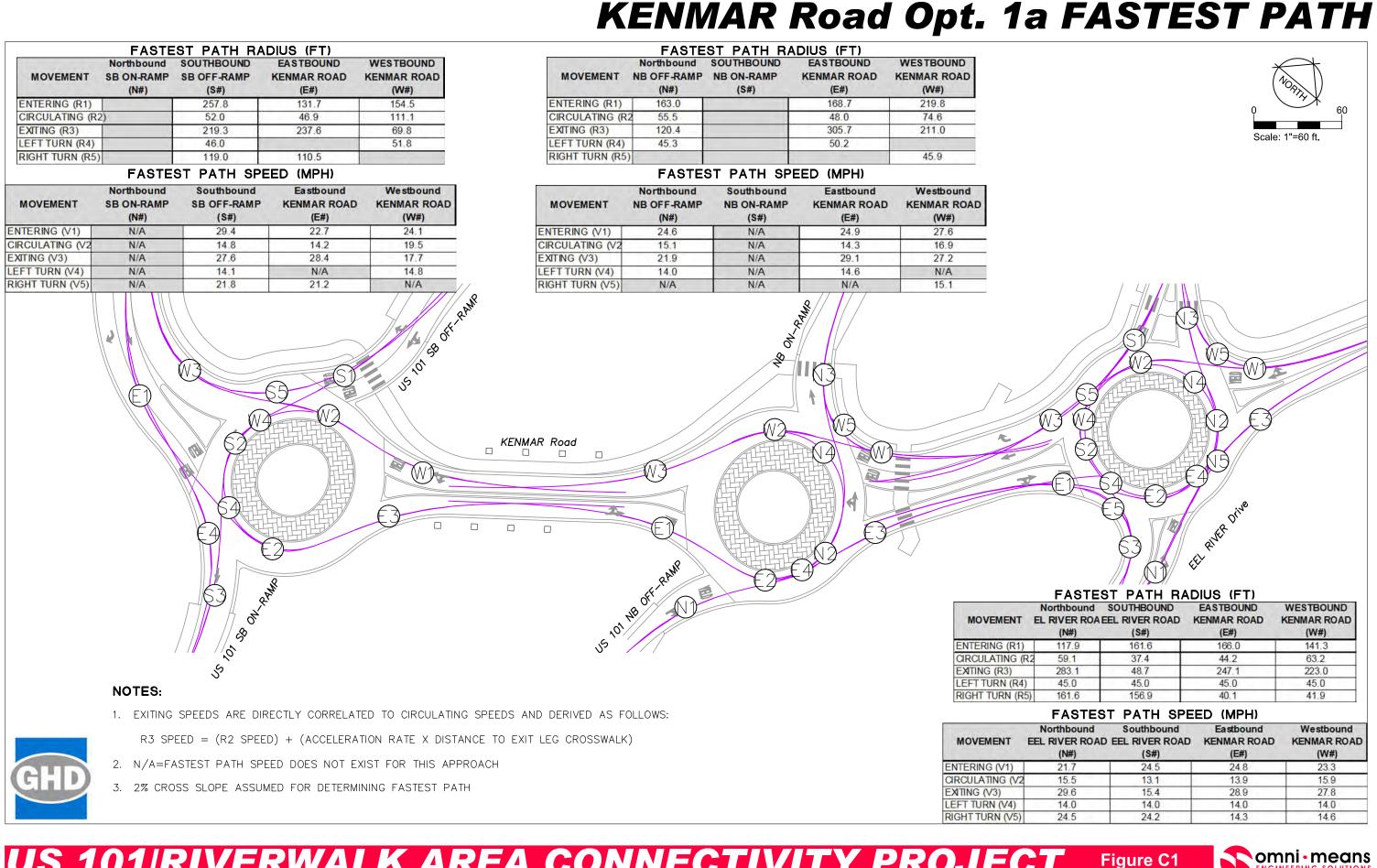




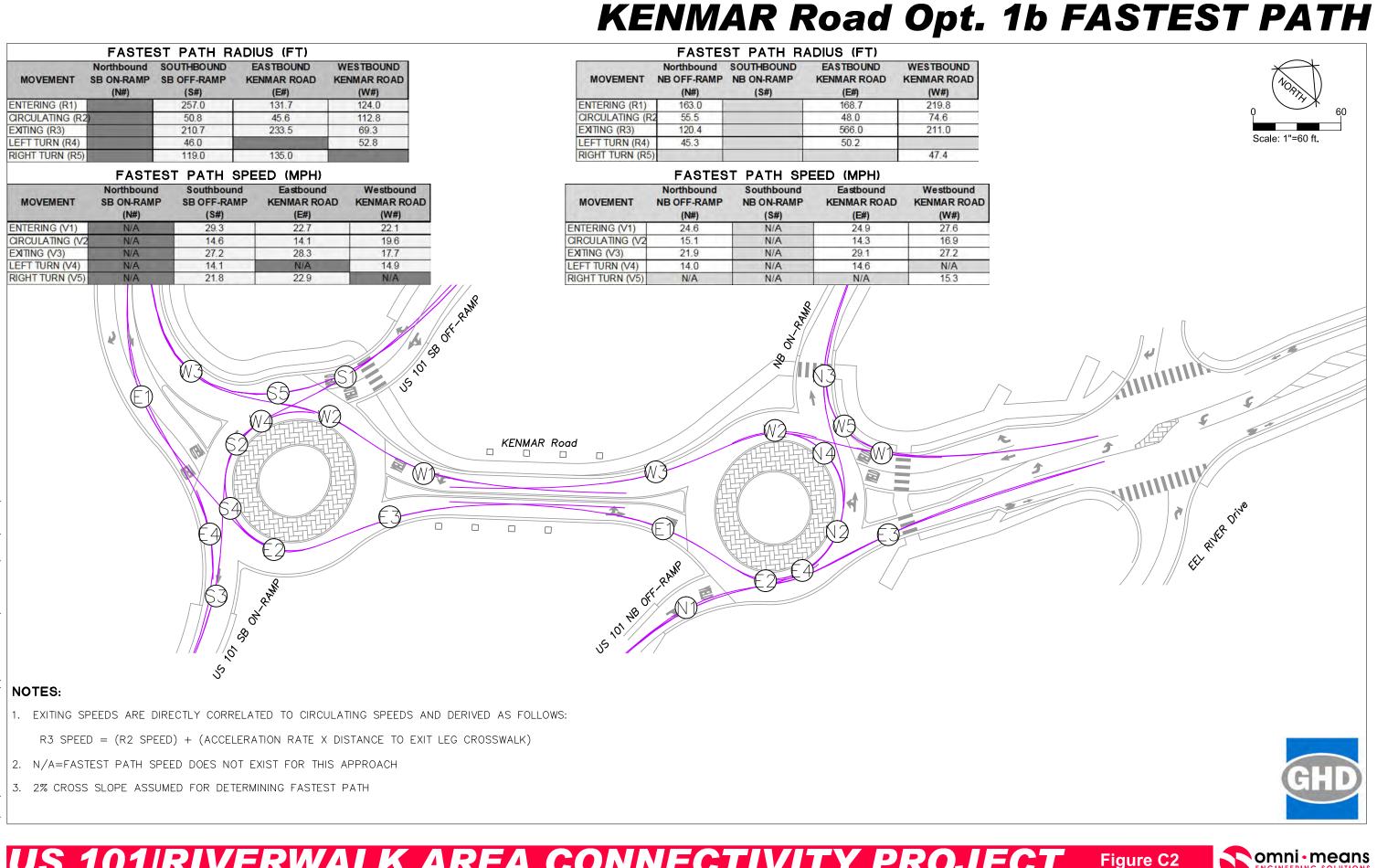


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**Attachment F - Fast Path Exhibits** 

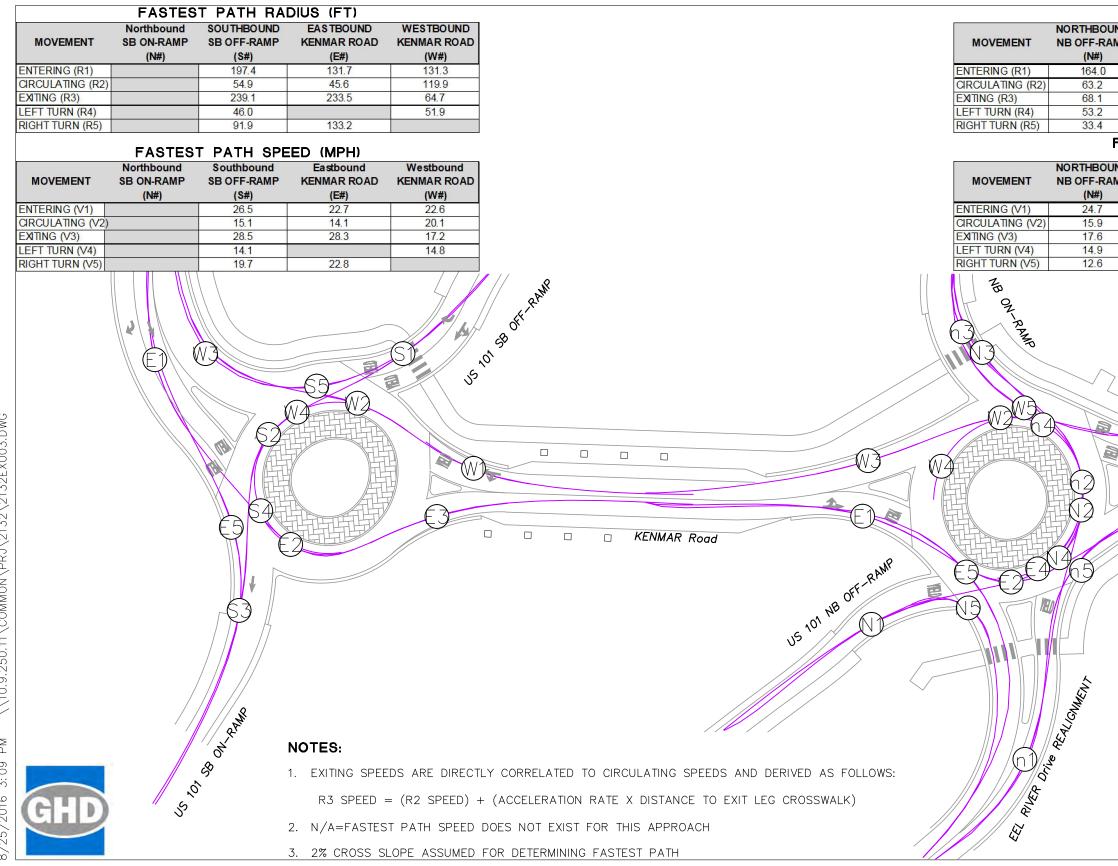


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# **KENMAR Road Opt. 2 FASTEST PATH**



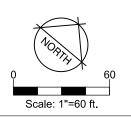
### US 101/RIVERWALK AREA CONNECTIVITY PROJECT Fortuna, California

FA	STEST PAT	H RADIUS (FT	-)	
ND	ORTHWES TBOUN	SOUTHBOUND	EASTBOUND	WESTBOUND
ΛP	EEL RIVER DRIVE	NB ON-RAMP	KENMAR ROAD	KENMAR ROAD
	(n#)	(S#)	(E#)	(W#)
	127.6		153.0	195.5
	63.2		50.5	98.1
	79.5		1000.0	381.5
	63.7		49.4	53.0
	81.6		112.0	81.1

### FASTEST PATH SPEED (MPH)

ND MP	NORTHWESTBOUND EEL RIVER DRIVE (n#)	Southbound NB ON-RAMP (S#)	Eastbound KENMAR ROAD (E#)	Westbound KENMAR ROAD (W#)
	22.4		24.0	26.4
	15.9		14.6	18.7
	18.6		29.2	31.4
	15.9		14.5	14.9
	17.4		19.6	17.4

EEL RIVER





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Attachment G - Preliminary Structures Analysis

### **Morrison Structures**

1890 Park Marina Drive, Ste 104 Redding, CA 96001

### **Structure Memorandum**

From: Bob Morrison, Jr., S.E., Morrison Structures, Redding, California

To: Josh Wolf, P.E, GHD, Eureka, California

Date: September 15, 2016

Re: Highway 101 Fortuna Downtown and Riverwalk Area Complete Streets and Connectivity Planning Study U.S. 101/Kenmar Road Undercrossing HUM-101-59.50

### **General**

The purpose of this memorandum is to provide structure information for the proposed alternatives for the Fortuna-Kenmar Road Undercrossing Interchange Improvements. The level of study we have conducted is a Project Study Report-Project Development Support (PSR-PDS) Cost Estimate. The purpose of our study was to determine preliminary scope, feasibility, rough cost range, and a list of potential project risks for the proposed structures work.

The Route U.S. 101 spans over Kenmar Road on a bridge (Kenmar Road Undercrossing, Br. No. 04-0128) at the interchange. The bridge is skewed approximately 34 degrees to the right and is a 3-span, 133-foot-long, concrete tee-beam structure, with a span arrangement of 34, 64, and 34 feet. The structure was constructed in 1962. End supports are diaphragm abutments on concrete pile foundations, and intermediate supports are 4-column bents on concrete pile foundations. The structure is in good condition with sufficiency rating equal to 98 and health index equal to 100. Kenmar Road currently passes under the 65 foot main span with a 14-foot 10-inch vertical clearance. The 40-foot-width of Kenmar Road currently accommodates two 12 foot travel lanes and two 8-foot shoulders. There are no sidewalks along either side of Kenmar Road.



Kenmar Road Undercrossing Looking West

### <u>Alternative 1 – Signal Concept for Kenmar Corridor (*Replace Kenmar Road Undercrossing at US* <u>101/Kenmar Road Interchange)</u></u>

The proposed Alternative 1 improvement intends to add traffic signals and improve Kenmar Road in the City of Fortuna by widening the roadway, maintain profile grade, and adding a pedestrian sidewalk along the north side of the roadway. The widening would accommodate five 12-foot traffic-lanes, 5-foot shoulders each side of the roadway and a 7-foot-wide sidewalk along the north side of the road. The overall width of Kenmar Road improvement is approximately 77 feet including the sidewalk. In order to provide for widening and improving Kenmar Road to this extent, it will be necessary to replace the existing 3-span undercrossing. The existing bridge is in fair condition, however its' main span is insufficient dimension to accommodate the Kenmore Road improvements.

Based on the conditions at the site and the interchange geometrics, the new undercrossing will be a singlespan, approximately 114 feet in length. The most economical structure type will likely be a precast, prestressed, concrete girder structure with a 6-foot structure depth. Supports would be high-cantilever wall type abutments founded on concrete piling. An increase in elevation of U.S. 101 on the order of 2 feet will be necessary to allow for a minimum 15 feet vertical clear distance from the bottom of soffit to Kenmar Road. The undercrossing will be designed to accommodate a Type 742 concrete left barrier, a minimum 10-foot left shoulder, two 12-foot lanes of southbound traffic, 5-foot southbound median shoulder, a Type 60 median barrier, a 5-foot northbound median shoulder, two 12-foot lanes northbound traffic, a 10-foot right shoulder, and a Type 742 concrete right barrier. Falsework is not necessary to erect this type of girder structure.

The new undercrossing can be constructed in two phases. The initial phase would likely be to remove and construct approximately the west half of the new bridge, while U.S. 101 traffic utilizes the east half of the existing bridge. The final phase would be to reroute U.S. 101 traffic to the new west half and remove and construct the east half of the new structure and a 3-foot wide deck closure pour. The anticipated structure cost is \$4,700,000 not including costs for mobilization or contingencies. Bridge removal costs represent \$180,000 of this figure.

### Alternative 2 – Kenmar Road Interchange Roundabout Concepts Option 2 (Add Retaining Wall at US 101/Kenmar Road Interchange for new Multi-use Path)

The proposed Alternative 2 intends to construct a permanent retaining wall parallel to and in front of the north abutment of the existing Kenmar Road Undercrossing (Abutment 4) and to add traffic roundabouts each side of the interchange on Kenmar Road. The retaining wall in front of the abutment is to accommodate a 10-foot-wide pedestrian/bicycle facility under the structure. The total length of proposed wall will be approximately 180 feet.

The proposed wall layout line is 15 feet from the face of the existing columns; however, the layout line could be located as close as 10 feet from the face of existing columns. We considered using a Caltrans Type 7 retaining wall for the proposed structure for the layout line 10 feet from the existing columns and the excavation for a Type 7 wall would likely be outside the influence zone of the Abutment 1 diaphragm. If the wall layout line is located more than 10 feet from the existing column face, then the new wall will need to be a permanent tie-back (ground anchor) diaphragm wall constructed from top down in a minimum of three lifts. The maximum wall height above the pedestrian surface will be approximately 12 feet depending on layout. The wall foundations will extend 2 to 3 feet below finish grade. Cable railing will be mounted on top of the wall. Permanent tie-backs will require a permanent construction easement. The anticipated structure cost is \$635,000 not including costs for mobilization or contingencies. The estimate does not include cost for traffic control or temporary K-railing that will be needed during the work.

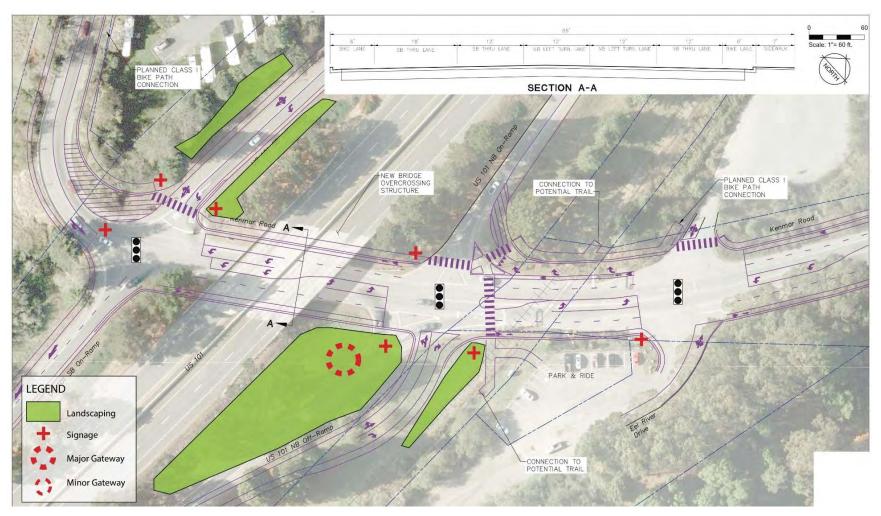
MORRISON STRUCTURES, INC.									
	Marina	Dr., Ste 104							
Redding, CA 96001									
BRIDGE GENERAL PLAN ESTIMATE	0.		NNING F	STIMATE					
STRUCTURE Kenmar Rd Ret Wall RW@US101	•	COUNTY HUM		RCVD. BY					
TYPE TYPE 1 & GRND ANCHOR DIST.	1	ROUTE US101		P.M.					
LENGTH 180 X WIDTH 14.5	=	2610 SF		EST. NO.		1			
PROJECT INCLUDES 1 ret wall STRUCTURES		QUANTITIES BY	RLM	DATE		9/2/2016			
AND \$ ROADWORK		CHECKED BY		DATE					
CONTRACT ITEMS	UNIT	QUANTITY		RICE		AMOUNT			
1 STRUCTURE EXCAVATION (RETAINING WALL)	CY	260		125.00	\$	32,500.00			
2 STRUCTURE EXCAVATION (GROUND ANCHOR WALL)	CY	106		125.00	\$	13,250.00			
3 STRUCTURE BACKFILL (RETAINING WALL)	CY	200		125.00	\$	25,000.00			
4 STRUCTURE BACKFILL (GROUND ANCHOR WALL)	CY	10	\$	275.00	\$	2,750.00			
5 PERVIOUS BACKFILL MATERIAL (RETAINING WALL)	CY	14	\$	205.00	\$	2,870.00			
6 GROUND ANCHOR 7 STRUCTURAL CONCRETE, RETAINING WALL	EA CY	60 125	\$ \$	5,450.00 825.00	\$ \$	327,000.00			
	LB	27150		825.00 1.35	ֆ \$	103,125.00 36,652.50			
8 BAR REINFORCING STEEL (RETAINING WALL) 9 STRUCTURAL SHOTCRETE	CY	27150	э \$	1,125.00	ֆ \$	36,652.50 63,000.00			
10 3" PLASTIC PIPE (RETAINING WALL)	LF	96		32.00	э \$	3,072.00			
11 GEOCOMPOSITE DRAIN	LS	1	\$	6,000.00	\$	6,000.00			
12 MINOR CONCRETE (GUTTER)	CY	6	\$	1,350.00	\$	8,100.00			
13 CABLE RAILING	LF	180	\$	63.00	\$	11,340.00			
14			\$	-	\$	-			
15			\$	-	\$	-			
16			\$	-	\$	-			
17			\$	-	\$	-			
18			\$	-	\$	-			
19			\$	-	\$	-			
20			\$	-	\$	-			
21			\$	-	\$	-			
22			\$	-	\$	-			
23			\$	-	\$	-			
26			\$	-	\$	-			
27			\$	-	\$	-			
28			\$	-	\$	-			
29			\$	-	\$	-			
30			\$	-	\$	-			
31			\$	-	\$	-			
COMMENTS	SUBTO		40	0/)	\$ ¢	634,659.50			
COMMENTS:	MOBILIZ	,	10	%)	\$ \$	70,517.72 705,177.22			
		AL STRUCTURE ITEMS GENCIES (	25	%)	ֆ \$	176,294.31			
COST ESTIM FOR 2016 CONSTRUCTION	TOTAL			%) / SF	ֆ \$	881,471.53			
		REMOVAL (CONTINGE	· ·		Ψ \$	-			
		Y RAILROAD OR UTILI		,	\$	_			
	GRAND				\$	881,471.53			
		DGET PURPOSES -	USE		\$	881,000.00			
	CON	MMENTS:							

### **MORRISON STRUCTURES, INC.**

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	M					C.			
		1890 Park M		,	104				
В	RIDGE GENERAL PL		ling, CA	96001			ESTIMATE		
	KENMAR RD UC/HWY			COUNTY			RCVD. BY		
TYPE		DIST.	1	ROUTE			P.M.		
LENGTH	114 X WIDTH	86.33	=	9842	SF		EST. NO.		1
PROJECT	INCLUDES 1	STRUCTURES	1	QUAN	TITIES BY	RLM	DATE		9/2/2016
	AND \$	ROADWORK		CHE	CKED BY		DATE		
			· · · · · · · · · · · · · · · · · · ·					1	
	CONTRACT ITEMS		UNIT	QUAN			RICE		AMOUNT
	' RAILING (TYPE K)		LF		80	\$	45.00	\$	66,600.0
	EXCAVATION (BRIDGE)		CY		35	\$	80.00	\$	258,800.0
	EXCAVATION (RET WALL)		CY CY		20 23	\$ \$	80.00 80.00	\$ \$	105,600.0
	BACKFILL (BRIDGE) BACKFILL (RET WALL)		CY	70		э \$	70.00	э \$	49,000.0
	ING(CLASS 90)(ALT "V")		LF	123		э \$	40.00	φ \$	493,680.0
	G (CLASS 90)(ALT "V")		EA	30		φ \$	1,275.00	φ \$	392,700.0
	L CONCRETE APPROACH	SLAR	CY	2		φ \$	1,150.00	\$	242,650.0
	L CONCRETE, BRIDGE FO		CY	36		\$	450.00	\$	163,350.0
	L CONCRETE, BRIDGE		CY	87		\$	1,200.00	\$	1,050,000.00
	L CONCRETE, RETAINING	WALI	CY	60		\$	1,000.00	\$	600,000.0
	PS CONC GIRDER (110'-12		EA		3		30.420.00	φ \$	395,460.0
		20)	EA		3 3	э \$	,	ֆ \$	
	S CONC GIRDER					•	6,425.00	<u> </u>	83,525.0
14 JOINT SEAL			LF		30	\$	75.00	\$	13,500.0
	RCING STEEL (BRIDGE)		LB		000	\$	1.35	\$	278,100.0
	RCING STEEL (RETAINING BARRIER (TYPE 742)	WALL)	LB LF	710 34		\$ \$	1.35 230.00	\$ \$	95,850.0 80,040.0
TT CONCRETE	BARRIER (11PE 742)		LF	34	+0	э \$	230.00	ф \$	80,040.0
						ф \$		\$	
						\$	-	\$	-
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1			SUBTO	TAL		. ·		÷	4,498,695.0
COMMENTS:	2-STAGE REMOVE	_	MOBILIZA	ATION	(	10	%)	\$	499,855.0
ND REPLACE		_	SUBTOTA	AL STRUCTU	IRE ITEMS			\$	4,998,550.0
et Wall type Wing	Walls on Piles	_	CONTING	SENCIES	(	25	%)	\$	1,249,637.5
DSTS ESTIM FOR	R 2016 CONSTRUCTION	_	BRIDGE		,	\$ 635		\$	6,248,187.5
		_	BRIDGE	REMOVAL (0	CONTINGEN	ICIES INC	L)	\$	180,000.0
		_			OR UTILIT	Y FORCE	S	\$	-
		-	GRAND						6,428,187.5
		-	FOR BU	DGET PUF	POSES -	USE		\$	6,429,000.0
		-	CON	MMENTS:					
		_							
		-							
		-							

Attachment H - Landscaping/Gateway Concepts



Kenmare Road Interchange Traffic Signal Concept - Landscaping Options



Kenmare Road Interchange Roundabout Concept - Landscaping Options

Attachment I - Cost Estimates

### Preliminary Opinion of Costs (Capital & Support) Kenmar Road Interchange Signal Concept

City of Fortuna

12/8/2017 25-3247-03/2132

Con	struction Costs					25-3247-03/2132
No.	Item Description	Units	Quantity	Unit Cost		Total
1	Traffic Control	LS	1	\$497,000.00		\$497,000.00
2	Remove Metal Beam Guard Railing	LF	600	\$16.00		\$9,600.00
3	Remove Roadside Sign	EA	29	\$102.00		\$2,958.00
4	Remove Asphalt Concrete Dike	LF	4140	\$4.00		\$16,560.00
5	Remove Concrete (Curb & Gutter)	LF	2400	\$10.00		\$24,000.00
6	Remove Tree	EA	7	\$1,400.00		\$9,800.00
7	Roadway Excavation	CY	7370	\$67.00		\$493,790.00
8	Class 2 Aggregate Base	CY	11350	\$70.00		\$794,500.00
	Hot Mix Asphalt (Type A)	TON	6790	\$140.00		\$950,600.00
	Bridge (US 101 Over Kenmar Road)	LS	1	\$4,700,000.00		\$4,700,000.00
11	Detectable Warning Surface	SQFT	300	\$35.00		\$10,500.00
12	Minor Concrete (Curb)	CY	4	\$1,320.00		\$5,280.00
	Minor Concrete (Curb and Gutter)	CY	149	\$806.00		\$120,427.99
	Minor Concrete (Sidewalk)	CY	130	\$680.00		\$88,400.00
	Storm Drain System	LS	1	\$100,000.00		\$100,000.00
	Midwest Guard Rail System (Wood Post)	LF	440	\$80.00		\$35,200.00
17	Thermoplastic Traffic Stripe	LF	10520	\$1.25		\$13,150.00
18	Thermoplastic Pavement Marking	SQFT	1684	\$6.00		\$10,101.00
	Signs	EA	45	\$350.00		\$15,750.00
20	Signal and Lighting	LS	3	\$225,000.00		\$675,000.00
21	Lighting & Electrical	LS	1	\$100,000.00		\$100,000.00
	Planting and Irrigation	SQFT	39500	\$5.00		\$197,500.00
	Mobilization	LS	1	\$837,400.00		\$837,400.00
24	Minor/ Supplemental Items	%	25%	\$8,373,116.99		\$2,093,279.25
	Subtotal (Construction Costs)				\$	11,800,796.23
	Construction Contingency			25%	\$	2,950,199.06
	Total Construction Costs				\$	14,750,995.29
	Total Construction Budget (Rounded)				\$	14,751,000.00
Diah	t of Way (Capital) and Utility Relocation Costs:					
1	Right Of Way	SQFT	0	\$20.00		\$0.00
2	Utility Relocation (TBD)	LS	1	\$200,000.00		\$200,000.00
2			-	\$200,000.00	*	
	Total Right of Way (Capital) and Utility Relocation	on Cost	s 		\$	200,000.00
	Total Project Capital Cost				\$	14,951,000.00
Drei	ect Support Costs					
			Canital Cast	400/	¢	4 405 400 00
1	Environmental Clearance (CEQA/NEPA)		Capital Costs	10%	\$	1,495,100.00
2	PS&E		Con. Costs	20%	\$	2,950,200.00
3	Right of Way Engineering & Acquisition Construction Support and Management		0-Parcels	\$25k/EA	\$	-
4			Con. Costs	15%	\$	2,212,700.00
	Total Project Support Costs				\$	6,658,000.00
	Total Estimated Project Costs				\$	21,609,000.00
	Rounded					21,610,000.00
	Noullaeu				φ	∠1,010,000.00

Assuptions

1. All new paving.

2. Only R/W costs are for private properties (not County, City, or State).

3. Bridge removal included in the cost for each bridge.

### Preliminary Opinion of Costs (Capital & Support)

Kenmar Road Interchange Roundabout Concept - Option 1a

12/8/2017 25-3247-03/2132

	struction Costs	11	0		1	<b>T</b> = (=1
No.	Item Description	Units	Quantity	Unit Cost		Total
	Traffic Control	LS LF	1 850	\$140,000.00		\$140,000.00
2	Remove Metal Beam Guard Railing Remove Roadside Sign	EA	29	\$16.00		\$13,600.00
<u>3</u> 4	Remove Asphalt Concrete Dike	LF	29 2460	\$102.00		\$2,958.00
4	Remove Concrete (Curb & Gutter)		1210	\$4.00 \$10.00		\$9,840.00 \$12,100.00
6	· · · · · · · · · · · · · · · · · · ·		7			
	Remove Tree Roadway Excavation	EA CY	5060	\$1,400.00 \$67.00		\$9,800.00 \$339,020.00
8	Class 2 Aggregate Base	CY	5510	\$70.00		\$385,700.00
	Hot Mix Asphalt (Type A)	TON	3060	\$140.00		\$428,400.00
	Detectable Warning Surface	SQFT	300	\$35.00		\$10,500.00
11	Minor Concrete (Curb)	CY	36	\$1,320.00		\$47,520.00
12	Minor Concrete (Curb - Truck Apron)	CY	47	\$1,160.00		\$54,520.00
	Minor Concrete (Curb and Gutter)	CY	187	\$806.00		\$150,722.00
	Minor Concrete (Stamped Concrete - Truck Apron)	CY	230	\$615.00		\$141,450.00
	Minor Concrete (Sidewalk)	CY	125	\$680.00		\$85,000.00
	Storm Drain System	LS	125	\$180,000.00		\$180,000.00
17	Midwest Guard Rail System (Wood Post)	LS	820	\$180,000.00		\$65,600.00
	Thermoplastic Traffic Stripe		4650	\$1.25		\$5,812.50
	Thermoplastic Pavement Marking	SQFT	1276	\$6.00		\$7,656.00
	Signs	EA	55	\$350.00		\$19,250.00
20	Lighting & Electrical	LA	1	\$260,000.00		\$260,000.00
	Planting and Irrigation	SQFT	13900	\$200,000.00		\$69,500.00
	Mobilization	LS	13300	\$229,900.00		\$229,900.00
	Minor/ Supplemental Items	%	25%	\$2,298,948.50		\$574,737.13
27		70	2070	ψ2,200,040.00	*	
	Subtotal (Construction Costs)			050/	\$	3,243,585.63
	Construction Contingency			25%	\$	810,896.41
	Total Construction Costs				\$	4,054,482.03
	Total Construction Budget (Rounded)				\$	4,054,500.00
Righ	l					
1	Right Of Way	SQFT	3800	\$20.00		\$76,000.00
2	Utility Relocation	ALLOW	1	\$200,000.00		\$200,000.00
	Total Right of Way (Capital) and Utility Relocati	on Costs	5		\$	276,000.00
	Total Project Capital Cost				\$	4,330,500.00
Proi	ect Support Costs					
	PA&ED		Capital Costa		\$	550,000.00
2	PS&E		Capital Costs Con. Costs	20%	Դ \$	810,900.00
2	Right of Way Engineering & Acquisition		1-Parcels	\$25k/EA	Դ \$	25,000.00
4	Construction Support and Management		Con. Costs	15%	ֆ \$	608,200.00
+			001. 00313	1J70		
	Total Project Support Costs				\$	1,994,100.00
	Total Estimated Project Costs				\$	6,324,600.00
						6,330,000.00

### Assuptions

1. All new paving.

2. Only R/W costs are for private properties (not County, City, or State).

3. Removing railroad tracks and equipment not included.

### Preliminary Opinion of Costs (Capital & Support)

Kenmar Road Interchange Roundabout Concept - Option 1b

12/8/2017 25-3247-03/2132

No.	struction Costs Item Description	Units	Quantity	Unit Cost		Total
-	Traffic Control	LS	<u>quantity</u> 1	\$140,000.00		\$140,000.00
	Remove Metal Beam Guard Railing	LF	850	\$16.00		\$13,600.00
3	Remove Roadside Sign	EA	29	\$102.00		\$2,958.00
4	Remove Asphalt Concrete Dike	LF	2460	\$4.00		\$9,840.00
5	Remove Concrete (Curb & Gutter)	LF	1210	\$10.00		\$12,100.00
6	Remove Tree	EA	6	\$1,400.00		\$8,400.00
	Roadway Excavation	CY	5160	\$67.00		\$345,720.00
8	Class 2 Aggregate Base	CY	5860	\$70.00		\$410,200.00
9	Hot Mix Asphalt (Type A)	TON	3270	\$140.00		\$457,800.00
	Detectable Warning Surface	SQFT	360	\$35.00		\$12,600.00
11	Minor Concrete (Curb)	CY	26	\$1,320.00		\$34,320.00
12	Minor Concrete (Curb - Truck Apron)	CY	32	\$1,160.00		\$37,120.00
		CY	192	\$806.00		\$154,752.00
	Minor Concrete (Stamped Concrete - Truck Apron)	CY	170	\$615.00		\$104,550.00
	Minor Concrete (Sidewalk)	CY	153	\$680.00		\$104,040.00
	Storm Drain System	LS	1	\$180,000.00		\$180,000.00
17	Midwest Guard Rail System (Wood Post)	LF	820	\$80.00		\$65,600.00
	Thermoplastic Traffic Stripe	LF	5000	\$1.25		\$6,250.00
	Thermoplastic Pavement Marking	SQFT	1578	\$6.00		\$9,468.00
20	Signs	EA	40	\$350.00		\$14,000.00
21	Lighting & Electrical	LS	1	\$220,000.00		\$220,000.00
22	Planting and Irrigation	SQFT	10300	\$5.00		\$51,500.00
	Mobilization	LS	1	\$225,500.00		\$225,500.00
24	Minor/ Supplemental Items	%	25%	\$2,254,818.00		\$563,704.50
	Subtotal (Construction Costs)				\$	3,184,022.50
	Construction Contingency			25%	\$	796,005.63
				2070		
	Total Construction Costs				\$	3,980,028.13
	Total Construction Budget (Rounded)				\$	3,980,100.00
	Int of Way (Capital) and Utility Relocation Costs:					
1	Right Of Way	SQFT	0	\$20.00		\$0.00
2	Utility Relocation	ALLOW	1	\$200,000.00		\$200,000.00
	Total Right of Way (Capital) and Utility Relocation	on Costs	5		\$	200,000.00
	Total Project Capital Cost				\$	4,180,100.00
Droi	ect Support Costs					
	PA&ED		Canital Casta		¢	FF0 000 00
1 2	PA&ED PS&E		Capital Costs Con. Costs	200/	\$	550,000.00
				20%	\$6	796,100.00
3	Right of Way Engineering & Acquisition		0-Parcels Con. Costs	\$25k/EA	\$ \$	- 507 400 00
4	Construction Support and Management		CON. COSIS	15%		597,100.00
	Total Project Support Costs				\$	1,943,200.00
	Total Estimated Project Costs				\$	6,123,300.00
	Total Estimated Project Costs				¥	•,•=•,••••••

### Assuptions

1. All new paving.

2. Only R/W costs are for private properties (not County, City, or State).

3. Removing railroad tracks and equipment not included.

### Preliminary Opinion of Costs (Capital & Support) Kenmar Road Interchange Roundabout Concept - Option 2

City of Fortuna

12/8/2017 25-3247-03/2132

	struction Costs					
No.	Item Description	Units	Quantity	Unit Cost		Total
1	Traffic Control	LS	1	\$182,000.00		\$182,000.00
2	Remove Metal Beam Guard Railing	LF	850	\$16.00		\$13,600.00
3	Remove Roadside Sign	EA	29	\$102.00		\$2,958.00
4	Remove Asphalt Concrete Dike	LF	2450	\$4.00		\$9,800.00
5	Remove Concrete (Curb & Gutter)	LF	1200	\$10.00		\$12,000.00
6	Remove Tree	EA	7	\$1,400.00		\$9,800.00
7	Roadway Excavation	CY	5510	\$67.00		\$369,170.00
8	Class 2 Aggregate Base	CY	6590	\$70.00		\$461,300.00
9	Hot Mix Asphalt (Type A)	TON	3770	\$140.00		\$527,800.00
10	Retaining Wall	LS	1	\$635,000.00		\$635,000.00
11	Detectable Warning Surface	SQFT	420	\$35.00		\$14,700.00
	Minor Concrete (Curb)	CY	16	\$1,320.00		\$21,120.00
	Minor Concrete (Curb - Truck Apron)	CY	32	\$1,160.00		\$37,120.00
14	Minor Concrete (Curb and Gutter)	CY	133	\$806.00		\$107,198.00
15	Minor Concrete (Stamped Concrete - Truck Apron)	CY	170	\$615.00		\$104,550.00
	Minor Concrete (Sidewalk)	CY	139	\$680.00		\$94,520.00
17	Storm Drain System	LS	1	\$180,000.00		\$180,000.00
18	Midwest Guard Rail System (Wood Post)	LF	990	\$80.00		\$79,200.00
19	Thermoplastic Traffic Stripe	LF	6180	\$1.25		\$7,725.00
20	Thermoplastic Pavement Marking	SQFT	1151	\$6.00		\$6,906.24
21	Signs	EA	50	\$350.00		\$17,500.00
22	Lighting & Electrical	LS	1	\$260,000.00		\$260,000.00
23	Planting and Irrigation	SQFT	14100	\$5.00		\$70,500.00
24	Mobilization	LS	1	\$304,300.00		\$304,300.00
25	Minor/ Supplemental Items	%	25%	\$3,042,467.24		\$760,616.81
	Subtotal (Construction Costs)				\$	4,289,384.05
	Construction Contingency			25%	\$	1,072,346.01
	Total Construction Costs				\$	5,361,730.06
	Total Construction Budget (Rounded)				\$	5,361,800.00
Riał	nt of Way (Capital) and Utility Relocation Costs:					
1	Right Of Way	SQFT	0	\$20.00		\$0.00
2	Utility Relocation	ALLOW	1	\$200,000.00		\$200,000.00
	Total Right of Way (Capital) and Utility Relocation	-		φ200,000.00	\$	200,000.00
	Total Project Capital Cost				\$	5,561,800.00
Proj	ect Support Costs					
1	PA&ED		Capital Costs		\$	550,000.00
2	PS&E		Con. Costs	20%	\$	1,072,400.00
3	Right of Way Engineering & Acquisition		0-Parcels	\$25k/EA	\$	-
4	Construction Support and Management		Con. Costs	15%	\$	804,300.00
	Total Project Support Costs				\$	2,426,700.00
					<b>^</b>	7 000 500 50
	Total Estimated Project Costs				\$	7,988,500.00
	Rounded				\$	7,990,000.00

Assuptions

1. All new paving.

Only R/W costs are for private properties (not County, City, or State).
 Removing railroad tracks and equipment not included.

Attachment J - Right-of-Way and Property Ownership



### **RIGHT OF WAY MEMORANDUM**

May 25, 2016

### **Base Mapping**

The base map consists of the Caltrans highway map 1 HUM-1-F coordinated on the California Coordinate System, Zone 1. This developed the centerline of Hwy 101 and the right of way lines through the project area. Parcels relinquished by Caltrans as part of the Hwy 101 Project are also shown on this map. The eastern Caltrans right of way line is the western line of the railroad right of way through most of this area.

### 12<sup>th</sup> Street Interchange

**Newburg Road** Book P of Deeds, Page 428 HCR describes the width of Newburg as 50 feet wide. Multiple tract maps were prepared on the North side of Newburg, however, no map references Book P of deeds or any other documentation for Newburg Road is listed on the maps. The Beacom subdivision map, recorded in Book 12 of Maps, page 138, lists the width for Newburg as 40 feet. The south side of Newburg in this area is all under one ownership by the Town of Scotia. Ground shots of existing improvements (back of walk to fence) indicate a width of 50 feet.

**12th Street south of railroad crossing and north of the overpass** the area south of the railroad right of way is owned by Caltrans, and has a width of 75 feet based on the Caltrans mapping. The Caltrans map shows the railroad crossing being relinquished to the County of Humboldt in 1978 per 1487 OR 184. The Caltrans map 1 HUM 1 F does not clearly delineate the transition from Caltrans to City of Fortuna ownership immediately south of the rail road crossing. This was at one time Sandy Prairie Road so the right of way was already existing before the Hwy 101 project. The County of Humboldt has a pavement maintenance agreement with Caltrans for the County roads carried over, under, or to the connecting freeway dated April 15<sup>th</sup> 1963. The exhibit attached to this agreement shows the County area of responsibility to be from a line approximately 50 feet south of the rail road tracks across the overpass to Dinsmore Drive on the south side of the overpass. Clendenen is the owner of the parcel to the west of 12<sup>th</sup> St., and Sequia Gas/ McWhorter owns the multiple parcels to the east of 12<sup>th</sup> St., including the abandoned Pond street.

**12th Street north** of the railroad right of way, Parcel Map 1828 Book 16, page 28 shows a half width of 12th street as 30 feet, and Parcel Map 2817 Book 25, page 103 shows a full width of 12th street as 60 feet.

**Dinsmore Drive/North end Riverwalk Drive** Caltrans relinquished this road to Humboldt County in 1963 per Book 760 Deeds, Page 517. The configuration shown is based on the Caltrans right of way map 1 HUM 1 F. In a letter dated January 10, 2005 between the City of Fortuna and the County of Humboldt, discusses the annexation & maintenance of Strongs Creek Road (Dinsmore Drive) by the City of Fortuna. The letter does not specifically describe the limits of maintenance. The bridge located on the north end of Riverwalk drive (over Strong's Creek) is shown to be in this Caltrans relinquishment area, however, the bridge itself does not appear to be listed in the County bridge maintenance logs. The specific location of the City/County change of ownership will need to be determined.

## Kenmar Road Interchange

**Kenmar Road east of the Freeway** - No documentation could be found for this portion of Kenmar Road. The south right of way line shown is based upon the survey for the park and ride (see key note 6), and the County's Eel River Drive overlay project #213500. The north line is based upon field ties to features and said County overlay project. The State Park and Ride location is based on a survey for Caltrans. However, the survey has no recorder stamp, and the book and page referenced do not refer to this survey at the recorder's office. This map was provided by Caltrans.

In general this area has a complex right of way situation due to the multiple highways and roads that were here at one time. Additional research and surveying will be needed to determine the right of way location. The County of Humboldt has a pavement maintenance agreement with Caltrans for the County roads carried over, under, or to the connecting freeway dated April 15<sup>th</sup> 1963. The exhibit attached to this agreement shows the County area of responsibility to be from the west line of the railroad tracks to west line of Hwy 101 at the intersection with Riverwalk Drive.

**Eel River Drive** - The east line of Eel River Drive was mapped to some extent by the County during the overlay survey, however, the County surveyor's office also discusses the complexity of the right of way in the area, and the need for more surveying to determine the true location.

**Riverwalk Drive** – Riverwalk Drive west of Hwy 101 was relinquished to the County in 1963 per 760 OR 517. The east side of the right of way has been delineated in a survey from 1992 recorded in Book 53 surveys, page 34. There have not been any surveys filed on the west side of Riverwalk drive at this location. Ground shots indicate a distance of 50' between back of walk on the east side and top of slope on the west side.



Attachments used per Area:

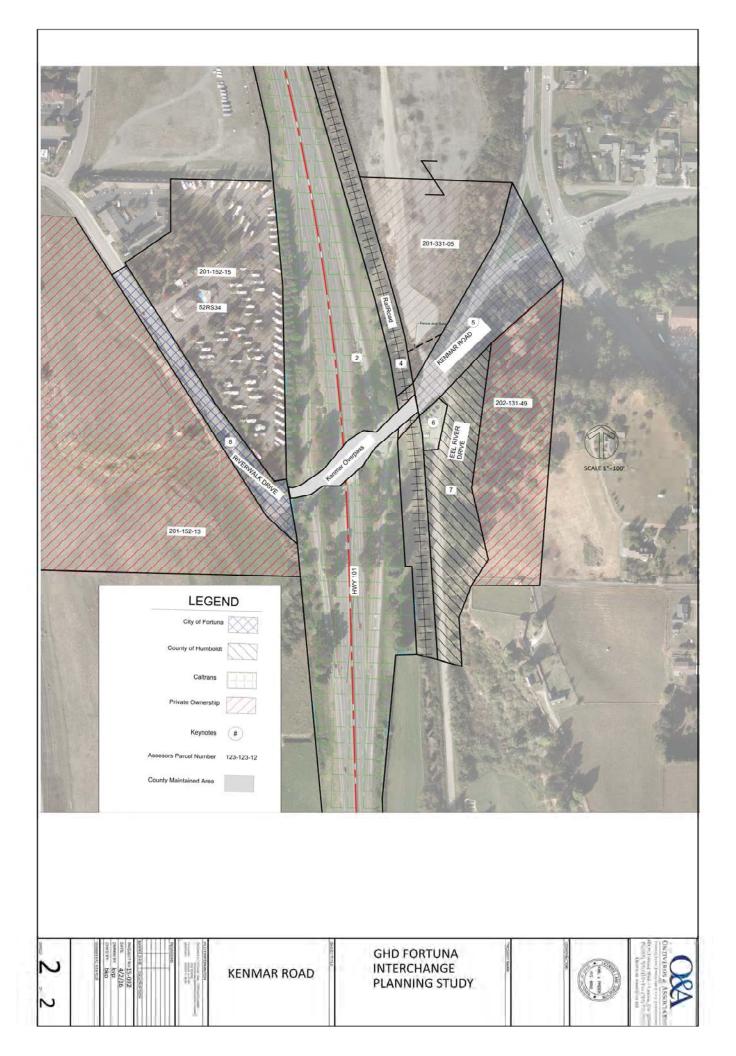
## 12<sup>th</sup> Street Interchange:

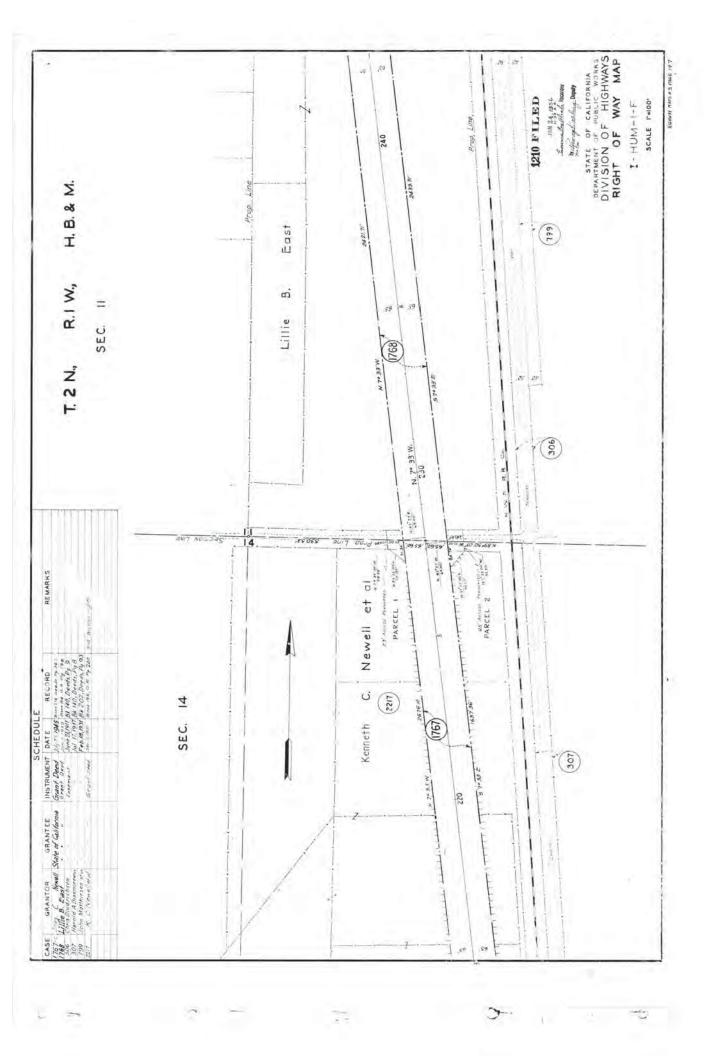
Book P of Deeds, Page 428 1 HUM 1 F Cal Trans Mapping 1705 Official Records 484 Parcel Map 2817 Book 25, page 103 Parcel Map 1828 Book 16, page 28 Book 12 of Maps, page 138 Book 29 Parcel Maps, Page 32 Book 13 Maps, Page 15 North Pacific Railroad Maps Book 13 Maps, Page 16 County Letter dated January 10, 2005 Book 13 Maps, page 35 Deed 1999-8138-4

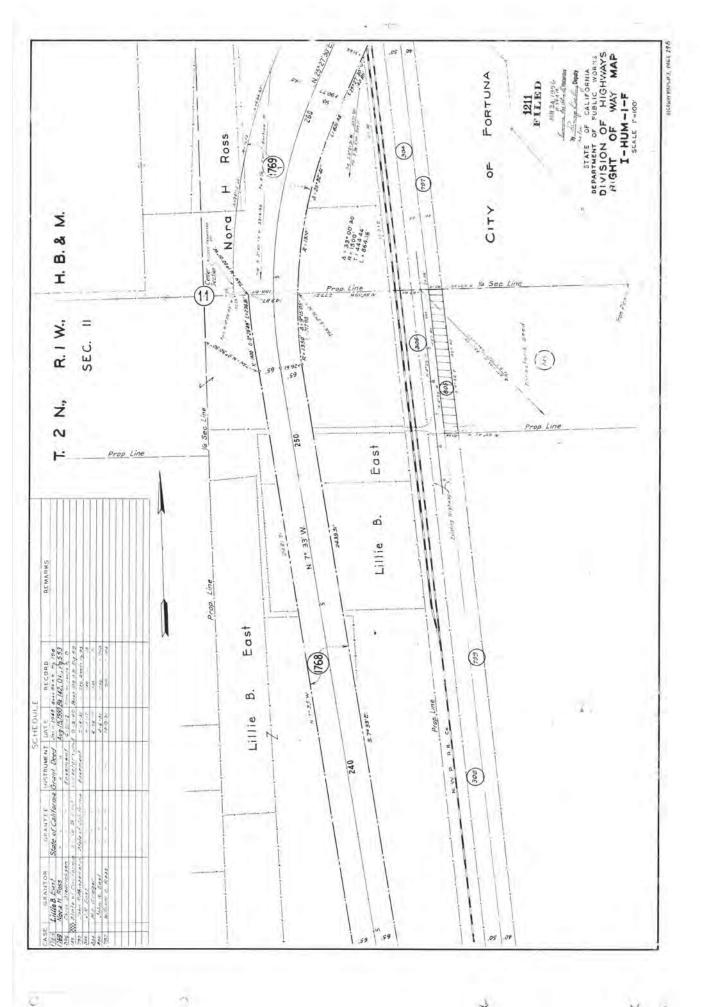
## Kenmar Road Interchange:

1 HUM-1-F Cal Trans Mapping North Pacific Railroad Maps Book 53 surveys, Page 34 Book 29 Surveys, Page 104 Book 38 Surveys, Page 59 Book 68 Surveys, Page 28 Book 67 Surveys, Page 56 County Letter dated January 10, 2005 County Project files Eel River Drive Overlay Old Rohnerville Map





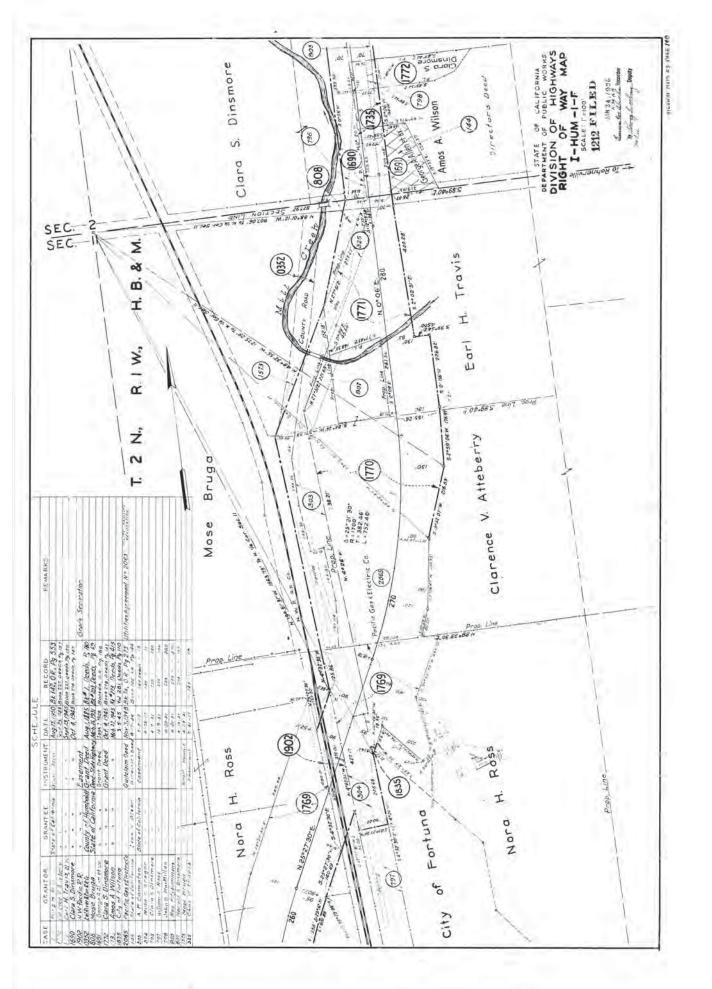




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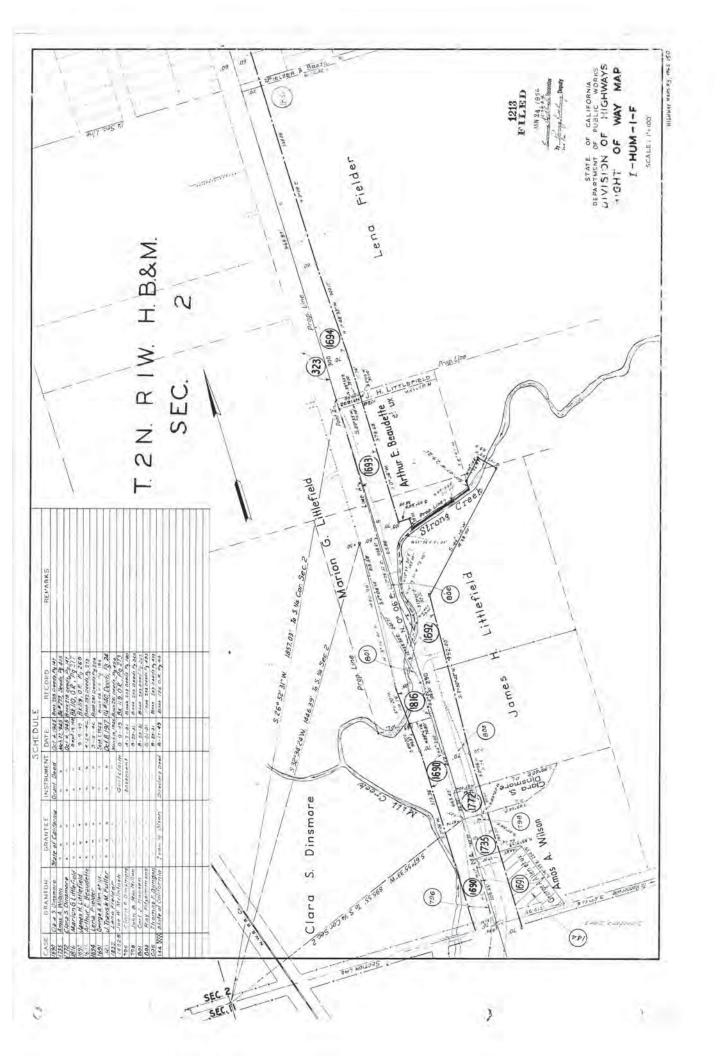
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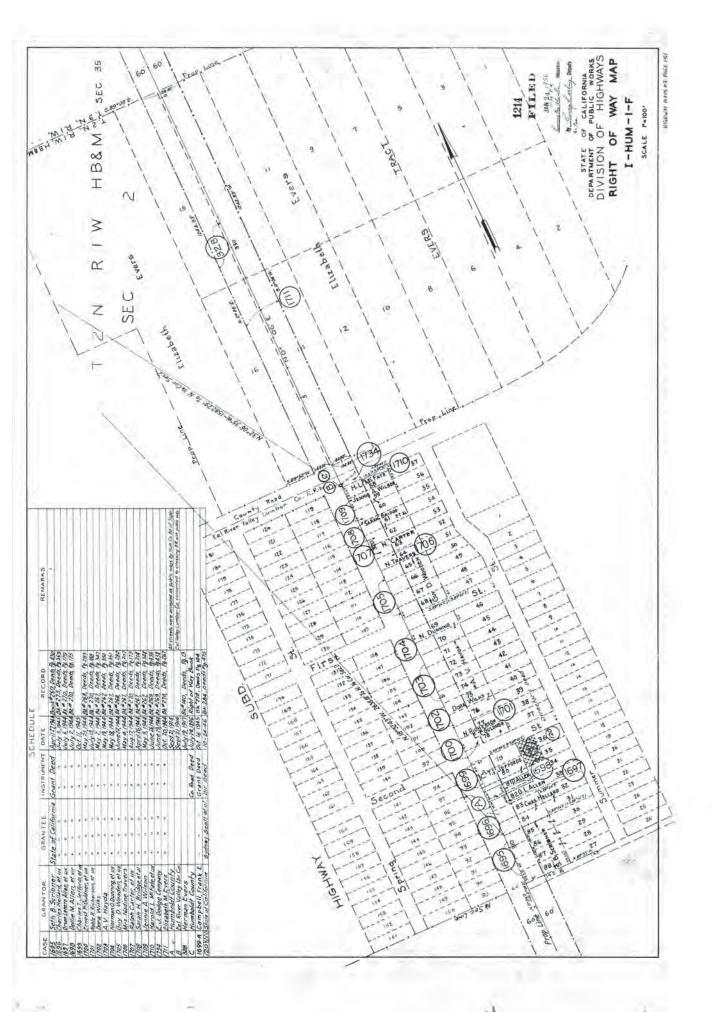
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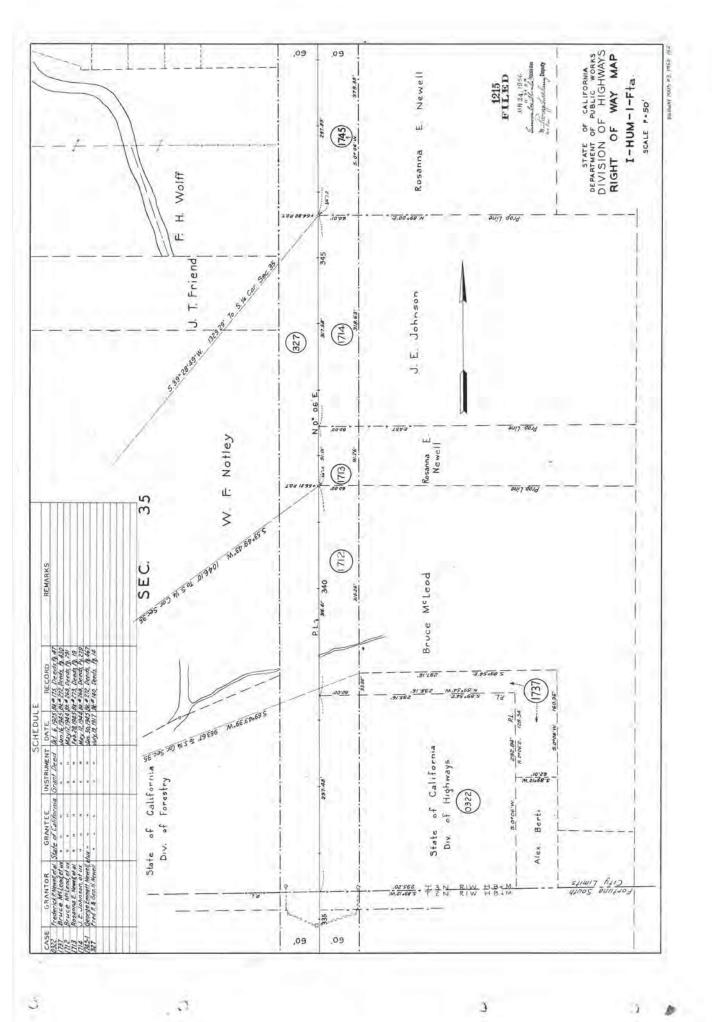
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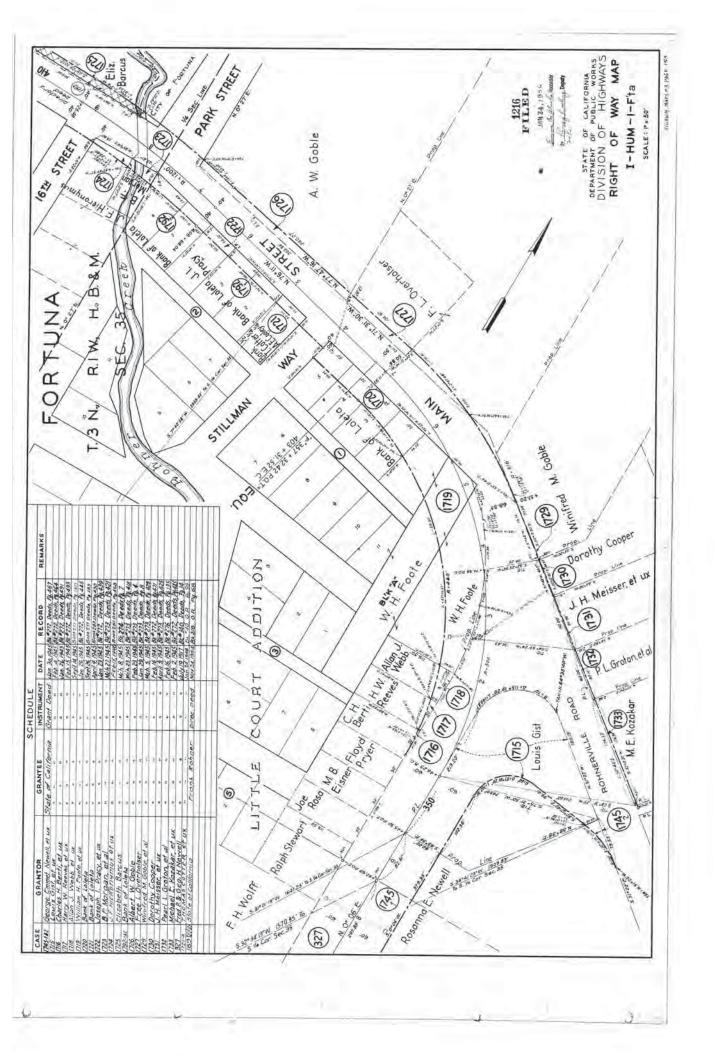


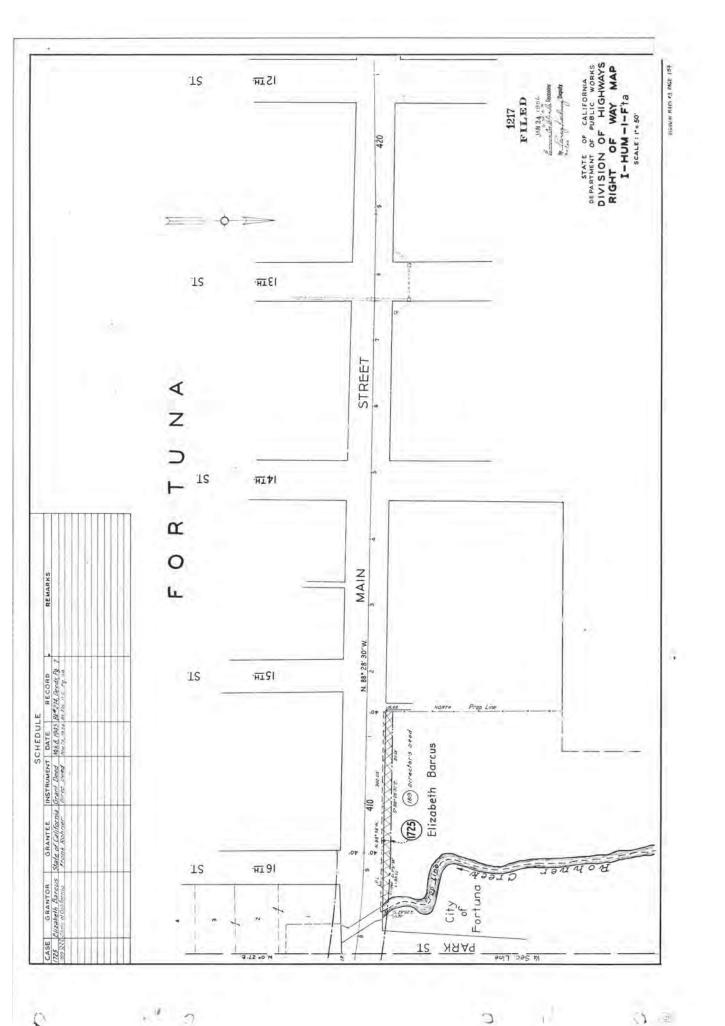
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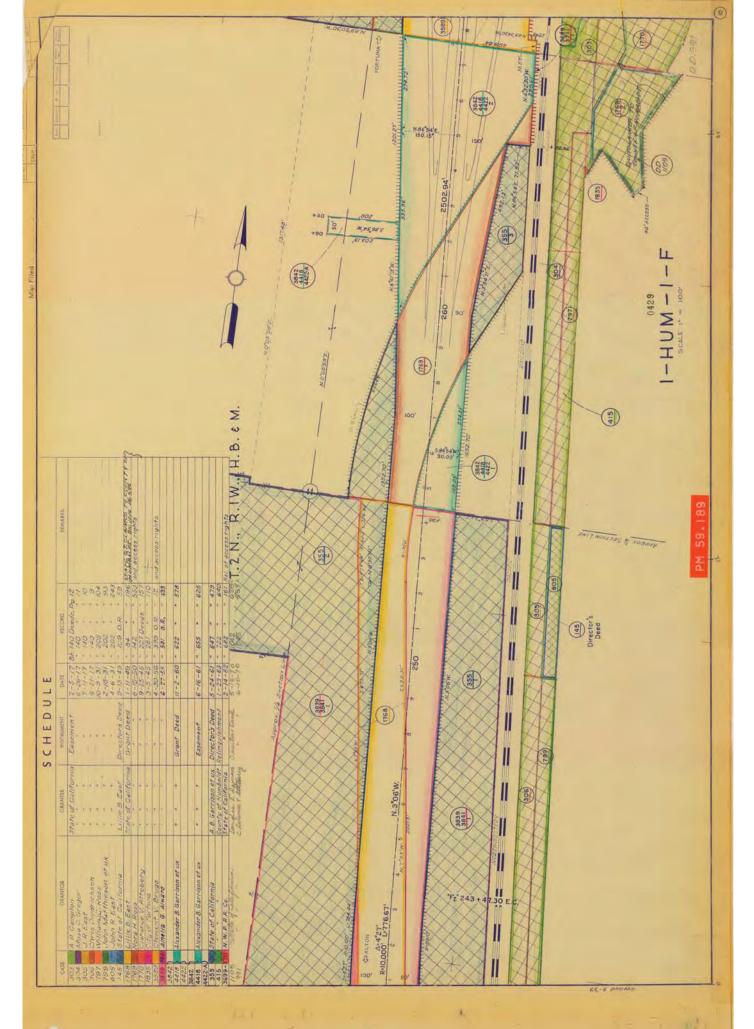


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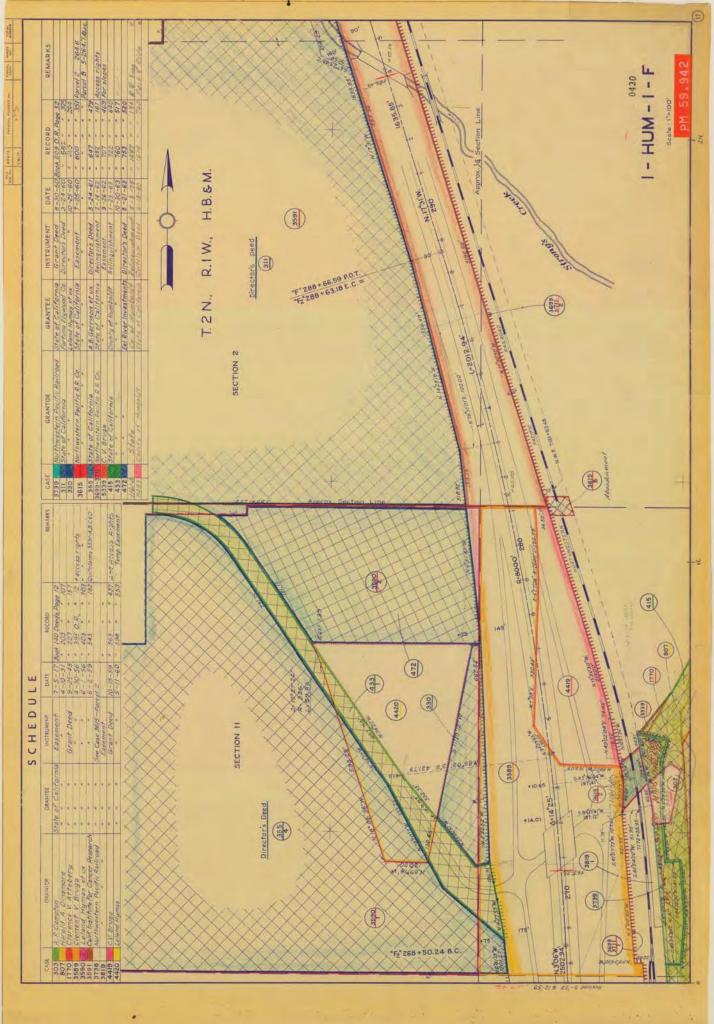
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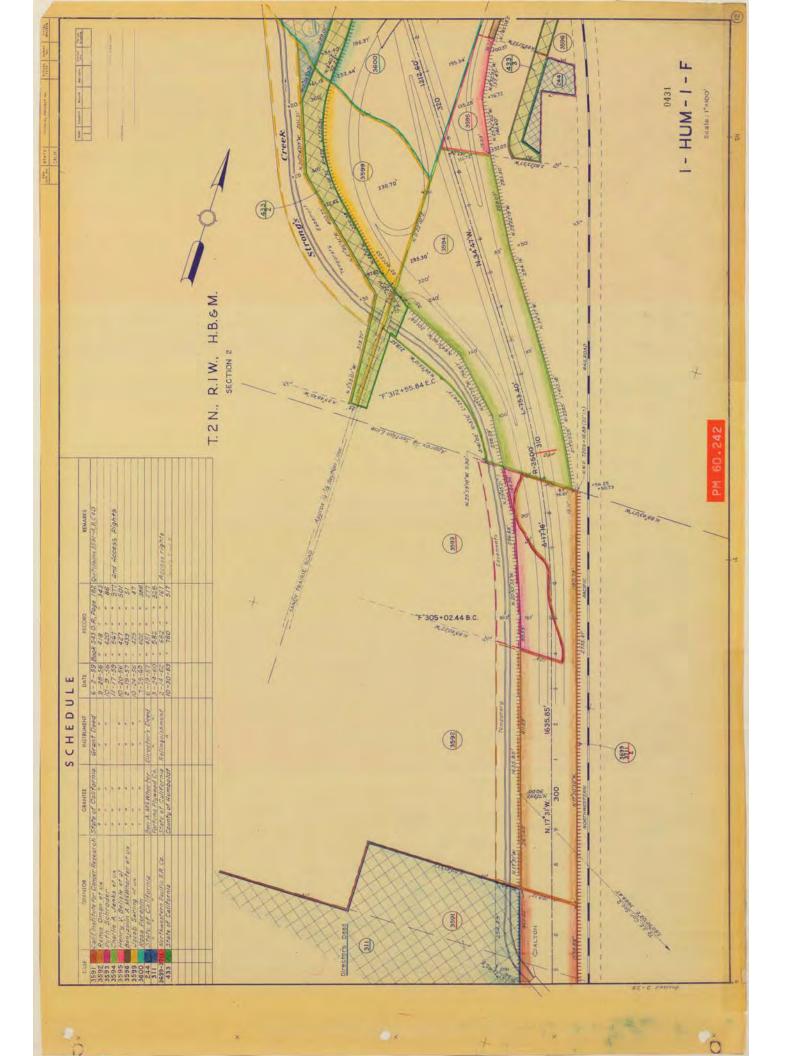
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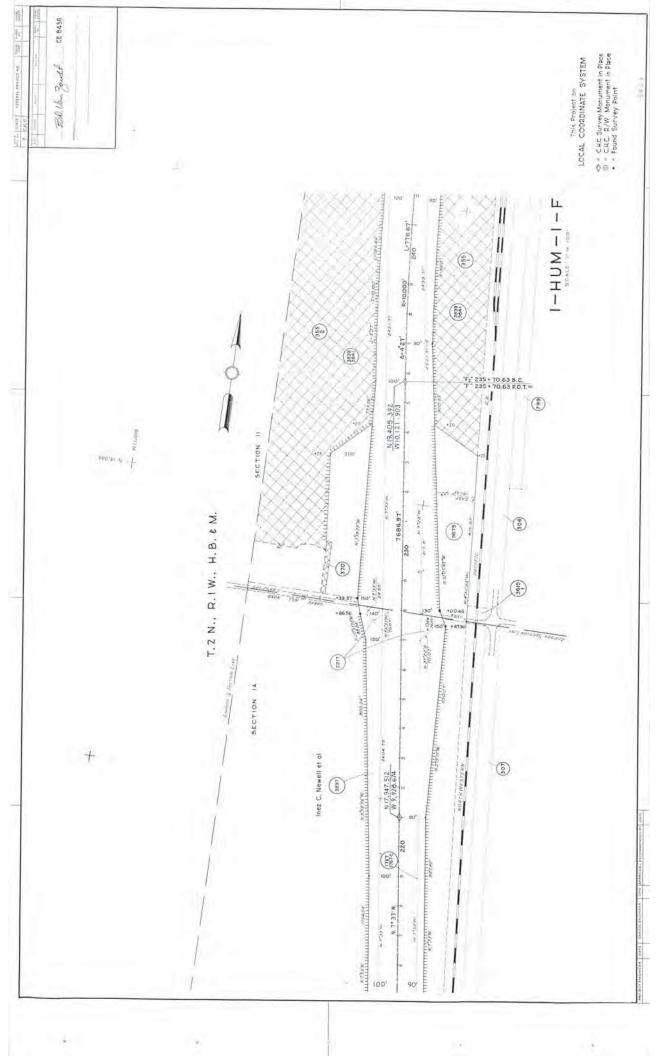
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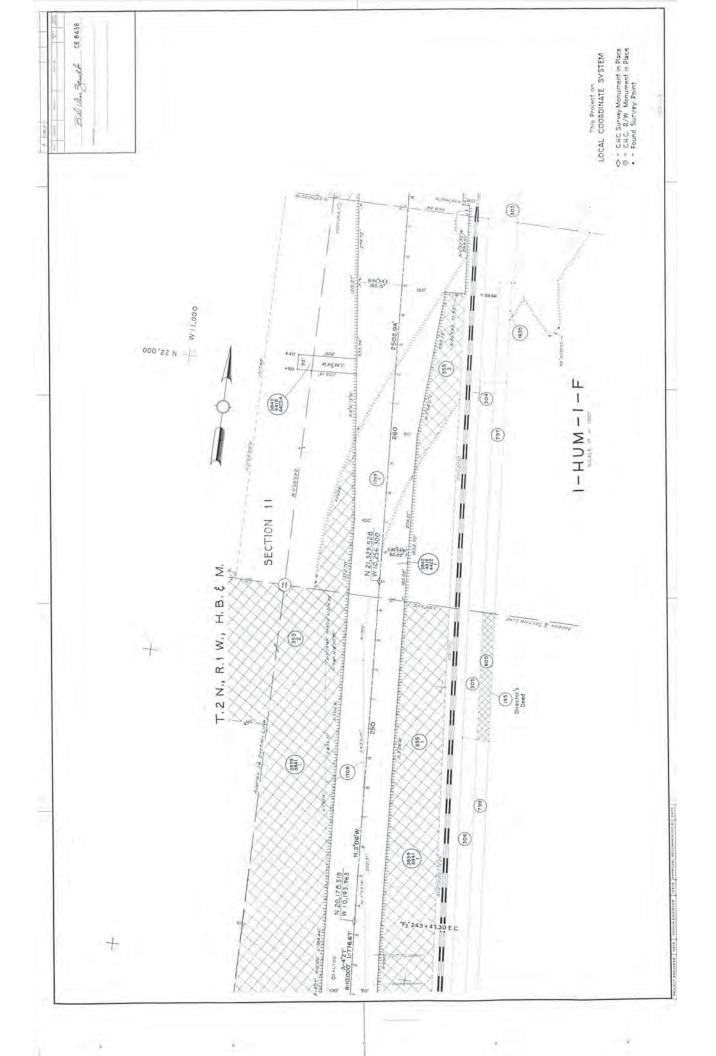


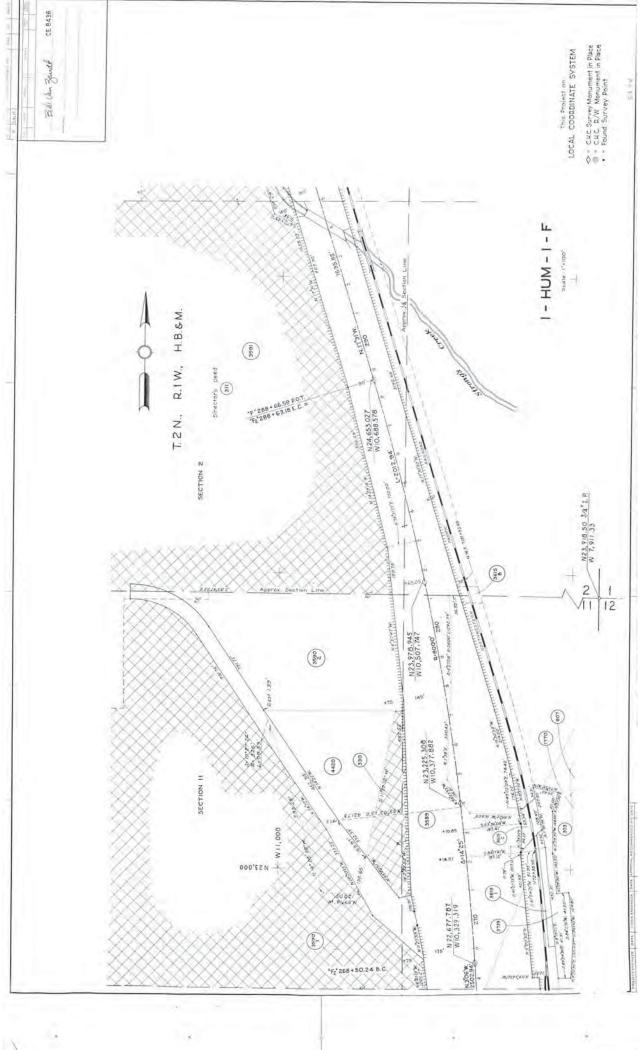
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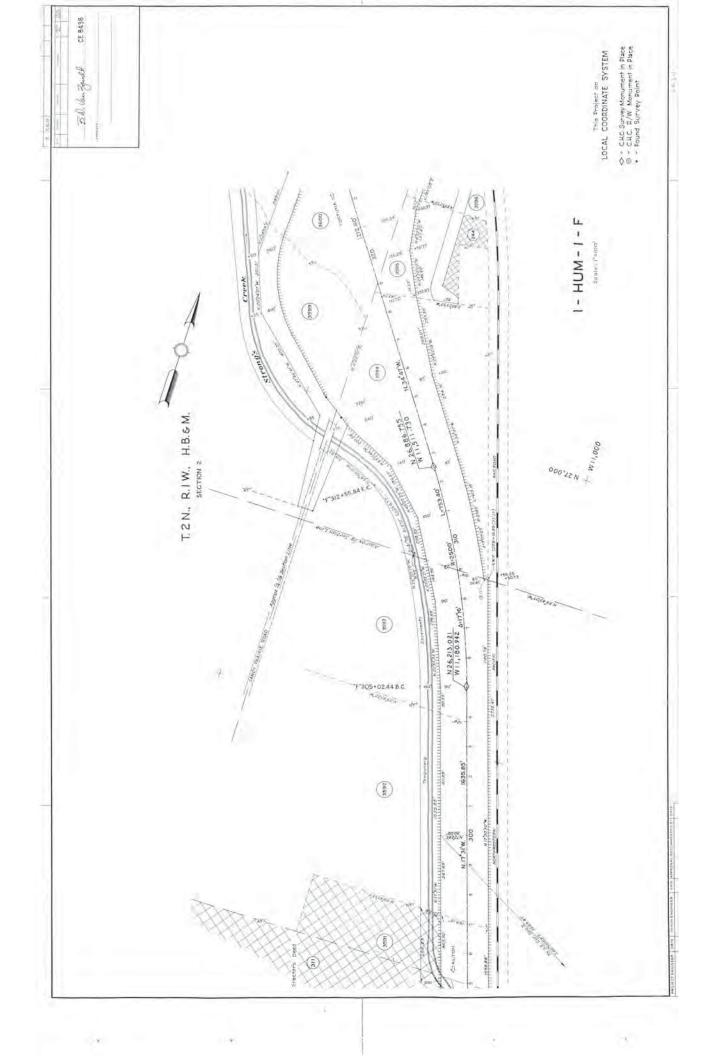
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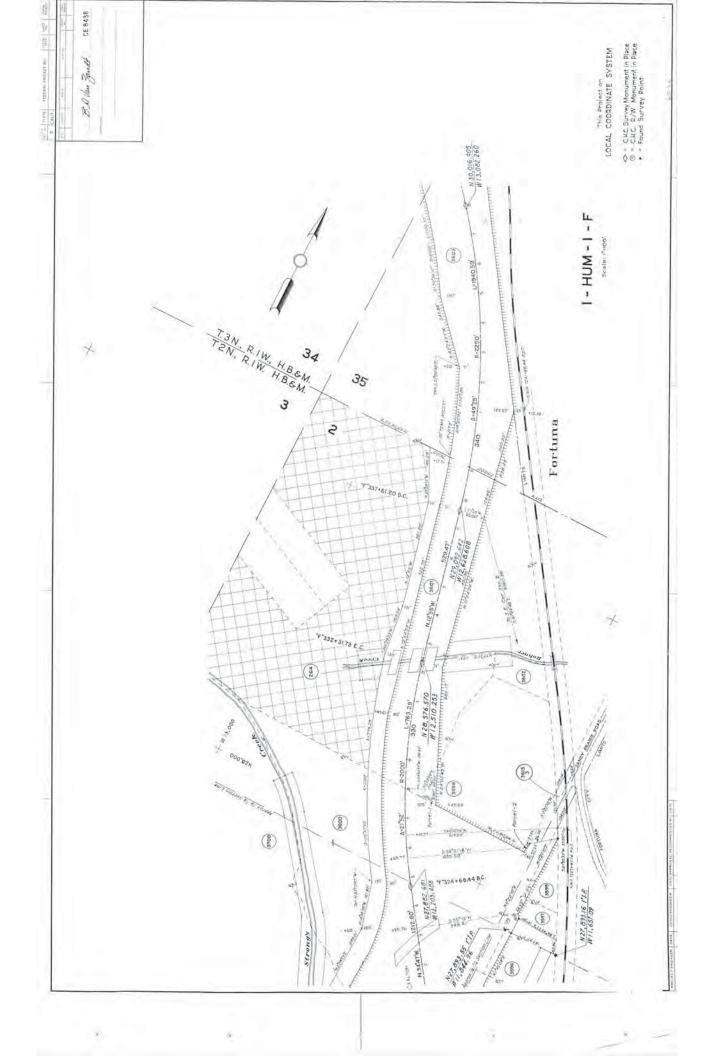


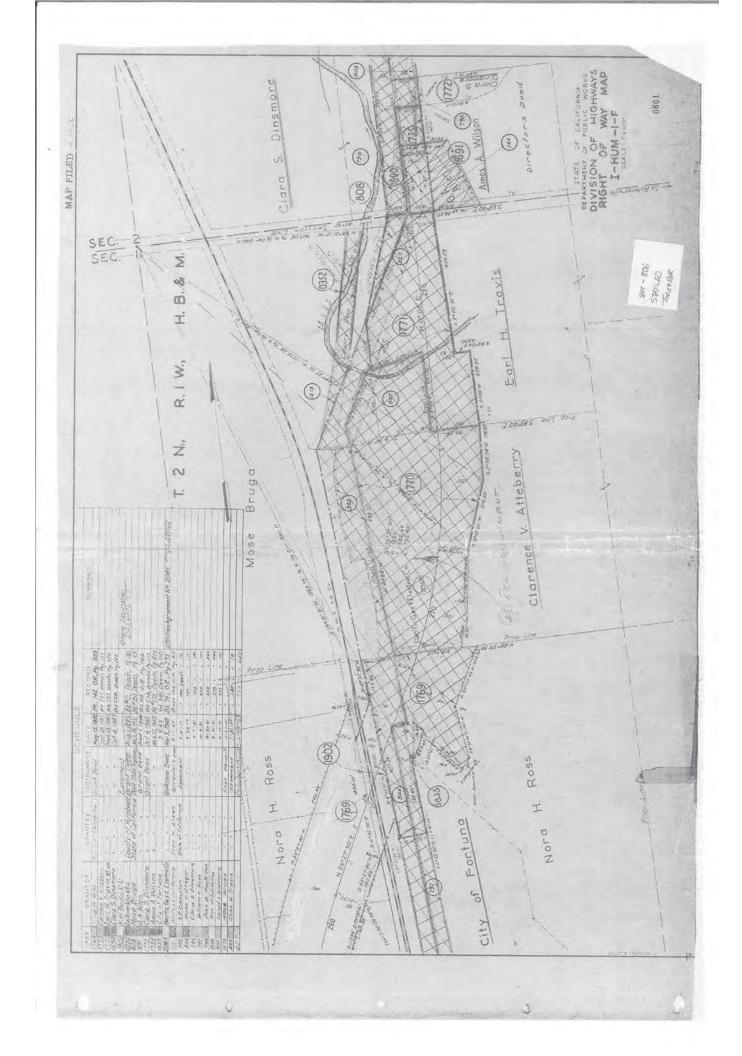


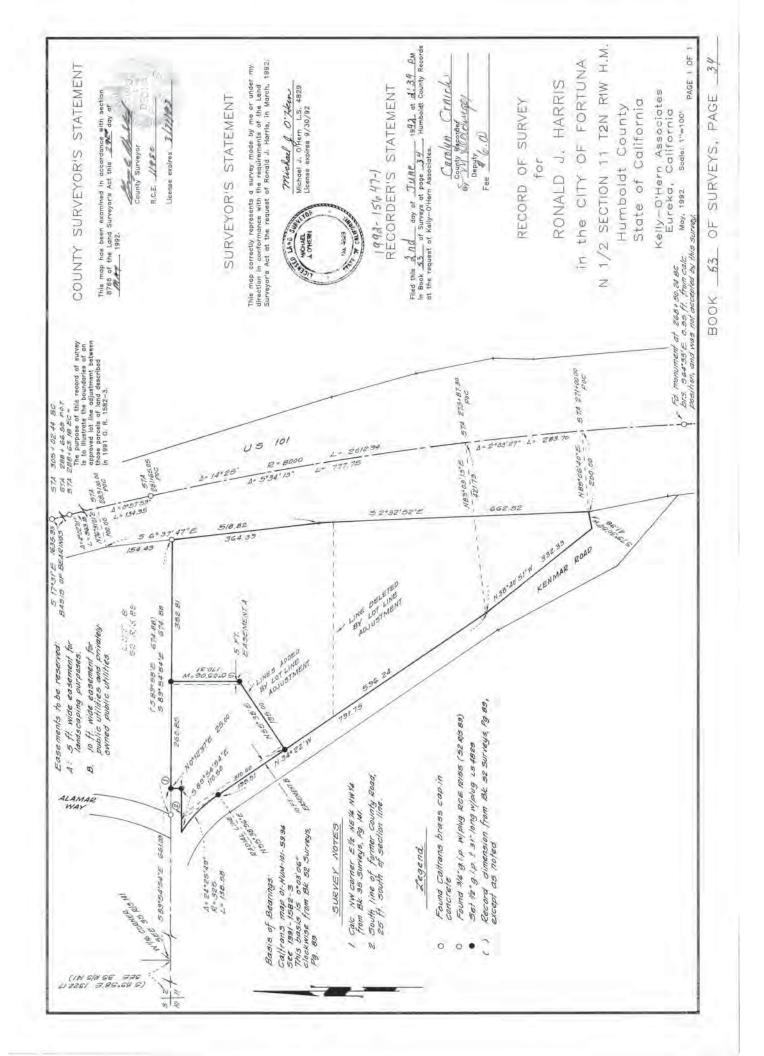












SCANNED

111

BOARD OF SUPERVISONS, COUNTY OF HUMBOLDT, S. ATE OF CALIFORNIA Certified copy of portion of proceedings, Meetings of \_\_\_\_\_\_

#### IN THE MATTER OF FRECUTING INDERTURE WITH MORTHWRETERS PACIFIC BAILROAD COMPANY CONCERNING NEWBURG DRAINAGE STRUCTURE.

Upon the mution of Supervisor Barelikes, seconded by Supervisor Soberlaon, Sam B. Marryman, Jr., Chairman of this Board of Supervisors, is hereby authorized to exercise on bobalf of the County of Eucobold's an Indoneuro, dated this date, by and between the County of Rembeld's and the Herthwestern Pacific Malleond Company. Sold indonture grants to the County the right to construct, reconstruct, maintain and operate a drainage structure beneath that cortain property of sold Satiroad as referred to therein and related to the Herthwestern The Auditor is hereby authorized and directed to draw a warranti payable to the Herthwestern Pacific Satiroad Company in the amount of fifty delivers (650, 66) as consideration in full for the sized of containing to the second of fifty delivers (650, 66) as consideration in full for the sized of cald indentore.

AYES: Supervisors- Lindley, Bereilics, Robertonn, Fellerson, Merryman NOES: Supervisors- None ABSENT: Supervisors- None

# STATE OF CALIFORNIA,

County of Humboldt

I, FRED J. MOORE, JR., County Clerk of the County of Humboldt, State of California, and exofficio Clerk of the Board of Supervisors of the County of Humboldt, do hereby certify the foregoing to be full, true and correct copies of the original orders made in the above entitled matters by said Board of Supervisors, at a meeting held in Eureka, California, on **June 1. 1960** and as the same now appears of record in my office.

IN WITNESS WHEREOF, I have hereunto set my hand and affixed the Seal of said Board of Supervisors this

day of 1000, 1000 FRED J. MOUGHE, JR. County Clerk and ex-officio Clerk of the Board of Supervisors of the County of Humboldt, State of California Bv Deputy Clerk.

LAN

THIS INDENTURE, made this \_\_\_\_\_ day of \_\_\_\_\_, 1959, by and between NORTHWESTERN PACIFIC RAILROAD COMPANY, a corporation of the State of California, herein termed "Railroad", and COUNTY OF HUMBOLDT, a political subdivision of the State of California, herein termed "Grantee".

WITNESSETH:

1. Railroad, for and in consideration of the faithful performance by Grantee of all the terms, covenants and conditions herein contained, hereby grants to Grantee the right to construct, reconstruct, maintain and operate a 42-inch reinforced concrete drainage pipe, hereinafter termed "structure" beneath the property of Kailroad, at or near Fortuna, in the County of Humboldt, State of California, in the location shown enclosed within red lines upon the print of Kailroad's San Rafael Drawing X-6778, revised October 23, 1959, hereto attached and made a part hereof.

This indenture will be supplemented to include a legal description of the property if requested by either party in writing.

use all the property describe herein in the performance of its duty as a common carrier, and, for that purpose, there is reserved unto Railroad, its successors and assigns, the right (consistent with the rights herein granted) to construct, reconstruct, maintain and use existing and future railroad tracks, facilities and appurtenances and existing and future transportation, communication and pipe line facilities and appurtenances in, upon, over, under. across and along said property.

3. This grant is made subject to all licenses, leases, easements, restrictions, conditions, covenants, encumbrances, liens and claims of title which may affect said property and the word GRANT as used herein shall not be construed as a covenant against the existence of any thereof.

4. The rights merein granted to brantse shall lapse and become void if the construction of said structure upon said property is not commenced within one (1) year from the date first herein written.

5. Grantee shall bear the entire cost and expense of constructing, reconstructing and maintaining said structure upon said property. Grantee agrees that all work upon or in connection with said structure shall be done at such times and in such manner as not to interfere in any way what-soever with the operations of Railroad. The plans for and the construction of said structure shall be subject to the approval of Railroad.

Grantee agrees to reimburse Railroad for the cost and expense to Railroad of furnishing any materials or performing any labor in connection with the construction, reconstruction, maintenance and removal of said structure, including, but not limited to, the installation and removal of such falsework and other protection beneath or along Railroad's tracks, and the furnishing of such watchmen, flagmen and inspectors as Railroad deems necessary.

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6. Generations consignation as a structure material with a set of the interaction of the standard of the structure of the set of the structure of the set of the structure of the set of th

7. Grantee, its agents and employees, shall have the privilege of entry on said property for the purpose of constructing, reconstructing, maintaining and making necessary repairs to said structure. Grantee agrees to keep said property in a good and safe condition free from waste, so far as affected by Grantee's operations, to the satisfaction of Railroad. If irantee fails to keep said property in a good and safe condition free from waste, then Railroad may perform the necessary work at the expense of irantee, which expense Grantee agrees to pay to Railroad upon demand.

8. In the event any work upon or in connection with said structure or its appurtenances, to be done upon or adjacent to the tracks and property of Railroad, should be let to a contractor by Grantee, such work shal not be begun until such contractor the track and end of a agreement with the railroad company works and the track and against all laims, liability, cost and experimented on a contractor of the perk to be done by such contractor.

Such contractor shall lurni , as one option of and without exmonse to Railroad, a reliable surery lond, in a constant and in a form satisfactory to said company. guarantees of thill performance of all the terms, covenants and conditions contained in said agreement, and a certified copy of a policy of Public Liability and Property Damage Insurance, within limits specified by, and in a form satisfactory to, said company, covering the contractual liability assumed by contractor in said agreement to be entered into with said company by such contractor.

9. Should Grantee, its successors or assigns, at any time abandon the use of said property or any part thereof, or fail at any time to use the same for the purpose contemplated herein for a continuous period of one (1) year, the right hereby given shall cease to the extent of the use so abandoned or discontinued, and Railroad shall at once have the right, in addition to but not in qualification of the rights hereinabove reserved, to resume exclusive possession of said property or the part thereof the use of which is so discontinued or abandoned.

Upon termination of the rights and privileges hereby granted, Grantee, at its own cost and expense, agrees to remove said structure from said property and restore said property as nearly as practicable to the same state and condition in which it existed prior to the construction of said structure. Should Grantee in such event fail, neglect or refuse to remove said structure and restore said property, such removal and restoration may be performed by Railroad at the expense of Grantee, which expense Grantee agrees to pay to Railroad upon demand

10. This indenture shall inure to the benefit of and be binding upon the successors and assigns of the parties hereto.

11. For the rights herein given, Grantee shall pay to Railroad the sum of Fifty (50) Dollars.

IN WITNESS WHEREOF, the parties hereto have caused these presents to be executed as of the day and year first herein written. (In duplicate)

NORTHWESTERN PACIFIC RAILROAD COMPANY

By\_\_\_\_\_("itle)

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(Assistant Secretary)

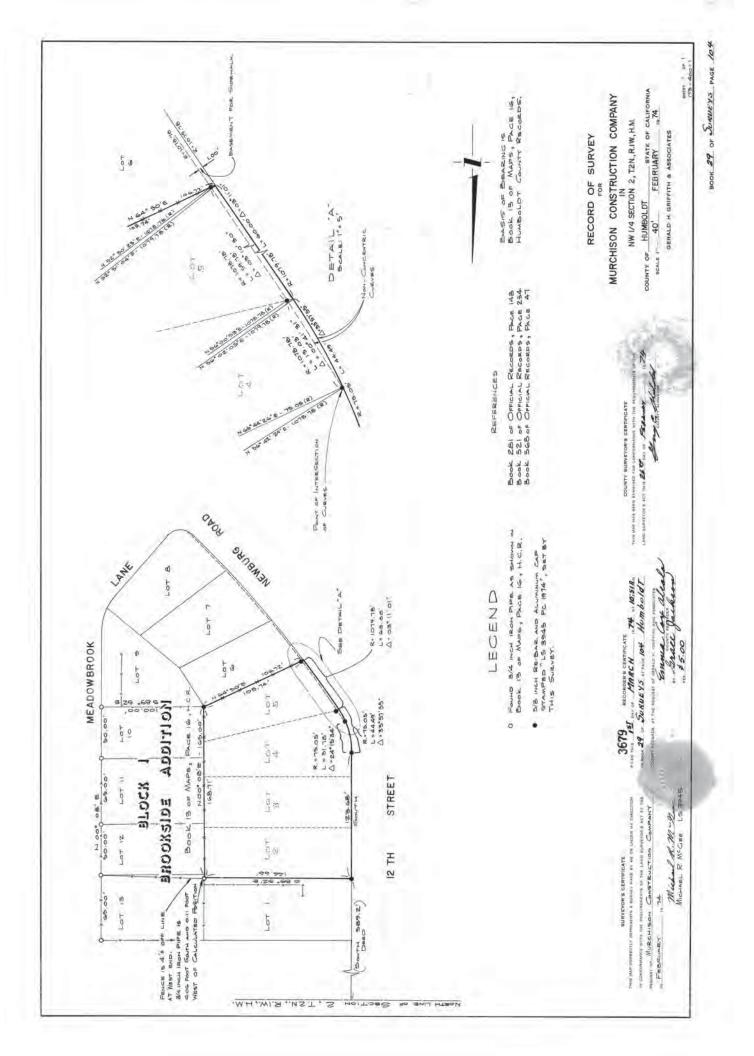
COUNTY OF HUMBOLDT

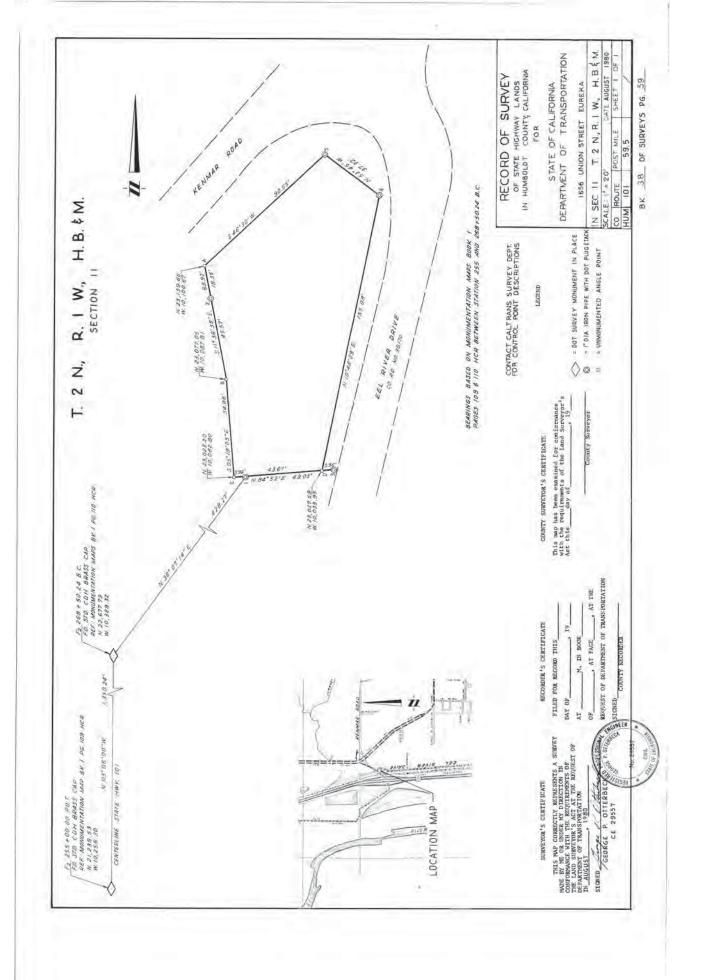
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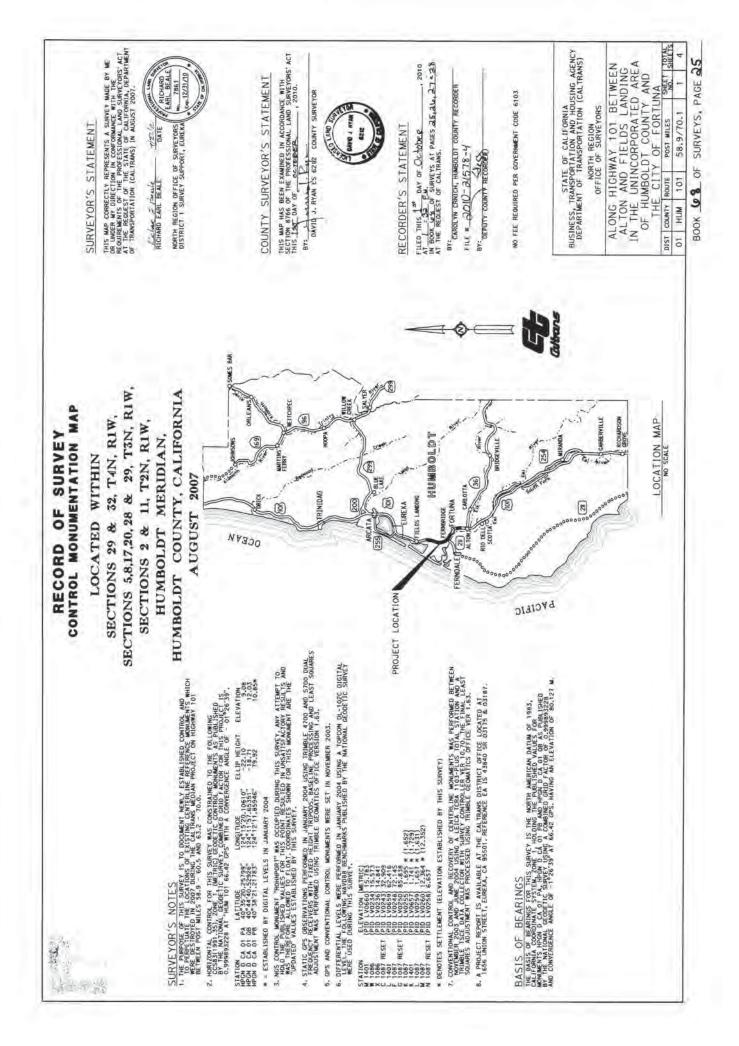
By Chairman, Board of Supervisors

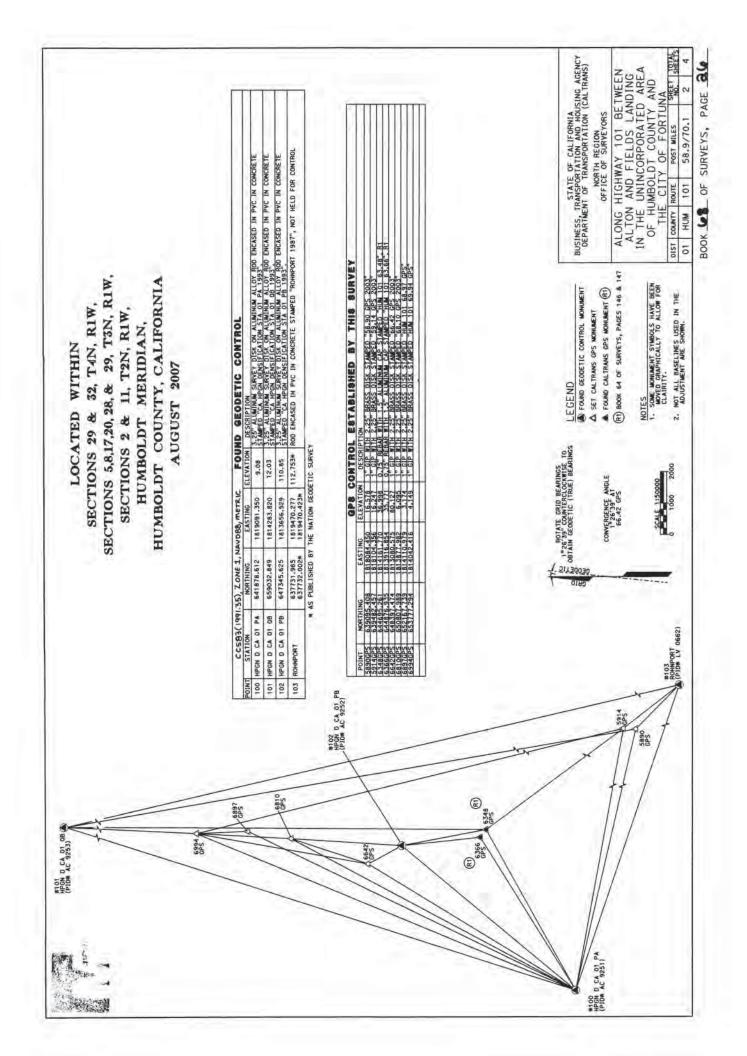
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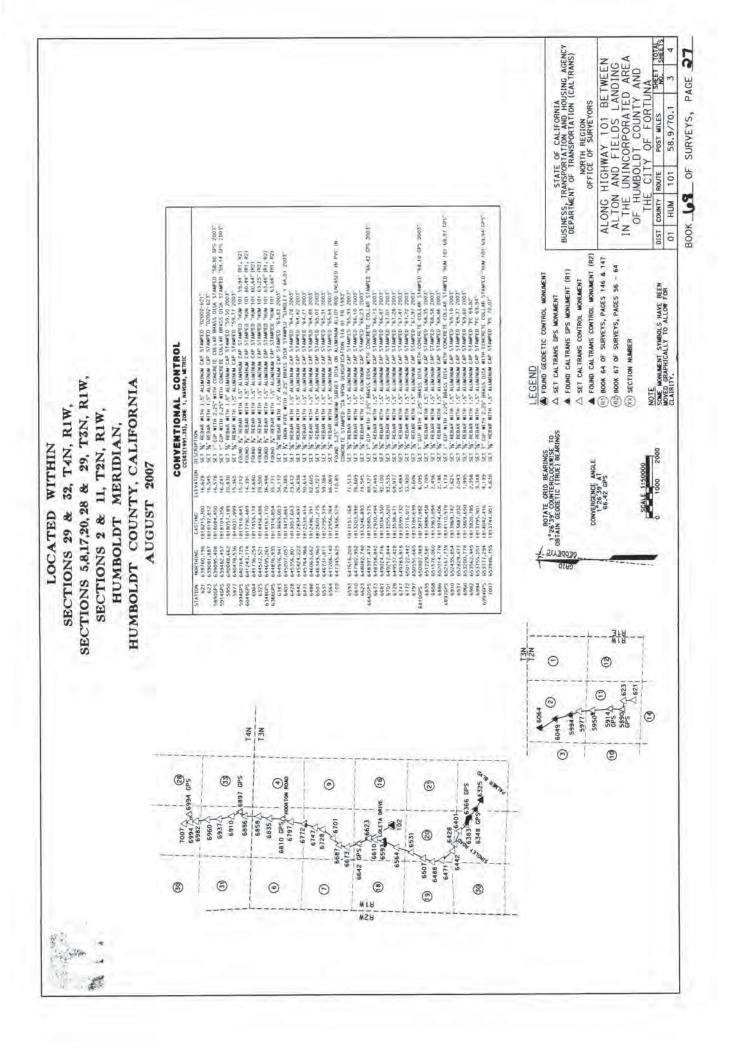
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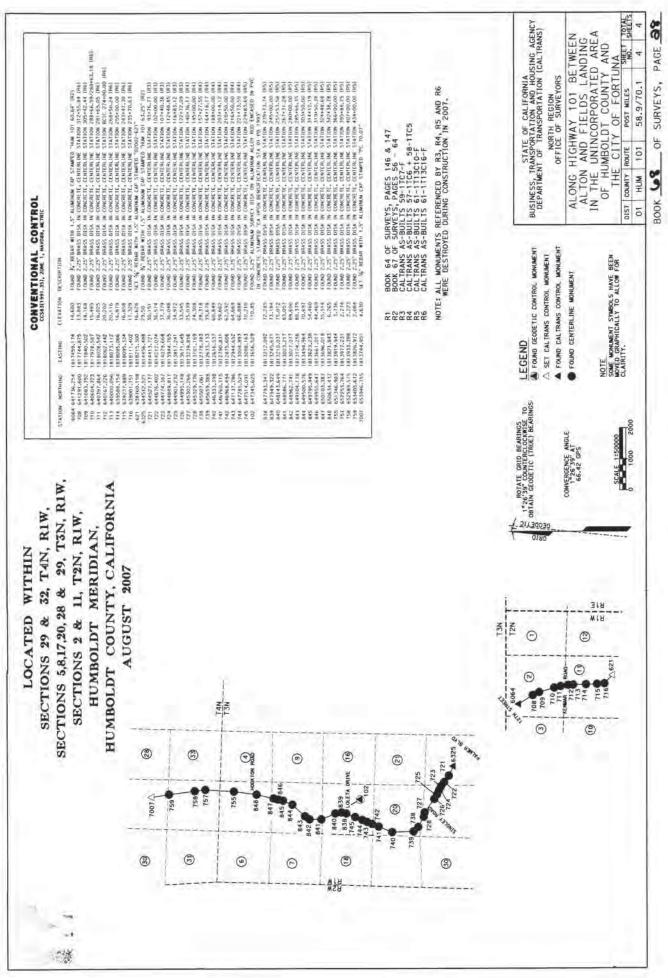


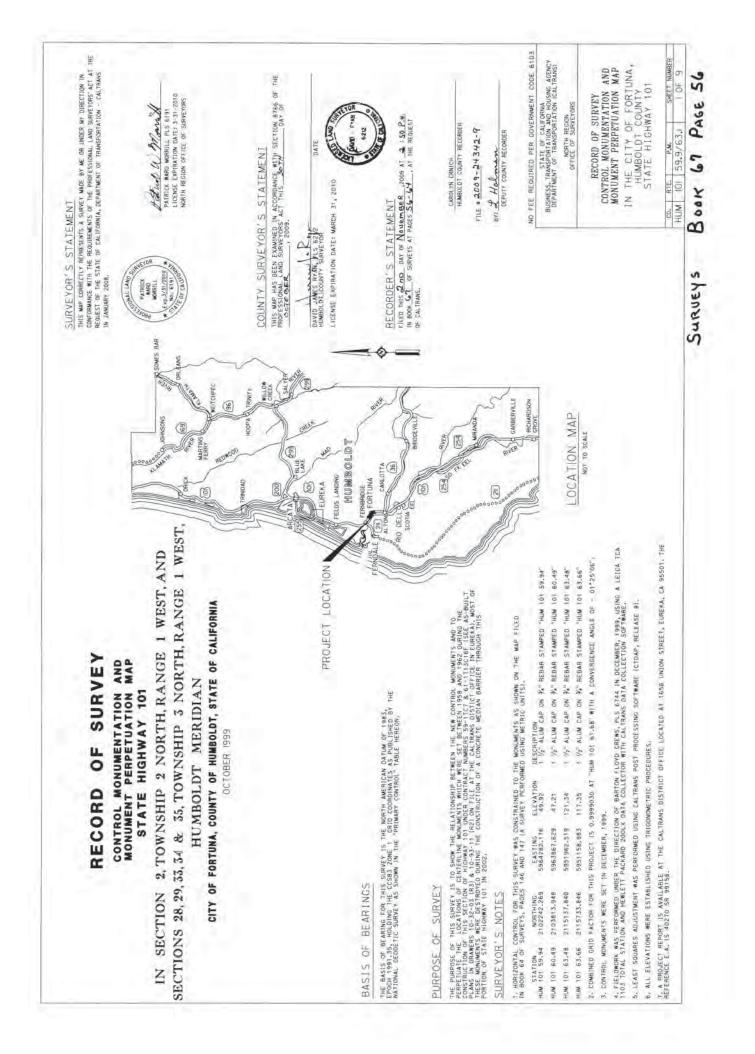


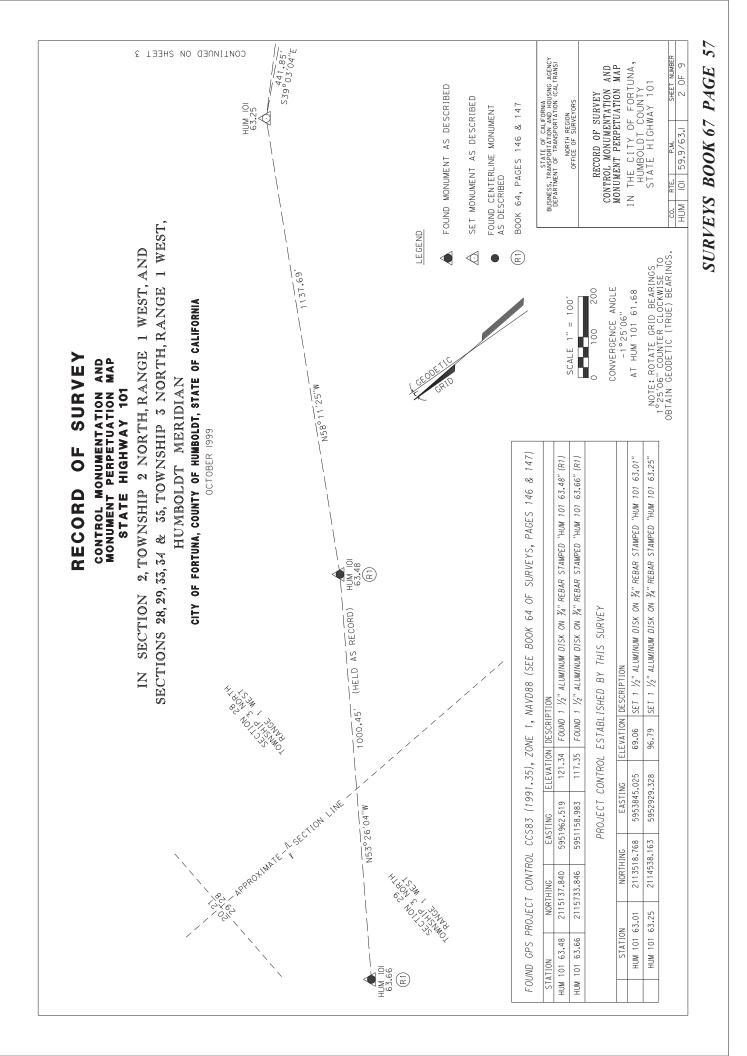


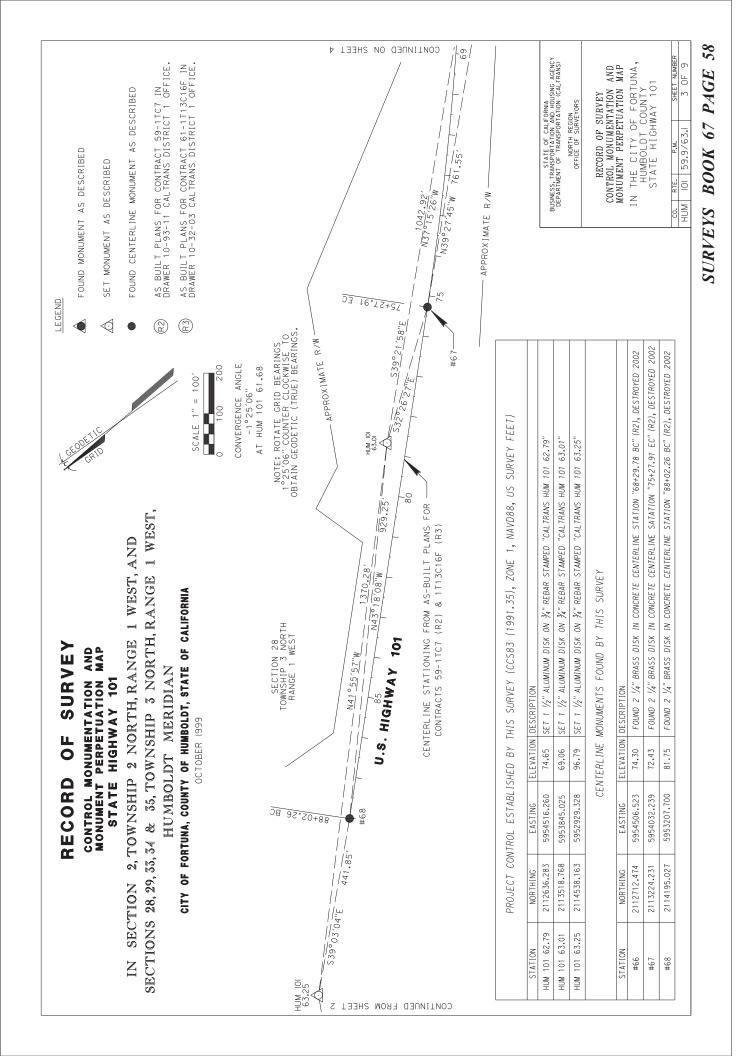


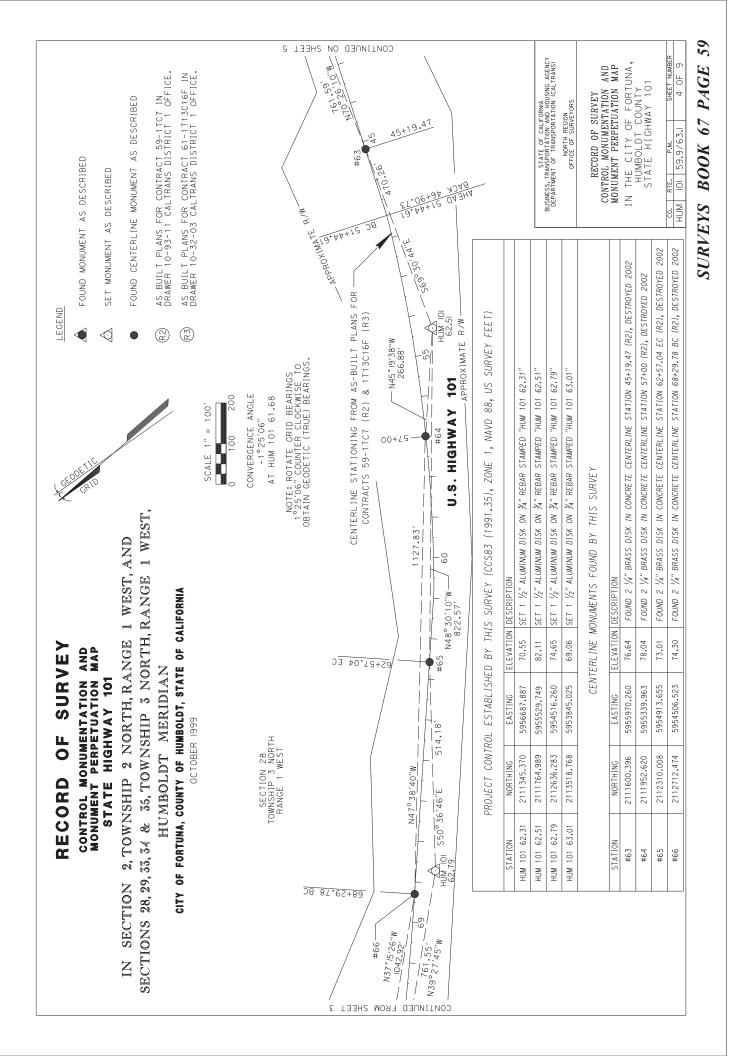


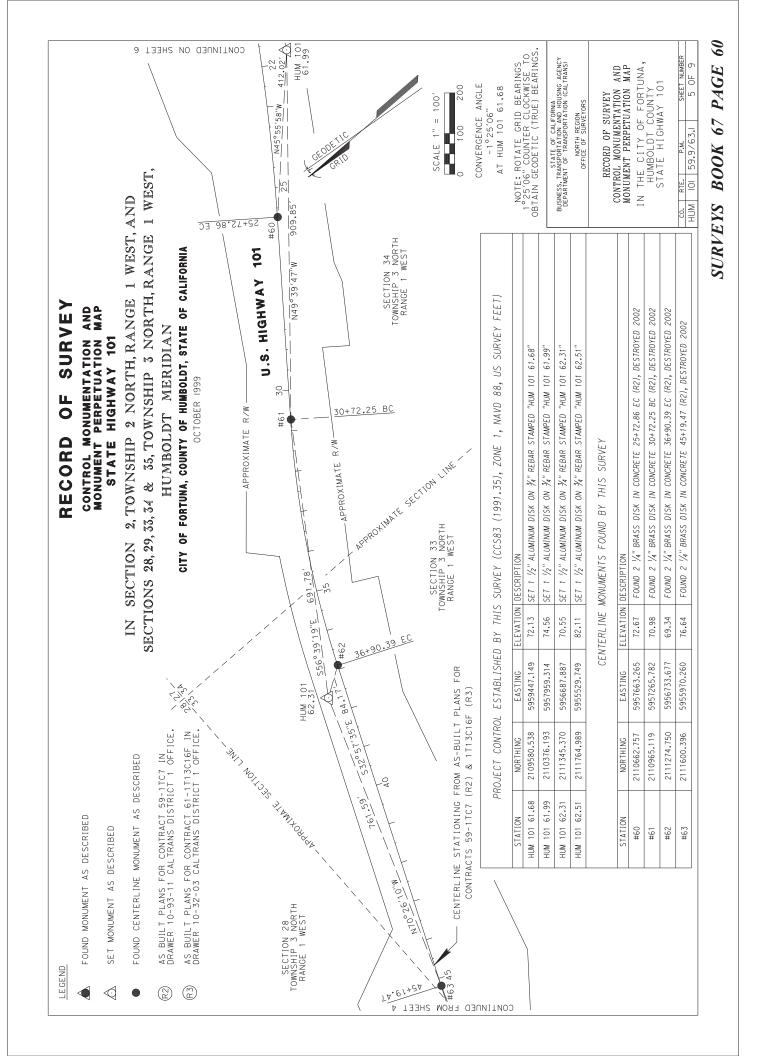


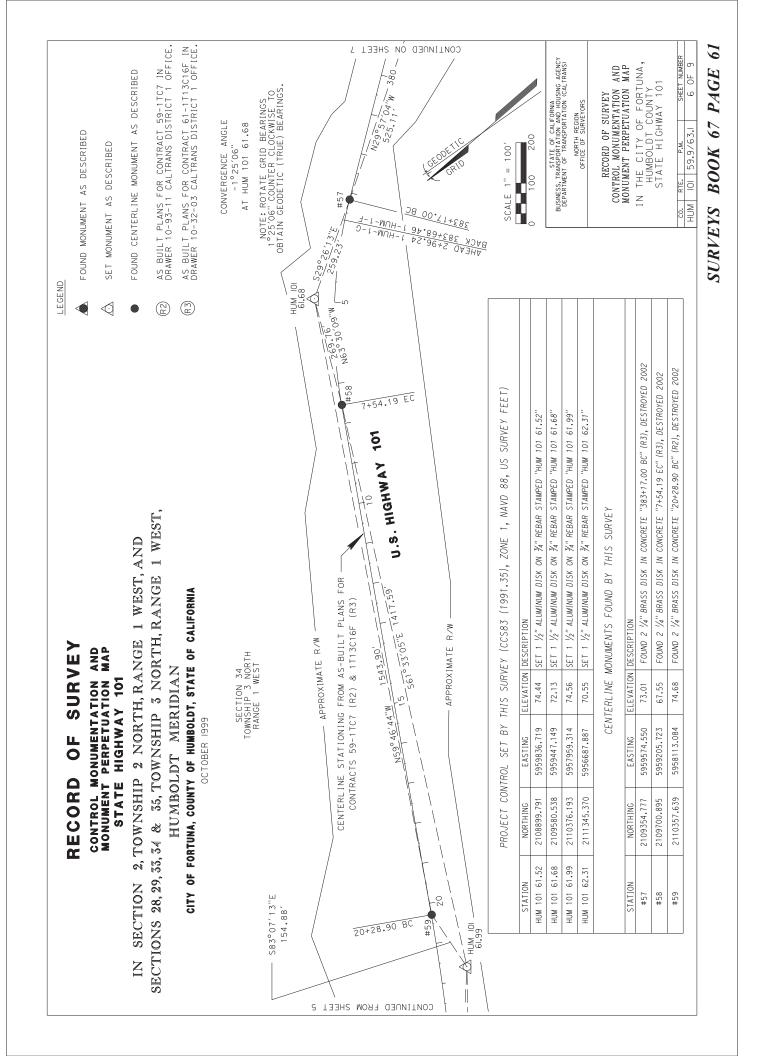


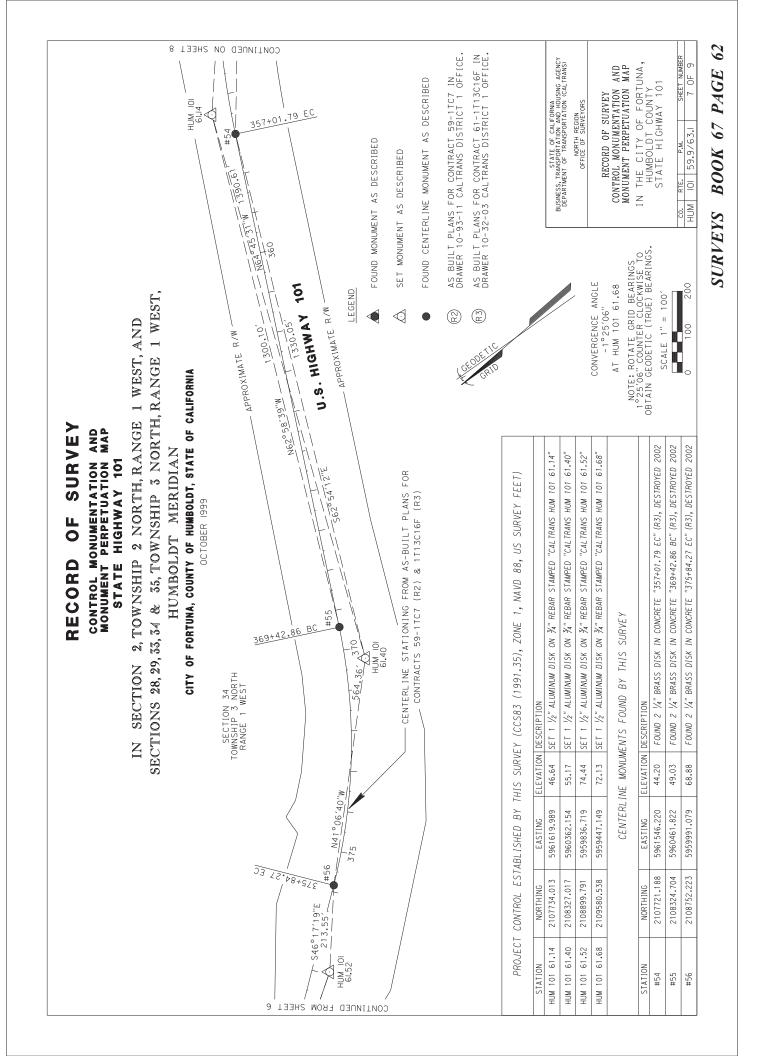


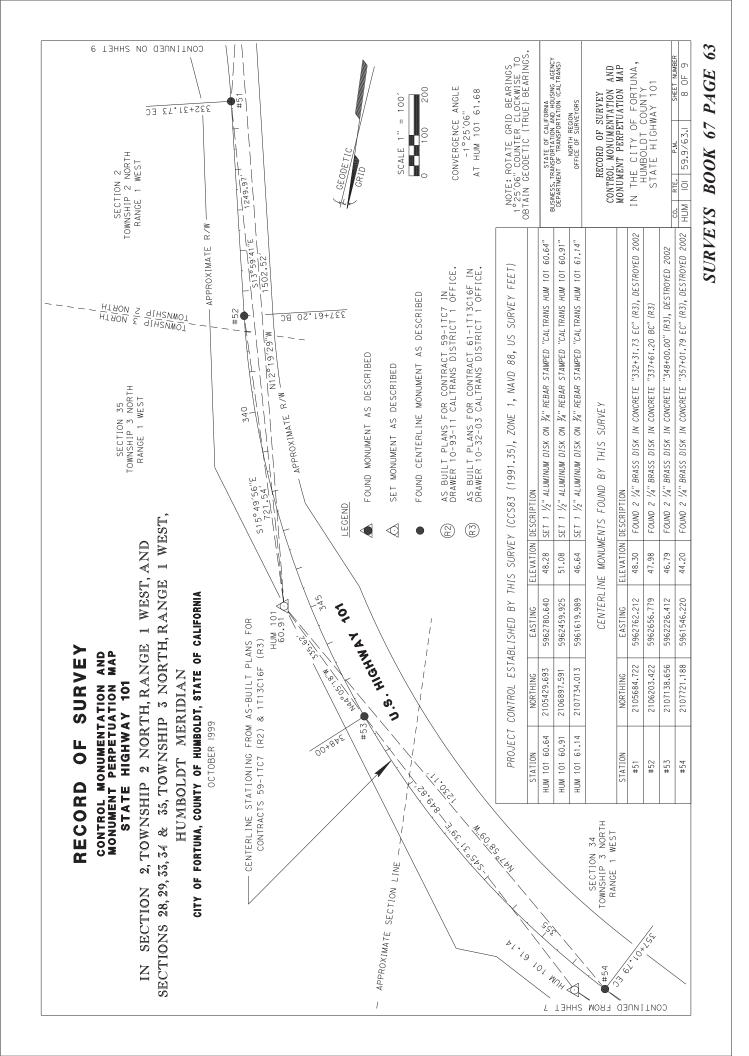


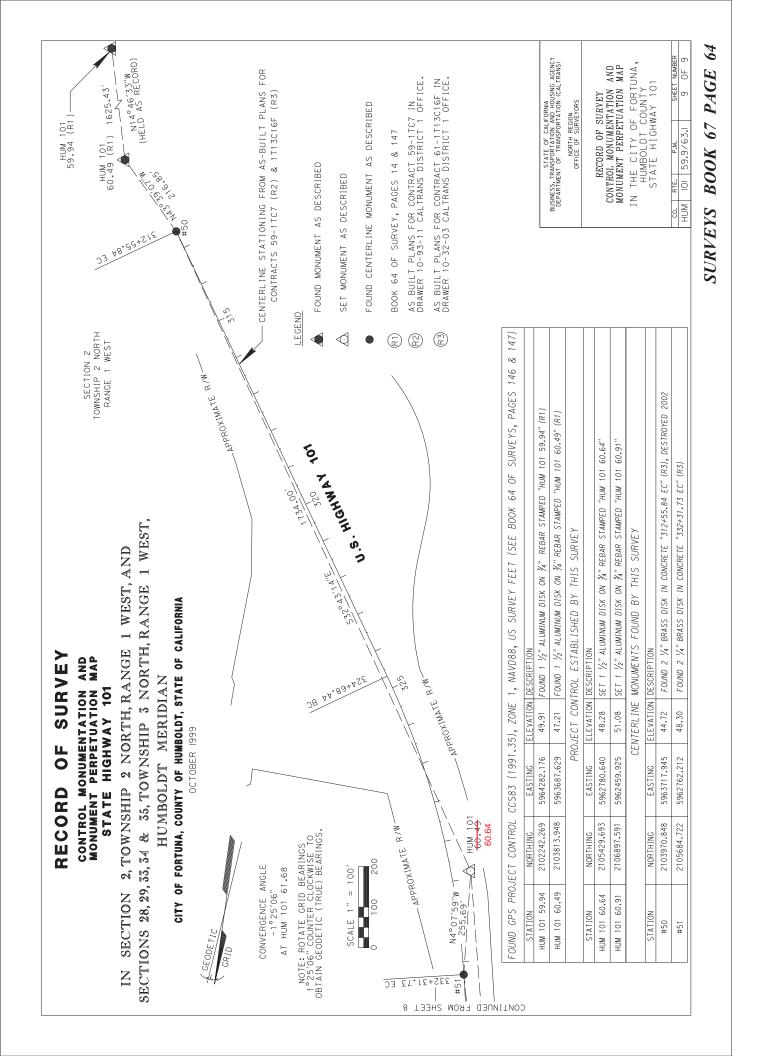












200-353.021



DEPARTMENT OF PUBLIC WORKS COUNTY OF HUMBOLDT

MAILING ADDRESS: 1106 SECOND STREET, EUREKA, CA 95501-0579 AREA CODE 707/FAX 445-7409

839-5401 AVIATION

PUBLIC WORKS BUILDING SECOND & L'ST., EUREKA						
ADMINISTRATION BUSINESS ENGINEERING	445-7491 445-7652 445-7377 ARCHITEC	NATURAL RESOURCES PARKS ROADS & EQUIPMENT MAINT. 445-7493	445-7741 445-7651 445-7421			

CLARK COMPLEX HARRIS & H ST., EUREKA				
LAND USE	445-7205			

January 10, 2005

Duane Rigge, City Manager City of Fortuna POB 545 Fortuna CA 95540

#### ANNEXATION BY THE CITY OF FORTUNA OF COUNTY ROADS ADJACENT RE: TO THE CITY OF FORTUNA

Allen Campbell, Director, Humboldt County Department of Public Works, has requested that I provide you some information. It is in response to a request by you associated with roads proposed for annexation that were discussed in our previous letters. The previous letters discussed the annexation of Strongs Creek Road (known by the City as Dinsmore Drive), Twelfth Street, a portion of Rohnerville Road, a portion of Eel River Drive, and Drake Hill Road.

The information requested was regarding the County's cost of maintenance of the roads over the last five years and what it would cost if the County was to perform the desired future maintenance of the surface of the roads. The following is a description of the road, the County's past cost of the maintenance of the road, and an estimate of the cost to maintain the roads to the desired standard. The desired standard of maintenance consists of sealing the roads at a 12 to 14 year period and resurfacing the roads every 25 years. The County cost for sealing a road is estimated to be \$0.12 per square foot and a \$1.30 per square foot for resurfacing a road. These are the prices used currently by the County in the estimation of costs for maintenance of new subdivision roads.

## STRONGS CREEK ROAD

This portion of road lies south of the city limits of Fortuna to the intersection of Riverwalk Drive. The County has not spent any funds on this road for the last 5 years or longer. When the City annexed the portion of land at the end of the road, the then city manager of Fortuna provided a letter to LAFCO and the County indicating that they would maintain this portion of Strongs Creek Road for the County as consideration of approval of the annexation.

The portion of Strongs Creek Road in the County is approximately 640 feet in length and has an average width of 26 feet. The cost of sealing the road would be estimated at \$2,000 and \$21,632 for resurfacing the road at today's cost.

#### TWELFTH STREET

This portion of road lies within the limits of the CalTrans right of way for State Highway 101. It lies between the railroad tracks, over the overpass, to the intersection of Strongs Creek Road and Riverwalk Drive. The County entered into an agreement with CalTrans to be responsible for the surface of the road. CalTrans is responsible for the overpass structure.

The County has spent \$37 over the last five years on this portion of road. The length of road is approximately 1,530 feet and has an average width of 30 feet. The cost of resealing the road is estimated at \$5,500 and \$69,670 for resurfacing the road.

#### ROHNERVILLE ROAD

This portion of road lies north of Loop Road to the south line of the park. The road has been presumed by the City to be located in the City. The Campton Heights and Fortuna High School annexations by the City appear to have not included this portion of road. The County has not been providing maintenance of the road. The road was presumed by the City to be within the city limits of Fortuna and has been maintained by the City for a number of years.

#### EEL RIVER DRIVE

This portion of road lies between Drake Hill Road and Kenmar Road. It does not front any developable property located in the County. CalTrans and NWP Railroad front the west side of the road. The entire east side of the road serves lands located in the City. It is approximately 0.93 mile in length and has an average width of 24 feet. The County has spent approximately \$26,000 on this road in the last 5 years. The majority of the cost was associated with clean up of the movement of earth off of property owned by the City. The slide occurred in 2002.

The cost of resealing the road is estimated to be approximately \$14,142 and \$153,205 for resurfacing the road.

#### DRAKE HILL ROAD

This portion of road is located between Rohnerville Road and Eel River Drive. The City is located along the entire north side of the road. The north side of the road is zoned and planned for residential development. The south side of the road located in the County is mainly zoned and planned for agricultural use. The road is 1.25 miles in length and is approximately 22 feet in width. The County has spent approximately \$79,000 in maintenance of this road in the last 5 years. Approximately \$67,000 of the maintenance cost was associated with resurfacing Drake Hill Road between Thelma Drive and Rohnerville Road in 2002. This stretch of road is where the heaviest density of residential property is located within the City.

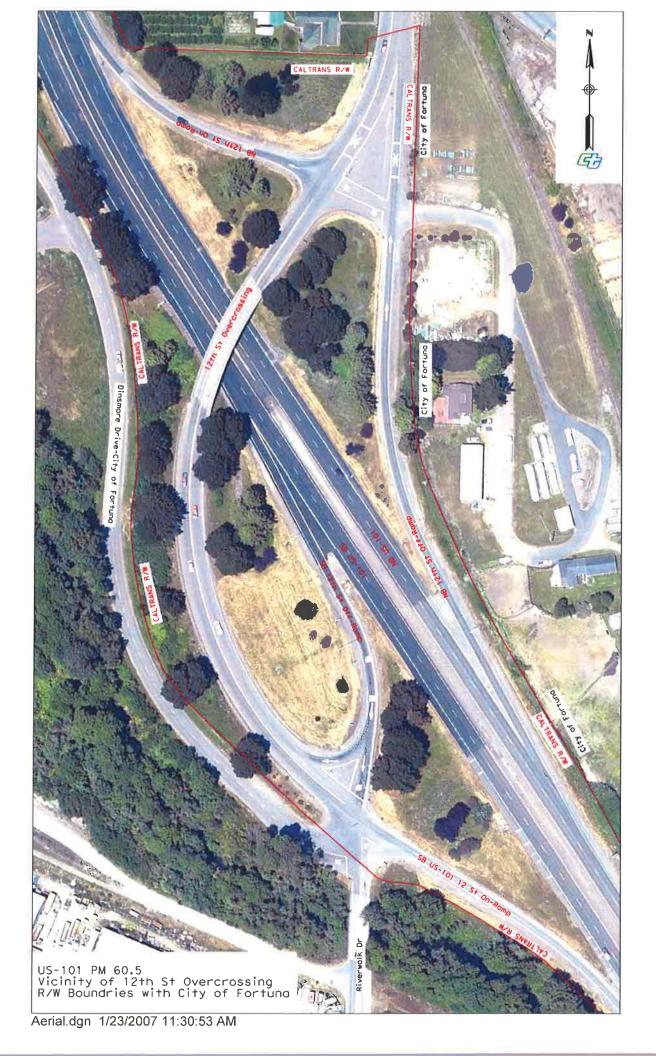
The cost of resealing the road is estimated to be approximately \$17,424 and \$188,760 is estimated for resurfacing the road.

If you have any additional questions regarding the roads or this letter, please don't hesitate to contact this office.

Sincerely,

Jan

Harless McKinley Land Use Division 445-7205



I-Hum-l-F,G

#### FREEWAY MAINTENANCE AGREEMENT

THIS AGREEMENT, made and entered into, in duplicate, this <u>15th</u> day of <u>April</u>, 1963, by and between the State of California, acting by and through the Department of Public Works, Division of Highways, hereinafter for convenience referred to as "the State", and the County of Humboldt, hereinafter for convenience referred to as "the County", witnesseth:

WHEREAS, on November 6, 1958, a Freeway Agreement was executed between the County and the State relating to the development of that portion of State Highway Route 1 in the County of Humboldt between 0.6 mile north of Route 35 and 0.3 mile north of Fortuna as a freeway, and

WHEREAS, under the provisions of said Freeway Agreement, the County agreed to certain adjustments in the County road system, and for the carrying of certain County roads over or under or to a connection with the freeway, and

WHEREAS, said freeway has now been completed or is nearing completion, and the parties mutually desire to clarify the division of maintenance responsibility as to separation structures, and County roads or portions thereof, within the freeway limits.

NOW, THEREFORE, IT IS AGREED:

1. ROADWAY SECTIONS

The County will maintain, at County expense, all portions of County roads and appurtenant structures and bordering areas, colored in yellow on the attached map marked Exhibit "A" and made a part hereof by this reference.

#### 2. VEHICULAR OVERCROSSINGS

The State will maintain, at State expense, the entire structure below the top of the concrete deck surface, exclusive of any bituminous surface treatment thereof. The County will maintain, at County expense, the top of the concrete deck surface, together with any bituminous surface treatment thereon, and all portions of the structure above the concrete deck surface, and shall perform such other work as may be necessary to insure an impervious and otherwise suitable surface. The County will also maintain all traffic service facilities provided for the benefit or control of County road traffic.

#### 3. VEHICULAR UNDERCROSSINGS

The State will maintain the structure proper. The roadway section, including the traveled way, shoulders, curbs, sidewalks, walls, drainage installations and traffic service facilities, will be maintained by the County.

#### 4. EFFECTIVE DATE

This agreement shall be effective upon the date of its execution by the State; it being understood and agreed, however, that the execution of this agreement shall not affect any pre-existing obligations of the County to maintain designated areas pursuant to prior written notice from the State that work in such areas, which the County has agreed to maintain pursuant to the terms of the Freeway Agreement, has been completed.

> STATE OF CALIFORNIA DEPARTMENT OF PUBLIC WORKS DIVISION OF HIGHWAYS

J. C. WOMACK STATE HIGHWAY ENGINEER

Approval Recommended

SAM HELWER District Engineer

EDWARD L. TINNEY Maintenance Engineer

Approved as to form:

RICHARD C. EAST Attorney for Department

Attorney

By <u>CHAS. E. WAITE</u> Deputy State Highway Engineer

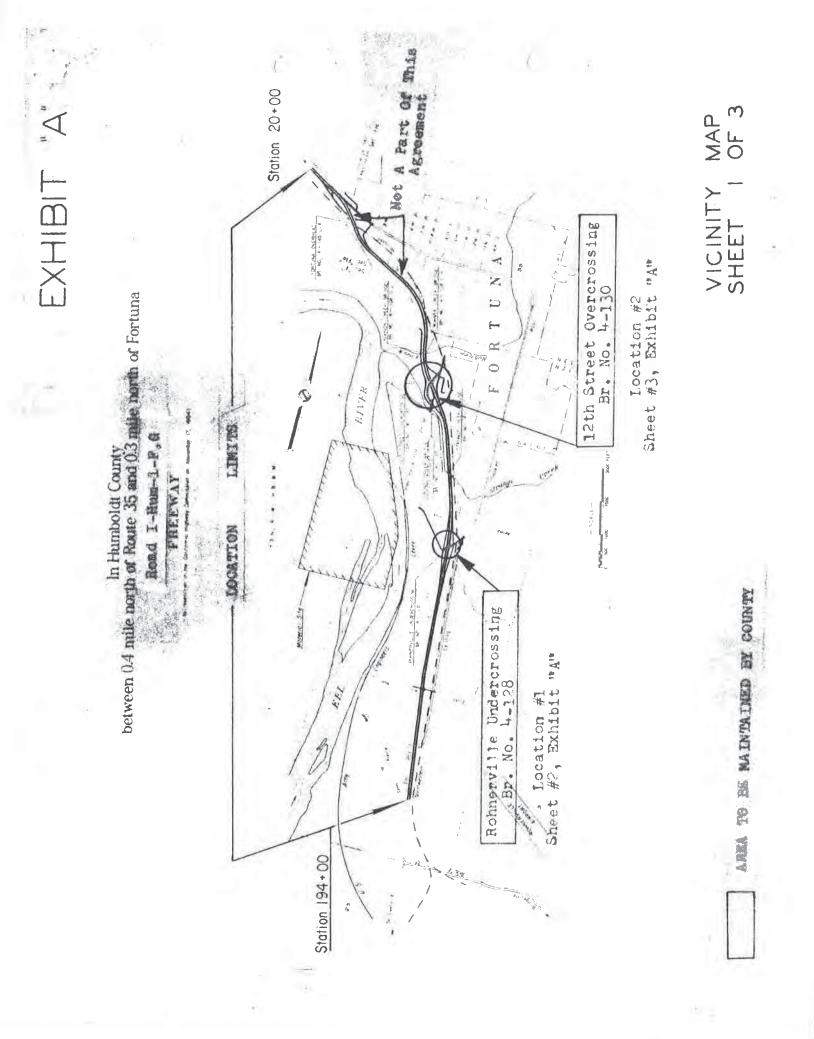
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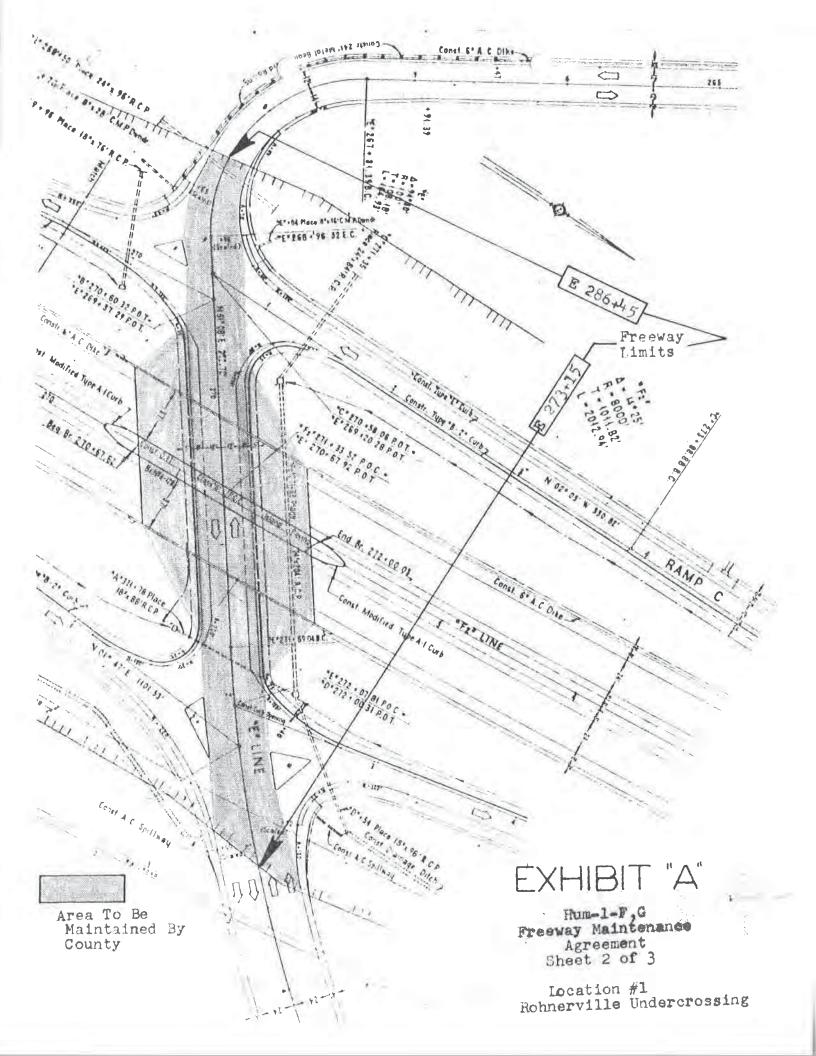
COUNTY OF HUMBOLDT

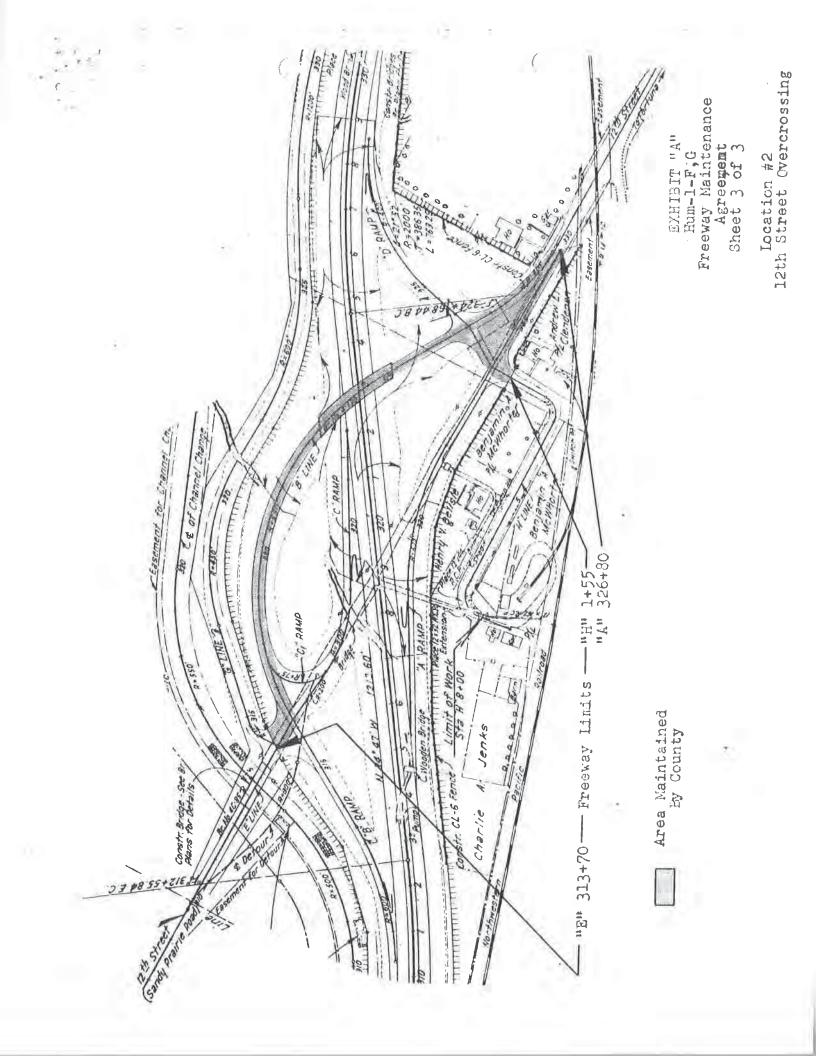
By <u>NORMAN R. ROBERTSON</u> Chairman, Board of Supervisors

By FRED J. MOORE, Jr. (SEAL) County Clerk

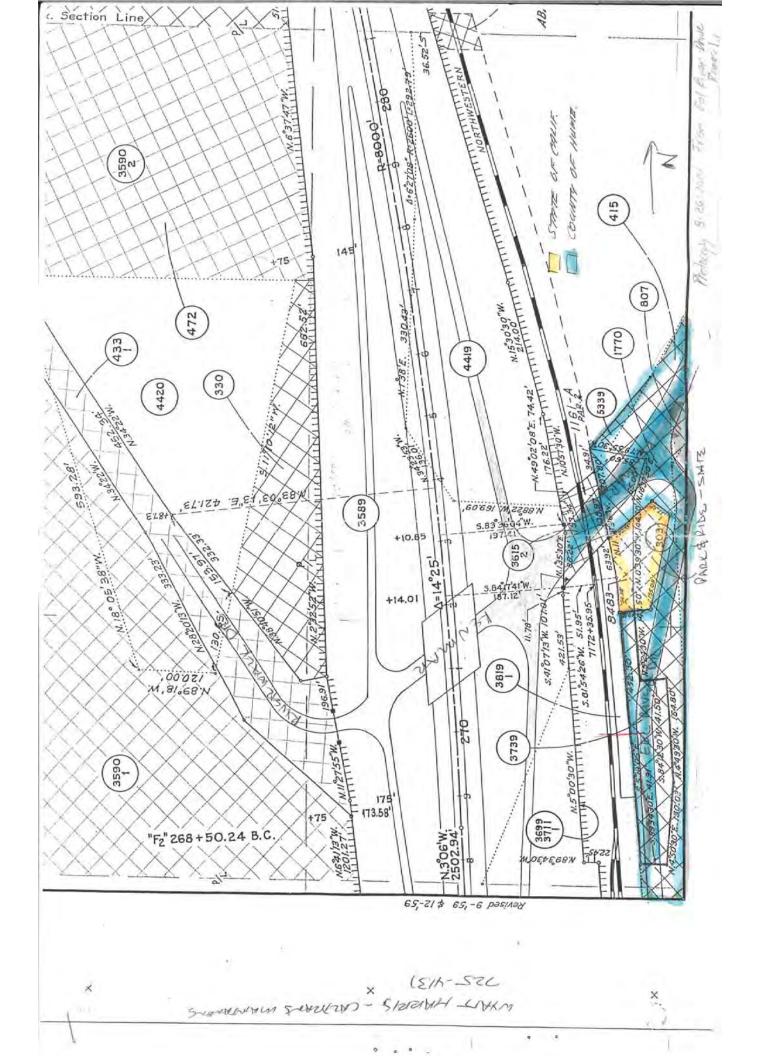
By W. E. SCHUSSMAN











#### Bisiar, Jim

From: Sent: To: Subject: Ryan, Dave Friday, December 05, 2014 4:27 PM Bisiar, Jim Eel River Drive R/W

#### Jim,

I couldn't find any more ways to check or calc the north ¼ corner and I didn't find any references in deeds to the NE corner of Sec. 2, so I gave up on that.

For what you send to Jill, I say skip the PC-Survey plot and tape together the highway maps that you put pt. nos. on, scan it and send that. I drew our R/W with a blueish/green marker. A plot may not even be necessary-pt. nos. just get jumbled. Craig could scan it-or if he's not around, Jason or I could help you do that. I want to see it anyways before we scan it.

As we talked about, recalc the south end of 202 Deeds 93 to be at sta. 279<sup>2</sup> rather than using the acreage calc. Calc the west side of this R/W up to just south of where the RR starts to curve-to your box for pt. no. 359. Also calc the little strip that got left out near your pt. nos. 308 & 317. Calc area 145- that's the notch the State granted back to East along the easterly R/W, that is not now R/W.

I need to look at the road register for Drake Hill Rd, but haven't done that yet.

Add the following note:

R/W for Eel River Drive in the area of this project (Drake Hill Road to Kenmar) is per Relinquishment No.<sup>4</sup>15 to County of Humboldt from Caltrans per 722 OR 640\*, recorded 2/7/1963. The right-of-way was initially state highway (1917 alignment and 1931 widening). The southerly end of the project consists of uniform width strips, so both sides of the County R/W have been calculated for this area. The northerly end is variable and complex where it approaches Caltrans highway 101 R/W, so we therefor did not calculate the westerly limits of the R/W for this area. Further work would be required if this location is deemed necessary by Design engineers.

The R/W as shown is preliminary as more field ties to controlling monuments would be necessary to depict it with greater accuracy. If work is contemplated in close proximity to any existing R/W lines shown, Surveys should be notified in order to verify accuracy. Proposed acquisitions for any new easements will require additional survey work.

#### Additional information regarding R/W

Initial fieldwork did not entail making any ties for determining R/W. After the topographic survey was done, minimal additional fieldwork was done to attempt to locate R/W. In the course of R/W research and analysis, it was ultimately determined certain section and quarter corners, plus other monuments cited within R/W deeds needed to be located. Many of these are either destroyed or could not be found through quick searches. Positions have been calculated primarily through data contained in the 1917 and 1931 alignments noted in deeds and highway maps, originating near the Alton interchange using Caltrans control. R/W analysis required analyzing approximately 20 deeds and obtaining old maps and other data from Caltrans archives, most of which was not on file in County RW records.

Dave



Attachment K - Environmental Constraints Analysis



# **Environmental Constraints Analysis**

Fortuna Highway 101/Riverwalk Area Connectivity Project

May 2016

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Appendix B (CNDDB Occurrence Report)

Appendix C (Site Photographs)

## 1. Introduction

## 1.1 **Project Summary**

The Fortuna Highway 101/Riverwalk Connectivity Planning Study focuses on the 12<sup>th</sup> Street and Kenmar Road crossings of Highway 101, and includes an evaluation of the existing conditions, identification of deficiencies from Caltrans standards, and the development of conceptual alternatives intended to provide multi-modal mobility and accessibility for all users through both interchanges, with the goal of improving safety and ensuring the continued commercial viability of the Riverwalk Area. The results of the study will provide the foundation for future project development phases with the goal of implementation of improvement projects at the 12<sup>th</sup> Street and Kenmar Interchanges.

The overall objectives of the project are to:

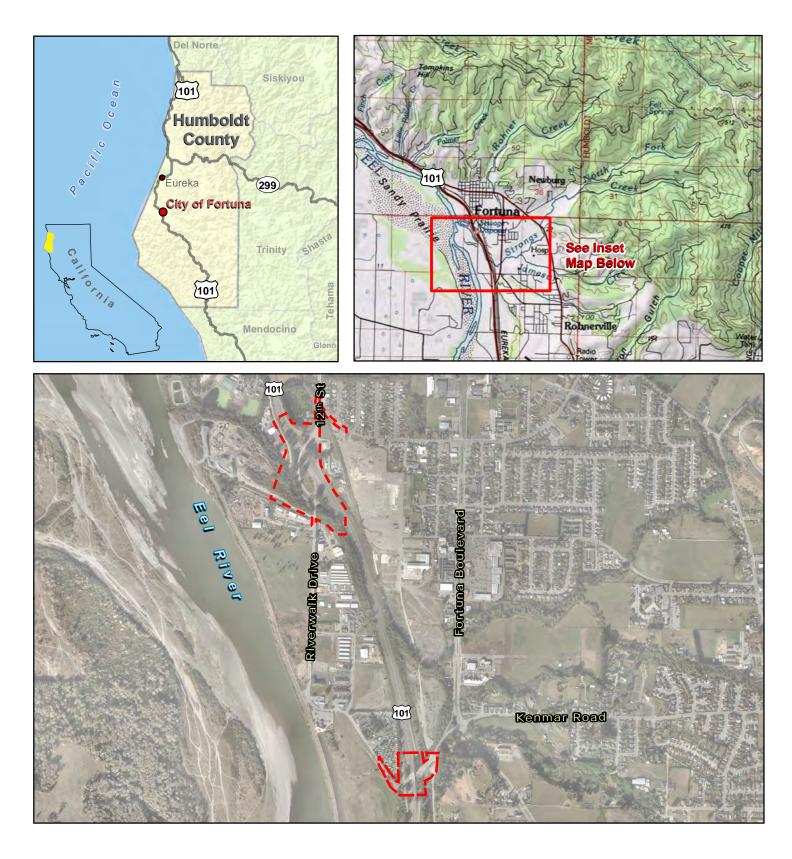
- Provide improved accessibility and connectivity between the Downtown and the Riverwalk Area for all users
- Support growth of business in the Riverwalk and Downtown areas by increasing the capacity of the 12<sup>th</sup> Street and Kenmar Interchanges while considering planned commercial growth
- Support economic growth by developing strategies to improve access to the Riverwalk and Downtown areas
- Improve the safety at the Kenmar and 12<sup>th</sup> Street Interchanges

### 1.2 Purpose of the Report

This Environmental Constraints Analysis is intended to document the biological conditions/constraints within the Study Area. A reconnaissance-level site investigation of existing conditions was conducted throughout the study area in February 2016, to identify the presence or potential presence of biological resources listed under the Federal Endangered Species Act (ESA), the presence of wetlands and Waters of the US as regulated by the US Army Corps of Engineers (USACE), the presence or potential presence of species listed as endangered or threatened under the California Endangered Species Act (CESA) or considered a species of special concern (SSC) by the California Department of Fish and Wildlife (CDFW), or the potential for special-status plant species having a rare plant ranking as determined by the California Native Plant Society (CNPS) rare plant inventory, and to present the potential of sensitive habitats as listed by the CDFW. This report also discusses the necessary steps required for the project to comply with federal, state, and local regulatory environmental compliance requirements and provides basic permit information. No permits or environmental compliance documents were collected, initiated, or completed for this effort, nor were regulatory agencies contacted for additional information.

### 1.3 Location

This Environmental Constraints Analysis is being undertaken in Fortuna, Humboldt County, California. Fortuna is approximately 14 miles south of Eureka and can be accessed from Highway 101. A vicinity map is included as Figure 1. The project study boundary (PSB) covers approximately 35 acres around the Kenmar Road and 12<sup>th</sup> Street interchanges for Highway 101. The PSB is depicted in Figures 2a and 2b and 3, and these areas were analyzed to evaluate the likeliness of environmental features and potential project constraints or likelihood of permitting requirements.





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Map Projection: Transverse Mercator Horizontal Datum: North American 1983 Grid: NAD 1983 UTM Zone 10N				gure 1

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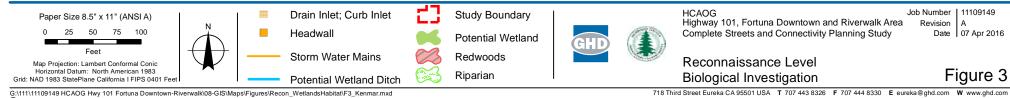


718 Third Street Eureka CA 95501 USA T 707 443 8326 F 707 444 8330 E eureka@ghd.com W www.ghd.com G:\111\11109149 HCAOG Hwy 101 Fortuna Downtown-Riverwalk\08-GIS\Maps\Figures\Recon\_WetlandsHabitat\F2a\_12th\_Street\_South.mxd © 2016. While every care has been taken to prepare this map, GHD make no representations or warranties about its accuracy, reliability, completeness or suitability for any particular purpose and cannot accept liability and responsibility of any way and for any repares. Losses, damages and/or costs (including indirect or consequential damage) which are or may be incurred by any party as a result of the map being inaccurate, incomplete or unsuitable in any way and for any reason. Data source: City of Fortuna GIS: utiliteis; GHD: wetland/habitat reconnaissance 2-29-2016 Created by:gldavidson



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G:\111\11109149 HCAOG Hwy 101 Fortuna Downtown-Riverwalk\08-GIS\Maps\Figures\Recon\_WetlandsHabitat\F3\_Kenmar.mxd 718 Third Street Eureka CA 95501 USA **T** 707 443 8 © 2016. While every care has been taken to prepare this map, GHD make no representations or warranties about its accuracy, reliability, completeness or suitability for any particular purpose and cannot accept liability and responsibility of any kind (whether in contract, tort or otherwise) for any expenses, losses, damages and/or costs (including indirect or consequential damage) which are or may be incurred by any party as a result of the map being inaccurate, incomplete or unsuitable in any way and for any reason. Data source: City of Fortuna GIS: Storm Drain, aerial imagery; GHD: wetland/habitat reconnaissance 2-29-2016 Created by:gldavidson

### 1.4 Overview of Study Area

The Study Areas are located in the western part of Fortuna, a city with a population of 11,926 as of the 2010 census. There are two distinct Study Areas located approximately one mile apart, and these are described in greater detail below.

The 12<sup>th</sup> Street PSB is an elongated irregularly shaped area oriented along the north-south centerline of 12<sup>th</sup> Street and Riverwalk Drive, and bisected by Highway 101 (Figure 2a and 2b). The area north of Highway 101 consists of paved roads and maintained grassy right of way with a few scattered ornamental trees, and is bordered by residential and commercial development. There are few natural feastures remaining in this section. South of Highway 101, Strong's Creek and associated riparian habitat makes up the southern end of the PSB, with a narrow area of shrubs and trees just to the north between Dinsmore Drive and 12<sup>th</sup> Street, and grassy swales with scattered Monterey cypress between the southern arc of 12<sup>th</sup> Street and Highway 101.

The Kenmar PSB is oriented generally northeast/southwest (Figure 3). The larger portion east of Highway 101 includes a steep slope with non-native eucalyptus at the extreme east end, with a parking lot immediately to the west. Continuing west, an inactive rail line runs through a series of mostly open areas of low herbaceous growth with scattered Monterey cypress. West of Highway 101 and associated ramps is an ephemeral ditch which has developed a riparian-like area dominated by dense shrub and sapling cover, and which includes a few redwoods of moderate size near the intersection of Riverwalk Drive and the Highway 101 ramps. The southwest limit of the PSB coincides with the top of a grade dropping down to the adjacent Eel River floodplain, which is not included in the PSB.

Wetland and riparian habitats are discussed in further detail below.

# 2. Methods

### 2.1 Research Methods

The initial analysis consisted of review of existing environmental literature and data results from database queries of potential on-site sensitive species which were evaluated using the Fortuna United States Geoligical Survey (USGS) 7.5 quadrangle. The database queries include the California Natural Diversity Database (CNDDB) [CDFW February 2016]; the California Native Plant Society's (CNPS) Inventory of Rare and Endangered Vascular Plants [CNPS February 2016]; and lists of special-status species and natural communities that may occur in the project area as provided by the U.S. Fish and Wildlife Service (USFWS) [USFWS, 2016].

Additional existing data was reviewed when available, such as soil and ecological maps and descriptions generated by the Natural Resources Conservation Service (NRCS) and wetlands mapping from USFWS National Wetlands Inventory (NWI) [USFWS 1987]. NWI maps are compiled using a variety of remote sensing data sources, including aerial photographs, infrared photography, and soils data. NWI maps do not necessarily represent an accurate extent of jurisdictional wetlands in the Study Area. Finally, the CalFlora database in conjunction with the Jepson Herbarium database was consulted for site specific species cross referencing for potential rare plants in the project vicinity. When available, Geographic Information System (GIS) data was overlaid with the PSB.

## 2.2 Environmental Reconnaissance Survey Methods

On February 20, 2016, GHD field staff performed a reconnaissance level investigation of environmental and biological resources within the two PSB's. The survey was meant to identify the potential for environmental impacts and to identify potential permits that would result from implementing the project. This field reconnaissance effort, focused on identifying the potential presence of wetland, riparian, and special-status plant species (listed as rare, threatened, endangered, or candidate for rare, threatened, or endangered species listing under the state or federal Endangered Species Acts, CNPS rare plant ranking, or of local importance) or habitats present within the proposed project trail segments. The project area topographic maps, aerial photography maps, the California Department of Fish and Wildlife CNDDB and CNPS Rare Plant Inventory were consulted using the Fortuna quadrangle prior to and during the survey to determine potential sensitive species or habitat occurrence.

Field work was conducted by walking each of the proposed PSB units and visually documenting findings through photographs and notes. Each location with a potential wetland or areas potentially containing special status species and/or habitats, was noted. These areas would then be recommended for further investigations or protocol level surveys in order to fulfill potential permit requirements as described in further detail in Section 3 of this report.

The likelihood of certain permits increases in locations in which the project intersects certain features. For instance, the likelihood of a USACE Clean Water Act 404 and CDFW 1600 permit increases in locations in which the project crosses a blue line stream. Section 4 considers each permit, discusses the nature of the permit, and identifies the threshold triggers for each permit.

## 3.1 Special Status Plants, Animals, & Habitats Literature Results

A compilation of flora and fauna obtained from the literature search can be found in Table 1 below. The combined list identifies six animal species and three plant species with a moderate or high potential to be present in the PSB. A list of federal endangered, threatened and candidate species for the Fortuna USGS quadrangle was downloaded from the web site of the USFWS Arcata Field Office on March 4, 2016 (Appendix A). The USFWS lists are often of a general nature and do not indicate presence, merely the need for further review. The CNDDB Occurrence Report Rare Find 4 lists species potentially present in the project vicinity, and includes the Fortuna quadrangle (Appendix B). Several of these were subsequently excluded because of an absence of suitable habitat.

Scientific Name	Common Name	Status	Habitat	Potential to Occur
Antrozous pallidus	Pallid bat	SSC	Dry rocky woodlands	Low, no suitable habitat
Arborimus pomo	Sonoma tree vole	SSC	Conifer forest	Low, no large stands of suitable habitat
Pekania (Martes) pennanti	Fisher	FC	Mature forest	None; no suitable habitat present
Ardea herodius	Great Blue Heron	None	Colonial nester, tall trees, marshes	Low, several miles to nearest known rookeries
Charadrius alexandrinus nivosus	Western Snowy Plover	FT	Beaches and dunes above high tide line, river gravel bars	None; no suitable habitat present
Coccyzus americanus	Yellow-billed Cuckoo	FT	Dense extensive riparian forest	Low; nearest documented recent records near Cock Robin Island
Brachyramphus marmorata	Marbled Murrelet	FT	Old-growth redwood and Douglas fir forest	None; no suitable habitat present
Riparia riparia	Bank Swallow	ST	Nests in vertical banks/cliffs along rivers	Low for nesting; known from the Eel near Fernbridge so nearby foraging

#### Table 1. Listed/Proposed Rare, Threatened and Endangered Species

				is possible
Strix occidentalis caurina	Northern Spotted Owl	FT	Mature forest	None; no suitable habitat present
Emys (Actinymys) marmorata	Western pond turtle	SSC	Ponds, rivers, marshes	Moderate
Rana aurora	Northern Red- legged Frog	SSC	Emergent wetlands and stream margins, and nearby wet meadows and woods	High especially in riparian areas
Rana boylii	Foothill Yellow- legged Frog	SSC, federal proposed	Margins of shallow rocky streams and riffles	High; known to occur in the Eel and tributaries
Oncorhynchus kisutch	S. OR/N. CA Coho Salmon	FT	Rivers and tributaries	Moderate; historic records from Strong's Creek
Oncorhynchus mykiss	N. CA Steelhead	FT	Rivers and tributaries	High; recent records from the lower Strong's Creek watershed
Oncorhynchus tshawytscha	CA Coastal Chinook	FT	Rivers and larger tributaries	Moderate; present in Eel near Fortuna
Spirinchus thalyichthys	Longfin Smelt	FC, ST	Estuaries, may enter freshwater to spawn	Low; present in lower 4.5 miles of Eel, historic (1956) seasonal occurrence up to Van Duzen mouth

Important habitat features include Strong's Creek and an associated riparian corridor in the south and southwest portion of the 12<sup>th</sup> Street PSB, and several large individual redwoods in the western part of the Kenmar PSB. While these habitat features are not extensive, they could harbor sensitive animals or plants and have habitat and aesthetic value.

A number of plant species identified as rare by the CNPS occur in the Fortuna quadrangle; CEQA requires that these species be considered in the planning process, thus a protocol level study is recommended during the appropriate bloom period (Table 2). Appendix B contains the CNDDB occurance report. If rare species are located mitigation measures may be required. At least one of these species (Siskiyou checkerbloom) sometimes grows within maintained road right-of-way.

## Table 2. Potential Rare Plant Occurrence and Bloom Periods

Scientific Name	Common Name	Rare Plant Rank	Bloom Time	Habitat	Liklihood to Occur
Fissidens pauperculus	Minute pocket moss	1B.2	n/a	Damp soil in dry stream beds and banks	Moderate
Sidalcia malviflora ssp. patula	Siskiyou checkerbloom	1B.2	May- August	Coastal scrub, coastal prairie, road cuts	Moderate
Clarkia amoena ssp. whitneyi	Whitney's farewell-to- spring	1B.1	June- August	Coastal bluff, coastal scrub	Moderate, based on a 1955 record from "west of Fortuna."
<i>Gilia capitata</i> ssp. <i>pacifica</i>	Pacific gilia	1B.2	April- August	Coastal scrub, coastal prairie	Low

# 4. Environmental Permits and Processes Discussion

### 4.1 California Environmental Quality Act

Review under the California Environmental Quality Act (CEQA) is required whenever a state or local government entity initiates a project, funds a project, or issues a permit decision. The CEQA document is prepared or overseen by a designated lead agency. An Initial Study determines the appropriate level of environmental review; for a project such as this one limited to relatively small portions of an urban fringe area but including a salmonid stream and associated riparian areas, there is a possibility that an Environmental Impact Report (EIR) would be required. However, if all identified impacts can be avoided or adequately mitigated, a Mitigated Negative Declaration (MND) may be adequate. The City of Fortuna would most likely be the CEQA lead agency for the project. Other likely agencies include the Humboldt County Association of Governments, Caltransor other non-federal agencies with permitting authority over the project.

Compliance with the National Environmental Policy Act (NEPA) is required whenever there is federal involvement in the project. If the ultimate project includes federal funding, it would trigger NEPA analysis; in addition, federal involvement may also include approval or issuance of permits. If the project does not qualify for a Categorical Exclusion (CE) or Programmatic Categorical Exclusion (PCE), additional environmental documentation under NEPA may be necessary prior to project approval of funding by a federal agency. Caltrans would most likely be the NEPA lead agency for the project.

### 4.2 Other CEQA/NEPA Considerations:

From a CEQA/NEPA perspective, project segmentation may occur when the project as described and analyzed in a single CEQA or NEPA process does not encompass the entire project. Segmentation can occur when portions of a project that are dependent on other portions of the project to make them functional are evaluated in separate documents. An example would be if each interchange were analyzed in separate CEQA documents but then constructed simultaneously. In this example, the "entire project" would consist of both interchanges, even though the project was analyzed in two separate documents and therefore "segmented." However, if the components could not function without the other, then these projects must be analyzed in the same document. Alternatively, if the projects are analyzed in separate documents, they must be analyzed in the cumulative impacts section of the document. Therefore, if the two interchanges are considered a single project, then the document should address all project components.

If a project has reasonably foreseeable additional components, they must be analyzed concurrently as part of a single project. The flaw of segmentation is that it can divide larger projects into smaller components, which, when viewed independently, may not lead to the identification of the full range and intensity of impacts resulting from the entire project when viewed as a whole. Linear infrastructure network projects (e.g. transmission lines, pipe networks, roads, trails) may present a special challenge when considering whether a project is in danger of being segmented, as there may be no clear cut method of determining where an individual project starts and ends - and

whether it should be analyzed as part of a larger project or as an individual action simply occurring on a larger network. Following court decisions, the standard for determining whether a road project is an individual action warranting individual CEQA/NEPA analysis is if it is: of substantial length; and is between logical termini, such as population centers or major crossroads, etc; and has independent utility.

#### 4.2.1 Cultural Resources

Preparation of CEQA/NEPA documents would trigger a need for cultural resources studies in at least some portions of the PSB. Reconnaissance level studies and inclusion of reasonable mitigation measures would likely be suitable for most areas, unless those studies identify concentrations of cultural resources.

#### 4.2.2 Other Special Studies for CEQA/NEPA

CEQA and NEPA require special studies for key resources that may be impacted by the project. For instance, the Protocol level surveys for special-status plants and animals would serve as special studies. Other special studies that could be required include aesthetic studies, air quality studies, geologic studies, hazardous materials studies, noise studies, and traffic studies. At this time, it is unknown if any of these studies would be required. However, it is possible that special studies could be required for parts of the project. For example, geotechnical surveys may be required in the creek crossing locations.

## 4.3 Permits

#### 4.3.1 U.S. Army Corps of Engineers (USACE) Section 404 Nationwide Permit

The USACE regulates discharges of dredged or fill material into Waters of the United States under Section 404 of the Clean Water Act (CWA). The project may result in unavoidable fill of some jurisdictional wetlands or Waters of the U.S. during project implementation. There are also potential stream crossings, although the project will likely be designed to avoid or minimize impacts to wetlands or waters of the U.S. However, if filling of wetlands or waters of the U.S. are unavoidable, the project will require a USACE Section 404 Permit. The project may qualify for a streamlined USACE Nationwide Permit. Prior to authorizing wetland fill under Section 404, a wetland delineation must be submitted and verified by the USACE. Impacts that cause a loss of jurisdictional wetland will require an approved wetland mitigation and monitoring plan (MMP), accompanied by an adaptive management plan and long term maintenance plan.

A formal wetland delineation is recommended during the planning phase of any segment which crosses a potential wetland identified in this report, and for those areas where ditches (potential Waters of the U.S.) occur adjacent to the roads, in order to verify potential wetlands or Waters of the U.S. and to request a jurisdictional determination. Wherever ground disturbing work would occur below the ordinary high water mark (OHWM) of a stream crossing, a delineation and 404 permit would also be required. Potential wetlands and waters of the U.S. are shown on Figures 2a and 2b and 3, and include Strong's Creek, several drainage ditches, and a few small degraded wet depressions and swales.

### 4.3.2 Regional Water Quality Control Board (RWQCB)

Section 401 Water Quality Certification and National Pollutant Discharge Elimination System (NPDES) Requirements: Pursuant to section 401 of the federal CWA, projects that require a

USACE permit for discharge of dredge or fill material must obtain water quality certification to confirm compliance with state water quality requirements. If the project results in unavoidable fill of wetlands or Waters of the U.S., Section 401 Certification from the RWQCB will be required. The RWQCB may encourage a CRAM evaluation of impacted habitats and mitigation for compensation of impacts.

The CWA requires that any discharge of pollutants to waters of the United States from any point source is unlawful unless the discharge complies with a NPDES permit. These regulations require that discharges of stormwater from construction projects that cause one or more acres of soil disturbance must be in compliance with an NPDES permit. If the project disturbs more than one acre of soil, it must comply with the construction general stormwater permit issued by the State Water Resource Control Board. The construction general permit requires the development and implementation of a Storm Water Pollution Prevention Plan (SWPPP).

Additionally, the RWQCB may take jurisdiction on a variety of drainage ditches and swales identified in the PSB and a formal delineation of the features will be required throughout the PSB.

#### 4.3.3 California Department of Fish & Wildlife Section 1602

Under Fish and Game Code Section 1602 (Streambed Alteration), the CDFW has jurisdiction over proposed activities that may substantially modify a river, stream, or lake. The PSB includes portions of Strong's Creek and several shallow ditches, and depending on final design direct or indirect impacts could occur in some of these locations. Additionally, CDFW jurisdiction extends at least to the top of bank and may sometimes include adjacent riparian zones. As a result, a 1600 Lake and Streambed Alteration Agreement including special conditions to avoid or minimize impacts is anticipated.

# 4.3.4 Federal Endangered Species Act Compliance (Protocol Level Surveys and Biological Assessments)

Based on available knowledge at this time, the project is not expected to result in any adverse impacts to federally threatened or endangered species or habitats, and GHD does not anticipate the need for formal Section 7 ESA consultation (this assumes no instream work). However, when a USACE permit is required for impacts to jurisdictional wetlands or other waters and the project has the potential to cause adverse impacts to federally-listed threatened or endangered species, the USACE must initiate consultation with the USFWS and/or the National Marine Fisheries Service (NMFS) pursuant to Section 7 of the ESA. Although unlikely for the proposed project, because no impacts to threatened, or endangered species are currently anticipated, if future studies determine that a listed species is present or if a species is added to the list and is present in the area, and if adverse effects are possible, then informal or formal consultation, including preparation of a Biological Assessment, may be required.

Potential issues include salmonids (steelhead, coho, chinook) which occur in the Eel River and tributaries including Strong's Creek. If project activities require dewatering of any portion of the creek, of if there is a possibility of sediment input to the stream or any other potential instream impact, then Section 7 consultation including preparation of a Biological Assessment may be necessary.

There is no documentation of terrestrial listed species in the project study boundary; however, if they are found to occur near the PSB, a variety of requirements ranging from pre-construction protocol surveys to seasonal noise and visual buffers during construction would be triggered, depending on distance to the nest.

# 4.3.5 California Endangered Species Act (Protocol Level Surveys and Biological Assessments):

The California Endangered Species Act (CESA) requires consultation with the CDFW when preparing CEQA documents to ensure that the lead agency actions do not jeopardize the existence of listed species.

A number of state listed or state sensitive species could potentially occur close to the PSB including bank swallow, northern red-legged frog, foothill yellow-legged frog, western pond turtle, and others. However no site-specific surveys are available at this time.

By incorporating the development of reasonable avoidance or mitigation measures in the CEQA document, such as seasonal work windows and buffer zones around bird and bat habitats and native migratory bird nests during the nesting season and pre-construction surveys for other species impacts can likely be reduced to less than significant. However, a thorough review is recommended, especially where wetland, stream, drainage ditches, or riparian impacts may occur.

### 4.3.6 Migratory Bird Treaty Act (Avian Surveys)

The Migratory Bird Treaty Act (MBTA) protects all native species of birds. USFWS has statutory authority to enforce the MBTA. To avoid impacts to nesting birds it is recommended that to the extent practical, construction activity occur outside the nesting season (approximately March 15 to August 15 in Humboldt County). This will be most crucial near riparian areas and large trees. If it is not possible to avoid the nesting season then avian surveys should occur within seven days prior to disturbance, and if active nests are identified then the biologist shall establish appropriate buffers. For common species typical of urban sites these are often very small, although buffers for raptors or special-status birds can be much larger (100 to 500 feet). Additonal protections for birds or requirements for avoidance are found in the Fish and Game Code and are often a part of CEQA compliance and mitigation measures.

### 4.3.7 California Department of Transportation (Caltrans)

Encroachment Permits (EP) and/or other agreements may be required for use of or alterations to any area within a Caltrans right-of-way.

A Humboldt County EP will be required if any work encroaches into County right-of-way. Additionally, a Humboldt County grading permit will need to be obtained for grading work in the County right-of-way which exceeds the thresholds identified in the County Grading Ordinance.

### 4.3.8 California State Lands Commission

The State Lands Commission (SLC) has jurisdiction over sovereign public lands, including the beds of California's naturally navigable rivers, lakes and streams, as well as the state's tide and submerged lands along the state's more than 1,100 miles of coastline, extending from the shoreline to three miles offshore. The location and extent of sovereign lands are generally defined by reference to the ordinary high and low water marks of tidal and navigable waterways. Because the boundaries of these lands are often legally based upon the last natural extent and location of the subject water body, they are not necessarily apparent from a present day site inspection, and substantial research is needed to define the extent of the state's ownership interests. Because the project crosses tributaries associated with the Eel River, further inquiry regarding the extent of SLC's jurisdiction should be conducted.

#### 4.3.9 Permit Summary

In summary, a variety of permits and related environmental review would be necessary for project planning and design. In general, agencies are more supportive of projects when they are a part of the early planning and collaboration process. Currently, the proposed project would occur mostly within already disturbed areas, and environmental impacts are most likely if design features cross wetland or riparian areas. Any work within the identified creek crossings or wetlands would also trigger various permit requirements. The present document is intended to identify potential permits and environmental planning considerations at a project-wide scale.

# 5. Conclusions

### 5.1 **Potential Permits and Environmental Constraints**

The project area is shown on Figures 1 through 3. Appendix C contains representative photographs of the different habitats or constraints observed during the field reconnaissance effort.

The project will require a formal wetland delineation following USACE protocol to identify impacts to wetland habitat or waters of the U.S.; particularly in the areas identified as potential wetland, ditch, and stream crossings. Parts of the PSB contains what appear to be drainage ditches that could fall under either the USACE and/or RWQCB jurisdiction. The types of ditches identified in Figures 2a and 2b and 3 and shown in photographs in Appendix C include drainage ditches with evident flow paths connected by culverts, drainages comprised of hydrophytic vegetation, and swales.

Potential biological surveys required for implementing this proposed project include, at a minimum, a protocol level intensive botanical site inventory of vascular plant species, with emphasis on species identified in the database queries. This survey will need to be conducted at the appropriate season(s) to locate flowering individuals of listed species.

A few state special concern wildlife species have been reported within the general project vicinity, and others could occur although no recent field data is available for the PSB. Federally listed salmonids have been reported in other parts of Strong's Creek in the past and are presumed to be present. The PSB also contains several large redwoods and other large trees and other viable habitat for migratory nesting birds as well as riparian habitat. Therefore, these areas may need to be further assessed with CEQA special studies in order to identify and offset adverse impacts to the potential fauna along these routes. Additional non-biological studies may be required by CEQA/NEPA.

# 6. References

Baldwin et al. The Jepson Manual Vascular Plants of California, Second Edition. 2012. University of California Press. Berkeley, CA.

California Department of Fish and Wildlife 2013. *California Natural Diversity Database (CNDDB)*. Fortuna USGS 7.5 Minute Quadrangle. California Department of Fish and Wildlife CDFW). Sacramento, California. Accessed March 4, 2016: https://nrmsecure.dfg.ca.gov/cnddb/view/query.aspx, February 22, 2013 (expires: August 5, 2013).

California Department of Fish and Game. May 2000. *Guidelines for Assessing the Effects of Proposed Development on Rare, Threatened, and Endangered Plants and Plant Communities.* Sacramento, CA.

California Native Plant Society (CNPS). 2016. Inventory of Rare and Endangered Plants (online edition, v8-01a). California Native Plant Society. Sacramento, CA. Accessed onMarch 4, 2016.

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USFWS, 1987. *National Wetland Inventory (NWI)*. U.S. Fish and Wildlife Service (USFWS). Portland, OR. Accessed: <u>http://www.fws.gov/wetlands</u>.

#### Prepared By;

Ken Mierzwa, Senior Environmental Scientist, GHD, Eureka, CA

#### **Reviewed BY:**

Josh Wolf, Project Manager, GHD, Eureka, CA Misha Schwarz, Senior Environmental Scientist, GHD, Eureka, CA



# Appendix A (USFWS Listed/Proposed Threatened and Endangered Species for the Fortuna Quad)

Candidate species included

U.S. Fish & Wildlife Service

# **12th Street Interchange**

# IPaC Trust Resources Report

Generated March 04, 2016 03:19 PM MST, IPaC v3.0.0

This report is for informational purposes only and should not be used for planning or analyzing project level impacts. For project reviews that require U.S. Fish & Wildlife Service review or concurrence, please return to the IPaC website and request an official species list from the Regulatory Documents page.



IPaC - Information for Planning and Conservation (<u>http://ecos.fws.gov/ipac/</u>): A project planning tool to help streamline the U.S. Fish & Wildlife Service environmental review process.

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Project Description	<u>1</u>
Endangered Species	<u>2</u>
Migratory Birds	<u>4</u>
Refuges & Hatcheries	<u>7</u>
Wetlands	<u>8</u>

# U.S. Fish & Wildlife Service IPaC Trust Resources Report



NAME

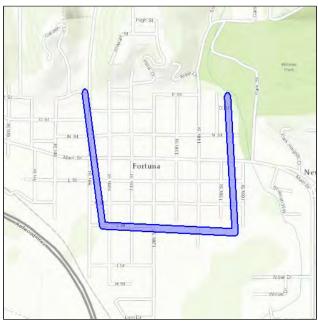
12th Street Interchange

LOCATION

Humboldt County, California

IPAC LINK

http://ecos.fws.gov/ipac/project/ 3IDEK-YDXKJ-BBLBW-TO465-QKA2Y4



# U.S. Fish & Wildlife Service Contact Information

Trust resources in this location are managed by:

Arcata Fish And Wildlife Office

1655 Heindon Road Arcata, CA 95521-4573 (707) 822-7201

# **Endangered Species**

Proposed, candidate, threatened, and endangered species are managed by the <u>Endangered Species Program</u> of the U.S. Fish & Wildlife Service.

### This USFWS trust resource report is for informational purposes only and should not be used for planning or analyzing project level impacts.

For project evaluations that require USFWS concurrence/review, please return to the IPaC website and request an official species list from the Regulatory Documents section.

<u>Section 7</u> of the Endangered Species Act **requires** Federal agencies to "request of the Secretary information whether any species which is listed or proposed to be listed may be present in the area of such proposed action" for any project that is conducted, permitted, funded, or licensed by any Federal agency.

## A letter from the local office and a species list which fulfills this requirement can only be obtained by requesting an official species list either from the Regulatory Documents section in IPaC or from the local field office directly.

The list of species below are those that may occur or could potentially be affected by activities in this location:

## **Birds**

Marbled Murrelet Brachyramphus marmoratus	Threatened
CRITICAL HABITAT There is <b>final</b> critical habitat designated for this species.	
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B08C	
Northern Spotted Owl Strix occidentalis caurina	Threatened
CRITICAL HABITAT	
There is final critical habitat designated for this species.	
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B08B	
Western Snowy Plover Charadrius alexandrinus nivosus	Threatened
CRITICAL HABITAT	
There is final critical habitat designated for this species.	
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B07C	
Yellow-billed Cuckoo Coccyzus americanus	Threatened
CRITICAL HABITAT	
There is <b>proposed</b> critical habitat designated for this species.	
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B06R	

# **Flowering Plants**

Beach Layia Layia carnosa	Endangered
CRITICAL HABITAT	
No critical habitat has been designated for this species.	
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=Q34T	
Menzies' Wallflower Erysimum menziesii	Endangered
CRITICAL HABITAT	
No critical habitat has been designated for this species.	
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=Q29W	
Western Lily Lilium occidentale	Endangered
CRITICAL HABITAT	
No critical habitat has been designated for this species.	
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=Q1Y0	
Mammals	
Fisher Martes pennanti	Proposed Threatened

CRITICAL HABITAT **No critical habitat** has been designated for this species. https://ecos.fws.gov/tess\_public/profile/speciesProfile.action?spcode=A0HS

# **Critical Habitats**

This location overlaps all or part of the critical habitat for the following species:

### Steelhead Critical Habitat Final designated

https://ecos.fws.gov/tess\_public/profile/speciesProfile.action?spcode=E08D#crithab

# **Migratory Birds**

Birds are protected by the <u>Migratory Bird Treaty Act</u> and the <u>Bald and Golden Eagle</u> <u>Protection Act</u>.

Any activity that results in the take of migratory birds or eagles is prohibited unless authorized by the U.S. Fish & Wildlife Service.<sup>[1]</sup> There are no provisions for allowing the take of migratory birds that are unintentionally killed or injured.

Any person or organization who plans or conducts activities that may result in the take of migratory birds is responsible for complying with the appropriate regulations and implementing appropriate conservation measures.

1. 50 C.F.R. Sec. 10.12 and 16 U.S.C. Sec. 668(a)

Additional information can be found using the following links:

- Birds of Conservation Concern <u>http://www.fws.gov/birds/management/managed-species/</u> <u>birds-of-conservation-concern.php</u>
- Conservation measures for birds <u>http://www.fws.gov/birds/management/project-assessment-tools-and-guidance/</u> <u>conservation-measures.php</u>
- Year-round bird occurrence data <u>http://www.fws.gov/birds/management/project-assessment-tools-and-guidance/</u> <u>akn-histogram-tools.php</u>

The following species of migratory birds could potentially be affected by activities in this location:

Allen's Hummingbird Selasphorus sasin	Bird of conservation concern
Season: Breeding	
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0LI	
Bald Eagle Haliaeetus leucocephalus	Bird of conservation concern
Year-round	
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B008	
Burrowing Owl Athene cunicularia	Bird of conservation concern
Year-round	
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0NC	
Calliope Hummingbird Stellula calliope	Bird of conservation concern
Season: Breeding	
https://ecos.fws.gov/tess.public/profile/speciesProfile.action?spcode=B0K3	

Fox Sparrow Passerella iliaca Season: Wintering	Bird of conservation concern
Lewis's Woodpecker Melanerpes lewis Season: Wintering	Bird of conservation concern
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0HQ	
Long-billed Curlew Numenius americanus Season: Wintering	Bird of conservation concern
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B06S	
Marbled Godwit Limosa fedoa Season: Wintering	Bird of conservation concern
https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0JL	
Olive-sided Flycatcher Contopus cooperi Season: Breeding https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0AN	Bird of conservation concern
Peregrine Falcon Falco peregrinus Year-round https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0FU	Bird of conservation concern
Purple Finch Carpodacus purpureus Year-round	Bird of conservation concern
Short-billed Dowitcher Limnodromus griseus Season: Wintering https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0JK	Bird of conservation concern
Short-eared Owl Asio flammeus Season: Wintering https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0HD	Bird of conservation concern
Snowy Plover Charadrius alexandrinus Season: Breeding	Bird of conservation concern
Western Grebe aechmophorus occidentalis Season: Wintering https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0EA	Bird of conservation concern
Whimbrel Numenius phaeopus Season: Wintering https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0JN	Bird of conservation concern
Willow Flycatcher Empidonax traillii Season: Breeding https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0F6	Bird of conservation concern
Yellow Warbler dendroica petechia ssp. brewsteri Season: Breeding https://ecos.fws.gov/tess_public/profile/speciesProfile.action?spcode=B0EN	Bird of conservation concern

Red Knot Calidris canutus ssp. roselaari

Season: Wintering https://ecos.fws.gov/tess\_public/profile/speciesProfile.action?spcode=B0G6 Bird of conservation concern

# Wildlife refuges and fish hatcheries

There are no refuges or fish hatcheries in this location

# Wetlands in the National Wetlands Inventory

Impacts to <u>NWI wetlands</u> and other aquatic habitats may be subject to regulation under Section 404 of the Clean Water Act, or other State/Federal statutes.

### For more information please contact the Regulatory Program of the local <u>U.S. Army</u> <u>Corps of Engineers District</u>.

#### DATA LIMITATIONS

The Service's objective of mapping wetlands and deepwater habitats is to produce reconnaissance level information on the location, type and size of these resources. The maps are prepared from the analysis of high altitude imagery. Wetlands are identified based on vegetation, visible hydrology and geography. A margin of error is inherent in the use of imagery; thus, detailed on-the-ground inspection of any particular site may result in revision of the wetland boundaries or classification established through image analysis.

The accuracy of image interpretation depends on the quality of the imagery, the experience of the image analysts, the amount and quality of the collateral data and the amount of ground truth verification work conducted. Metadata should be consulted to determine the date of the source imagery used and any mapping problems.

Wetlands or other mapped features may have changed since the date of the imagery or field work. There may be occasional differences in polygon boundaries or classifications between the information depicted on the map and the actual conditions on site.

#### DATA EXCLUSIONS

Certain wetland habitats are excluded from the National mapping program because of the limitations of aerial imagery as the primary data source used to detect wetlands. These habitats include seagrasses or submerged aquatic vegetation that are found in the intertidal and subtidal zones of estuaries and nearshore coastal waters. Some deepwater reef communities (coral or tuberficid worm reefs) have also been excluded from the inventory. These habitats, because of their depth, go undetected by aerial imagery.

#### DATA PRECAUTIONS

Federal, state, and local regulatory agencies with jurisdiction over wetlands may define and describe wetlands in a different manner than that used in this inventory. There is no attempt, in either the design or products of this inventory, to define the limits of proprietary jurisdiction of any Federal, state, or local government or to establish the geographical scope of the regulatory programs of government agencies. Persons intending to engage in activities involving modifications within or adjacent to wetland areas should seek the advice of appropriate federal, state, or local agencies concerning specified agency regulatory programs and proprietary jurisdictions that may affect such activities.

### There are no wetlands in this location

Appendix B (CNDDB Occurrence Report)

Fortuna Quad





#### Query Criteria: Quad is (Fortuna (4012452))

Species	Element Code	Federal Status	State Status	Global Rank	State Rank	Rare Plant Rank/CDFW SSC or FP
Agelaius tricolor	ABPBXB0020	None	None	G2G3	S1S2	SSC
tricolored blackbird						
Antrozous pallidus	AMACC10010	None	None	G5	S3	SSC
pallid bat						
Arborimus pomo	AMAFF23030	None	None	G3	S3	SSC
Sonoma tree vole						
Ardea herodias	ABNGA04010	None	None	G5	S4	
great blue heron						
Bombus caliginosus	IIHYM24380	None	None	G4?	S1S2	
obscure bumble bee						
Bombus occidentalis	IIHYM24250	None	None	G2G3	S1	
western bumble bee						
Clarkia amoena ssp. whitneyi	PDONA05025	None	None	G5T1	S1	1B.1
Whitney's farewell-to-spring						
Emys marmorata	ARAAD02030	None	None	G3G4	S3	SSC
western pond turtle						
Fissidens pauperculus	NBMUS2W0U0	None	None	G3?	S2	1B.2
minute pocket moss						
Gilia capitata ssp. pacifica	PDPLM040B6	None	None	G5T3T4	S2	1B.2
Pacific gilia						
Lasiurus cinereus	AMACC05030	None	None	G5	S4	
hoary bat						
Montia howellii	PDPOR05070	None	None	G3G4	S3	2B.2
Howell's montia						
Oncorhynchus clarkii clarkii coast cutthroat trout	AFCHA0208A	None	None	G4T4	S3	SSC
Polemonium carneum	PDPLM0E050	None	None	G3G4	S2	2B.2
Oregon polemonium						
Rana aurora	AAABH01021	None	None	G4	S3	SSC
northern red-legged frog						
Rana boylii	AAABH01050	None	None	G3	S3	SSC
foothill yellow-legged frog						
Riparia riparia	ABPAU08010	None	Threatened	G5	S2	
bank swallow						
Sidalcea malviflora ssp. patula	PDMAL110F9	None	None	G5T2	S2	1B.2
Siskiyou checkerbloom						
Spirinchus thaleichthys	AFCHB03010	Candidate	Threatened	G5	S1	SSC
longfin smelt						

**Record Count: 19** 

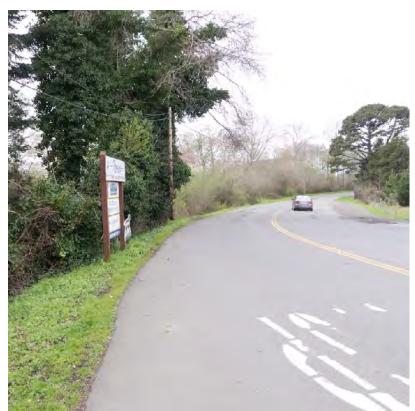
Appendix C (Site Photographs)



Strong's Creek and associated riparian area



Riverwalk Drive bridge, looking east toward Hwy 101



Dinsmore Drive north of bridge, with riparian edge on left



Dinsmore Drive north of Riverwalk Drive, looking north. Riparian on left, willow and Monterey cypress on right



Southbound Hwy 101 exit ramp at 12<sup>th</sup> Street, looking NE



Potential wetland swale within area shown in photo above, looking NW with  $12^{\text{th}}$  Street in background



Eucalyptus on slope east of Kenmar Rd. interchange and parking lot



Parking lot, looking west toward Hwy 101 with inactive rail line in middle ground



Potential wetland south of Kenmar, between rail line and Hwy 101 ramp, looking south



Ditch/potential wetland north of Kenmar and east of Hwy 101, looking north



Ephemeral ditch north of Kenmar and west of Hwy 101, with adjacent riparian area



Degraded riparian habitat north of Kenmar/Riverwalk and west of Hwy 101



Redwood west of Hwy 101 and north of Kenmar/Riverwalk



Ephemeral ditch flowing toward Eel River floodplain, south of Kenmar/Riverwalk and west of Hwy 101. Note redwood in top right.

# www.ghd.com



#### PROJECT

#### PRELIMINARY COST ESTIMATE®

EA: 01-0K300 PID: 120000056

EA: 01-0K300

PID: 120000056

District-County-Route: 01-HUM-101 PM: 59.2/59.8

Type of Estimate : Project Report

Program Code : Local

Project Limits : Kenmar Road Undercrossing

Project Description: Improve traffic operations at the US101 Kenmar Road Interchange Scope : Construct two single-lane roundabouts and reconstruct ramps

Alternative : Alternative No. 1

#### SUMMARY OF PROJECT COST ESTIMATE

Current Year Cost		E	scalated Cost	
TOTAL ROADWAY COST	\$	13,047,100	\$	16,027,013
TOTAL STRUCTURES COST	\$	2,686,063	\$	3,299,550
SUBTOTAL CONSTRUCTION COST	\$	15,733,163	\$	19,326,563
TOTAL RIGHT OF WAY COST	\$	450,000	\$	500,000
TOTAL CAPITAL OUTLAY COSTS	\$	16,184,000	\$	19,827,000
PA/ED SUPPORT	\$	550,000	\$	550,000
PS&E SUPPORT	\$	2,000,000	\$	2,000,000
RIGHT OF WAY SUPPORT	\$	100,000	\$	100,000
CONSTRUCTION SUPPORT	\$	2,500,000	\$	2,500,000
TOTAL SUPPORT COST	\$	5,150,000	\$	5,150,000
TOTAL PROJECT COST	\$	21,350,000	\$	25,000,000

#### Programmed Amount

	Date of Estimate (Month/Year)	Month 12		<u>Year</u> 22	
	Estimated Construction Start (Month/Year)	7	1	25	
		Number of Working Days	=	180	
Estima	ated Mid-Point of Construction (Month/Year)	1	1	26	
	Estimated Construction End (Month/Year)	6	1	26	
	Number	of Plant Establishment Days		260	
	Estimated Project Schedule				
	PID Approval			2017 (Local PSR)	
	PA/ED Approval			TBD	
	PS&E			12/1/2025	
	RTL			1/1/2025	
	Begin Construction			7/1/2025	
Reviewed by District O.E. or Cost Estimate Certifier		xx/xx/xxxx		(xxx) xxx-xxxx	
_	Office Engineer / Cost Estimate Certifier	Date		Phone	
Approved by Project Manager		xx/xx/xxxx		(xxx) xxx-xxxx	
	Project Manager	Date		Phone	

EA: 01-0K300 PID: 120000056

## I. ROADWAY ITEMS SUMMARY

	Section			Cost
1	Earthwork		\$	1,457,000
2	Pavement Structural Section		\$	3,846,100
3	Drainage		\$	678,300
4	Specialty Items		\$	482,200
5	Environmental		\$	1,357,000
6	Traffic Items		\$	1,309,900
7	Detours		\$	100,000
8	Minor Items		\$	369,300
9	Roadway Mobilization		\$	960,000
10	Supplemental Work		\$	270,500
11	State Furnished		\$	152,200
12	Time-Related Overhead		\$	362,800
13	Roadway Contingency		\$	1,701,800
	TOTAL ROADWAY IT	EMS	\$	13,047,100
Estimate Prepared By :				530-953-6486
	Russ Wenham, Sr. Technical GHD Inc.	Jir.	Date	Phone
Estimate Reviewed By	:			707-267-2264
	Josh Wolf, Project Manager GHD Inc.		Date	Phone

By signing this estimate you are attesting that you have discussed your project with all functional units and have incorporated all their comments or have discussed with them why they will not be incorporated.

#### PROJECT COST ESTIMATE

1,457,000

TOTAL EARTHWORK SECTION ITEMS \$

#### SECTION 1: EARTHWORK

Item code		Unit	Quantity		Unit Price (\$)		Cost
190101	Roadway Excavation	CY	16,000	х	50.00	=	\$ 800,000
152320	Lead Compliance Plan	LS	1	х	4,000.00	=	\$ 4,000
198010	Imported Borrow	CY	9,200	х	65.00	=	\$ 598,000
16010X	Clearing & Grubbing	LS	1	х	40,000.00	=	\$ 40,000
170101	Develop Water Supply	LS	1	х	15,000.00	=	\$ 15,000
210130	Duff	ACRE	0	х	0.00	=	\$ -

#### SECTION 2: PAVEMENT STRUCTURAL SECTION

Item code		Unit	Quantity		Unit Price (\$)			Cost	
401050	Jointed Plain Concrete Pavement	CY	500	х	750.00	=	\$	375,000	
390132	Hot Mix Asphalt (Type A)	TON	12,800	х	130.00	=	\$	1,664,000	
198209A	Subgrade Enhancement Geotextile, Class TBD (B2 or B3)	SQYD	14,500	х	10.00	=	\$	145,000	
260203	Class 2 Aggregate Base	CY	14,300	х	85.00	=	\$	1,215,500	
397005	Tack Coat	TON	16	х	1,200.00	=	\$	19,200	
731521	Minor Concrete (Sidewalk)	CY	150	х	800.00	=	\$	120,000	
731502	Minor Concrete (Miscellaneous Construction)	CY	510	х	750.00	=	\$	382,500	
731504	Minor Concrete (Curb and Gutter)	CY	225	х	800.00	=	\$	180,000	
730020	Minor Concrete (Curb)	CY	95	х	750.00	=	\$	71,250	
39407X	Place Hot Mix Asphalt Dike (Type TBD)	LF	2,800	х	12.00	=	\$	33,600	
394090	Place Hot Mix Asphalt (Miscellaneous Area)	SQYD	100	х	75.00	=	\$	7,500	
153103	Cold Plane Asphalt Concrete Pavement	SQYD	500	х	15.00	=	\$	7,500	
			TOTAL PA	VEM	ENT STRUCTU	RAL	. SEC	CTION ITEMS \$	3,

#### EA: 01-0K300 PID: 120000056

### SECTION 3: DRAINAGE

Item code		Unit	Quantity		Unit Price (\$)			Cost	
710136	Remove Pipe	LF	800	х	20.00	=	\$	16,000	
710152	Remove Headwall	EA	4	х	500.00	=	\$	2,000	
710150	Remove Inlet	EA	1	х	500.00	=	\$	500	
152430	Adjust Inlet	EA	3	х	3,000.00	=	\$	9,000	
510502	Minor Concrete (Minor Structure)	CY	70	х	3,000.00	=	\$	210,000	
610108	18" Alternative Pipe Culvert	LF	1,720	х	190.00	=	\$	326,800	
610112	24" Alternative Pipe Culvert	LF	250	х	260.00	=	\$	65,000	
705311	18" Alternative Flared End Section	EA	7	х	1,000.00	=	\$	7,000	
721XXX	Rock Slope Protection (TBD, Method B)	CY	45	х	300.00	=	\$	13,500	
72901X	Rock Slope Protection Fabric (Class TBD)	SQYD	140	х	30.00	=	\$	4,200	
750001	Miscellaneous Iron and Steel	LB	8,100	х	3.00	=	\$	24,300	
					тот	AL	DRA	INAGE ITEMS	\$ 678

### SECTION 4: SPECIALTY ITEMS

Item code		Unit	Quantity		Unit Price (\$)			Cost	
141120	Treated Wood Waste	LB	13,000	х	1.00	=	\$	13,000	
839752	Remove Guardrail	LF	1,200	х	2.50	=	\$	3,000	
800302	Chain Link Fence (Type CL-4)	LF	700	х	100.00	=	\$	70,000	
80XXXX	Chain Link Fence (Abutment Security Fencing)	LS	1	х	30,000.00	=	\$	30,000	
832055	Midwest Guardrail System	LF	120	х	60.00	=	\$	7,200	
839584	Alternative In-line Terminal System	EA	2	х	3,500.00	=	\$	7,000	
83964X	Concrete Barrier (Type TBD)	LF	260	х	300.00	=	\$	78,000	
511035	Architectural Treatment	SQFT	5,598	х	40.00	=	\$	223,920	
XXXXXX	Remove Railroad Facilities	LS	1	х	50,000.00	=	\$	50,000	
					тот	AL	SPEC		\$ 482

#### SECTION 5: ENVIRONMENTAL

5A - ENVIRONMENTAL MITIGATION									
Item code	Unit	Quantity		Unit Price (\$)			Cost		
XXXXXX Biological Mitigation	LS	1	х	40,000.00	=	\$	40,000		
				Subtotal	Envi	ronn	nental Mitigation	\$	40,000
5B - LANDSCAPE AND IRRIGATION							-		
Item code	Unit	Quantity		Unit Price (\$)			Cost		
20XXXX Landscaping and Irrigation System	LS	1	х	1,000,000.00	=	\$	1,000,000		
		•	~			•	be and Irrigation	\$	1,000,000
5C - EROSION CONTROL				Gubtotur	Lune	locup	o ana migatori	Ψ	1,000,000
Item code	Unit	Quantity		Unit Price (\$)			Cost		
210010 Move In/Move Out (Erosion Control)	EA	3	х	2000.00	=	\$	6.000		
210350 Fiber Rolls	LF	6,000	x	6.00	=	φ \$	36,000		
210430 Hydroseed	SQFT	85,000	x	0.20	=	ф \$	17,000		
211111 Permanent Erosion Control Establishment Work	LS	05,000	x	20000.00	=	ф \$	20.000		
		I	~		Cub	•	Erosion Control	¢	70.000
					Sub	lotai	Erosion Control	\$	79,000
5D - NPDES		• • • •					<b>a</b> .		
Item code	Unit	Quantity		Unit Price (\$)			Cost		
130300 Prepare SWPPP	LS	1	х	5,000.00	=	\$	5,000		
130100 Job Site Management	LS	1	х	40,000.00	=	\$	40,000		
130330 Storm Water Annual Report	EA	2	х	2,000.00	=	\$	4,000		
130310 Rain Event Action Plan (REAP)	EA	25	х	500.00	=	\$	12,500		
130320 Storm Water Sampling and Analysis Day	EA	30	х	700.00	=	\$	21,000		
130520 Temporary Hydraulic Mulch	SQYD	10,000	х	1.00	=	\$	10,000		
130505 Move-In/Move-Out (Temporary Erosion Control)	EA	12	х	1,000.00	=	\$	12,000		
130640 Temporary Fiber Roll	LF	4,000	х	10.00	=	\$	40,000		
130900 Temporary Concrete Washout	LS	1	х	10,000.00	=	\$	10,000		
130710 Temporary Construction Entrance	EA	4	х	5,000.00	=	\$	20,000		
130610 Temporary Check Dam	LF	300	х	20.00	=	\$	6,000		
130620 Temporary Drainage Inlet Protection	EA	30	х	250.00	=	\$	7,500		
130730 Street Sweeping	LS	1	х	50,000.00	=	\$	50,000		
						Su	btotal NPDES	\$	238.000
						50		Ψ	230,000
				тот	AL	ENV	RONMENTAL	\$	1,357,000
Supplemental Work for NPDES									

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ouppion		
066595	Water Pollution Control Maintenance Sharing*	LS
066596	Additional Water Pollution Control**	LS
066597	Storm Water Sampling and Analysis***	LS

x 10,000.00 = \$ 10,000 x 10,000.00 = \$ 10,000 x 5,000.00 = \$ 5,000 Subtotal Supplemental Work for NDPS \$

25,000

#### SECTION 6: TRAFFIC ITEMS

6A - Traffic Electrical									
Item code	Unit	Quantity		Unit Price (\$)			Cost		
770090 Lighting (City Street)	LS	1	х	350,000.00	=	\$	350,000		
77009X Lighting (Trail)	LS	1	х	25,000.00	=	\$	25,000		
870600 Traffic Monitoring Station System	LS	1	х	100,000.00	=	\$	100,000		
872001A Temporary Lighting Systems (Freeway)	LS	1	х	25,000.00	=	\$	25,000		
872131A Modify Lighting Systems (Freeway)	LS	1	х	40,000.00	=	\$	40,000		
				Su	ıbtot	al Tra	affic Electrical	\$	540,000
6B - Traffic Signing and Striping									
Item code	Unit	Quantity		Unit Price (\$)			Cost		
56601X Roadside Sign (Type TBD)	EA	80	х	300.00	=	\$	24,000		
810170 Delineator (Class 1)	EA	60	х	50.00	=	\$	3,000		
82013X Object Marker (Type TBD)	EA	12		50.00	=	\$	600		
8207XX Furnish Single Sheet Aluminum Sign (Thick TBD)	SQFT	1,000	х	30.00	=	\$	30.000		
8202XX Remove Roadside Sign	EA	30	x	100.00	=	\$	3,000		
820XXX Relocate Roadside Sign	EA	10	x	250.00	=	\$	2,500		
820860 Install Sign (Strap and Saddle Bracket Method)	EA	5	~	150.00		\$	750		
84XXXX Permanent Pavement Delineation (EWNV + RPMs	LS	1	х	180,000.00	=	\$	180,000		
				Subtotal Traff	fic S	ignin	g and Striping	\$	243,850
6C - Traffic Management Plan									
Item code	Unit	Quantity		Unit Price (\$)			Cost		
12865X Portable Changeable Message Signs	EA	5	х	\$ 10,000.00	=	\$	50,000		
129152 Temporary Radar Speed Feedback	EA	4	x	\$ 6,000.00	=		24,000		
				Subtotal Tra	affic	Man	agement Plan	\$	74,000
6C - Stage Construction and Traffic Handling									
Item code	Unit	Quantity		Unit Price (\$)			Cost		
01XXXX Alternative Temporary Crash Cushion	EA	5	х	5,000.00	=	\$	25,000		
120XXX Channelizing Devices (Various)	LS	1	x	10,000.00	=	\$	10,000		
120100 Traffic Control System	LS	1	x	350,000.00	=	\$	350,000		
120320 Temporary Barrier System	LF	700	x	60.00	=	\$	42,000		
1201XX Temporary Pavement Delineation	LS	1	x	25,000.00	=	\$	25,000		
		Subto	tal S	tage Constructio	on ai	nd Tr	affic Handling	\$	452,000
				т	ота	L TR	AFFIC ITEMS	\$	1,309,900
			L		-			Ŧ	,,

SECTION 7: DETOURS
Includes constructing, maintaining, and removal

Item code XXXXXX Temporary Road Wideni	ing & Removal	<i>Unit</i> LS		<b>Quantity</b> 1	x	<b>Unit Price (\$)</b> 100,000.00	=	\$	<b>Cost</b> 100,000		
						TOTAL	DE	TOU	RS	\$	100,000
					5	SUBTOTAL SE	СТІ	ONS	1 through 7	\$	9,230,500
SECTION 8: MINOR ITEMS	;										
8A - Americans with Disabilities	Act Items										
ADA Items 8B - Bike Path Items						1.0%		\$	92,305		
Bike Path Items (Include 8C - Other Minor Items	ed in items)					0.0%		\$	-		
Other Minor Items						3.0%		\$	276,915		
	Total of Section 1-7		\$	9,230,500	x	4.0%	=	\$	369,220		
						TOTAL		OR IT	EMS	\$	369,300
SECTIONS 9: ROADWAY	NOBILIZATION										
ltem code 999990	Total Section 1-8		\$	9,599,800	x	10%	=	\$	959,980		
			Ŷ	0,000,000	~			•	,		
						TOTAL RO	ADV	VAYN	OBILIZATION	\$	960,000
SECTION 10: SUPPLEMEN	ITAL WORK										
Item code		Unit		Quantity		Unit Price (\$)			Cost		
066670 Payment Adjustments For	or Price Index	LS		1	х	52,800.00	=	\$	52,800		
066070 Maintain Traffic		LS		1	х	50,000.00	=	\$	50,000		
066094 Value Analysis		LS		1	х	10,000.00	=	\$	10,000		
066919 Dispute Resolution Boar	ď	LS		1	х	7,500.00	=	\$	7,500		
066015 Federal Trainee Program		LS		1	х	7,200.00	=	\$	7,200		
066610 Partnering		LS		1	x	20,000.00	=	\$	20,000		
XXXXXX Additional Earthwork		LS		1	x	50,000.00	=	\$	50,000		
	Cost of <b>NPD</b>		oleme	ntal Work spe		d in Section 5D	=	\$	25,000		
	Total Section 1-8		\$	9,599,800		0.5%	=	\$	47,999		
								EME	NTAL WORK	\$	270,500
						IUTAL SU	<b>7 7</b>		ITTAL WORK	φ	270,500

### SECTION 11: STATE FURNISHED MATERIALS AND EXPENSES

tem code		Unit	Quant	ly i	Unit Price (\$)			Cost	
066062	COZEEP Contract	LS	1		100,000.00	=		\$100,000	
066063	Traffic Management Plan - Public Information	LS	1	2	10,000.00	=		\$10,000	
066105	Resident Engineers Office	LS	1	2	83,200.00	=		\$83,200	
	Water Expenses	LS	1	2	1,000.00	=		\$1,000	
066871	Electrical Service Connections	LS	1	1	20,000.00	=		\$20,000	
	Total Section 1-8		\$ 9,5	9,800	0.50%	=	\$	47,999	
					то	TAL S	TATE	FURNISHED	\$152,200
ECTION	N 12: TIME-RELATED OVERHEAD Total of Roadway and Structures Contract Items excluding Total Construction Cost (excluding TRO and C Estimated Time-Related Overhead (T	Contingency)	\$13	668,563 (u	· · ·	greater	r than \$	5 million excluding contin	gency)
	Total of Roadway and Structures Contract Items excluding Total Construction Cost (excluding TRO and C	Contingency) RO) Perce	\$1: entage (0%	668,563 (u o 10%) :	sed to check if project is	: greater	r than \$	-	gency)
ECTION	Total of Roadway and Structures Contract Items excluding Total Construction Cost (excluding TRO and C	Contingency)	\$13	668,563 (u o 10%) :	sed to check if project is	greater	r than \$	5 million excluding contin	gency)
em code	Total of Roadway and Structures Contract Items excluding Total Construction Cost (excluding TRO and C	Contingency) RO) Perce	\$1: entage (0%	668,563 (u o 10%) :	sed to check if project is 3.00% Unit Price (\$)	: greater	r than \$	-	gency)

Total Section 1-12	\$ 11,345,300	х	15%	=	\$1,701,795	
				TOTAL	CONTINGENCY	\$1,701,800

### **II. STRUCTURE ITEMS**

DATE OF ESTIMATE Bridge Name Bridge Number Structure Type Width (Feet) [out to out] Total Bridge Length (Feet) Total Area (Square Feet)	Retaining Walls Ground Anchor Walls 08/03/22 N/A N/A Ground Anchor 0 LF 0 LF 2150 SQFT	Retaining WallsSoil Nail Walls06/30/22N/AN/ASoil Nail0LF06220SQFT	- 0 LF 0 LF 0 SQFT
Structure Depth (Feet) Footing Type (pile or spread)	0 LF	0 LF	0 LF
Cost Per Square Foot	\$330 \$709,500	\$200	\$0 <b>\$0</b>

DATE OF ESTIMATE Bridge Name Bridge Number Structure Type Width (Feet) [out to out] Total Bridge Length (Feet) Total Area (Square Feet) Structure Depth (Feet) Footing Type (pile or spread) Cost Per Square Foot	- 0 LF 0 SQFT 0 LF \$0	- 0 LF 0 LF 0 SQFT 0 LF \$0	- 0 LF 0 LF 0 SQFT 0 LF \$0
COST OF EACH	\$0	\$0	\$0

	TOTAL COST O	TOTAL COST OF BRIDGES		
	TOTAL COST OF	TOTAL COST OF BUILDINGS		
	STRUCTURES MOBILIZATION	10%	\$195,350	
Recommended Contingency: (Pre-PSR 30%-50%, PSR 25%, Draft PR 20%, PR Total recommended percentages includes any quantified risk based contingency	, , ,			
Total recommended percentages includes any quantified risk based contingency	STRUCTURES CONTINGENCY	25%	\$537,213	
то	TAL COST OF STRUCTURES		\$2,686,063	

Estimate Prepared By:

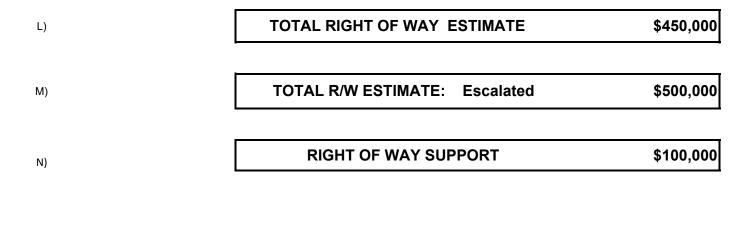
Anthony Richardson, Biggs Cardosa Associates, Inc.

Date

### **III. RIGHT OF WAY**

Fill in all of the available information from the Right of Way Data Sheet.

A)	<ul><li>A1) Acquisition, including Excess Land Purchases, Damages &amp; Goodwill, Fees</li><li>A2) SB-1210</li></ul>	\$ \$	250,000 0
B)	Acquisition of Offsite Mitigation	\$	0
C)	<ul><li>C1) Utility Relocation (State Share)</li><li>C2) Potholing (Design Phase)</li></ul>	\$ \$	200,000 0
D)	Railroad Acquisition	\$	0
E)	Clearance / Demolition	\$	0
F)	Relocation Assistance (RAP and/or Last Resort Housing Costs)	\$	0
G)	Title and Escrow	\$	0
H)	Environmental Review	\$	0
I)	Condemnation Settlements 0%	\$	0
J)	Design Appreciation Factor 0%	\$	0
K)	Utility Relocation (Construction Cost)	\$	0



Support Cost Estimate Prepared By	Project Coordinator <sup>1</sup>	Phone	
Utility Estimate Prepared			
Ву	Utility Coordinator <sup>2</sup>	Phone	
R/W Acquisition Estimate			
Prepared By	Right of Way Estimator <sup>3</sup>	Phone	
Note: Items G & H applied to items A + I	3		
<sup>1</sup> When estimate has Support Costs only	<sup>2</sup> When estimate has Utility Relocation	<sup>3</sup> When R/W Acquisition is required	

#### EA: 01-0K300 PID: 120000056

#### IV. SUPPORT COST ESTIMATE SUMMARY

Run a Support Cost Estimate Summary report (D11 Project Management Support onramp) for component data.

		Unescalated-Risk Loaded					Escalated (4.2% per year for ETC, effective 1/2/2018)					
Total by FY		PA&ED	PS&E	RW	CON	Total \$	PA&ED	PS&E	RW	CON	Total \$	
<2016	Expended											
	ETC											
2017	Expended											
	ETC											
2018	Expended											
	ETC											
2019	Expended											
	ETC											
2020	Expended											
	ETC											
2021	Expended											
	ETC											
2022	Expended											
	ETC											
2023	Expended											
	ETC											
2024	Expended											
	ETC											
2025	Expended											
	ETC											
2026	Expended											
	ETC											
2027	Expended											
	ETC											
2028 2029 >2030	Expended											
	ETC Expended											
	EXpended											
	Expended ETC											
EAC (Expor		\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	\$0	<u>60</u>	
EAC (Expended + ETC)												
sk Amount from Risk Register		\$0	\$0	\$0		Escalated Risk Amo	\$0	\$0	\$0	\$0	\$0	
Support Escalation Rate		0.0%	0.0%	0.0%	0.0%							
ration to mid-point component		0.00	0.00	0.00	0.00							
Total including Risk Amount		\$0	\$0	\$0	\$0	Total Esc. Support	\$0	\$0	\$0	\$0	\$0	
Approved Bu												
Difference (Budget - EAC)		\$0	\$0	\$0	\$0							
Support Ratio (EAC / Cap Cost)		0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	

Total Capital Cost:	\$16,184,000	
Total Capital Outlay Support Cost:	\$0	
Overall Percent Support Cost:	0.00%	

PRSM workplan hours/costs verified against approved MWA:

Approved by:

Office Chief -

Date

Project Control -

Date